FDOT FLORIDA DEPARTMENT OF TRANSPORTATION





SR 869 (SAWGRASS EXPRESSWAY) WIDENING PROJECT DEVELOPMENT & ENVIRONMENT (PD&E) STUDY From West of US 441 (SR 7) to Powerline Road (SR 845)

FPID No.: 437153-1-22-01 • ETDM No.: 14280 • Broward County











44





POND SITING REPORT

Florida Department of Transportation Florida's Turnpike Enterprise

SR 869/Sawgrass Expressway Project Development and Environment (PD&E) Study Broward County, Florida

> Financial Project ID Number: 437153-1-22-01 ETDM Number: 14280

DRAFT

JANUARY 2024



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1.0 EXECUTIVE SUMMARY

The Florida Department of Transportation (FDOT), Florida's Turnpike Enterprise (FTE), is performing a Project Development and Environment (PD&E) Study for State Road 869 (SR 869)/Sawgrass Expressway from west of US 441/SR 7 to Powerline Road (SR 845), a distance of approximately 4 miles (see Figure 1.1). The objective of this PD&E Study is to evaluate corridor modifications to improve operations and interchange access. The proposed improvements will address existing and future traffic needs, improve travel time reliability, enhance safety, and provide long-term mobility options along the corridor. The study is evaluating additional lanes, new collector distributor (CD) roadway systems and interchange improvements.

The study also includes 2.7 miles of the Florida's Turnpike (SR 91) from Wiles Road to the Broward/Palm Beach County Line. The study area is located in Broward County and traverses the cities of Parkland, Coral Springs, Coconut Creek, and Deerfield Beach, as well as an area of unincorporated Broward County.

This Pond Siting Report includes the preliminary drainage calculations to determine the water management facilities necessary for the widening from the existing six-lane rural divided expressway to ten-lane or twelve-lane rural divided expressway alternatives. The corridor traverses the Cities of Coral Springs, Parkland, Coconut Creek and Deerfield Beach, as well as an area of unincorporated Broward County.

The project site is located under the jurisdiction of South Florida Water Management District (SFWMD) Hillsboro Basin and primarily discharges to local drainage district canals along the Sawgrass Expressway and Florida's Turnpike. The local drainage districts are, Pinetree Drainage District, Cocomar Drainage District and Broward County Water Control District #2.

The current stormwater management facilities were designed and permitted for an 8-lane roadway section when it was originally designed for a four-lane section. During the design and construction of the widening to 6-lane section, the facilities were regraded to ensure the provision of the original permitted storage. The preferred alternative is analyzed for stormwater management requirements. The ten-lane option with CD system will significantly impact the roadside swales and therefore the interchange ponds will be utilized for stormwater management. Roadside swales will



be used as much as possible to convey the runoff into the proposed water management facilities.

There are several offsite FTE parcels available within project corridor to accommodate stormwater management needs. No Right-of-Way acquisition is deemed necessary to provide the required water quality, quantity, and floodplain compensation storage with the preferred stormwater management option.

This PD&E Study cannot accommodate the stormwater needs of The Florida's Turnpike widening project south of Wiles Road. However, it can meet stormwater requirements for the Turnpike section north of the Hillsboro Boulevard.

The vertical datum for this study is NAVD 1988.



2.0 INTRODUCTION

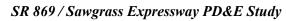
The Florida Department of Transportation (FDOT), Florida's Turnpike Enterprise (FTE), is performing a Project Development and Environment (PD&E) Study for State Road 869 (SR 869)/Sawgrass Expressway from west of US 441/SR 7 to Powerline Road (SR 845), a distance of approximately 4 miles (see Figure 1.1). The objective of this PD&E Study is to evaluate corridor modifications to improve operations and interchange access. The proposed improvements will address existing and future traffic needs, improve travel time reliability, enhance safety, and provide long-term mobility options along the corridor. The study is evaluating additional lanes, new CD roadway systems and interchange improvements.

The study also includes 2.7 miles of the Florida's Turnpike (SR 91) from Wiles Road to the Broward/Palm Beach County Line. The study area is located in Broward County and traverses the cities of Parkland, Coral Springs, Coconut Creek, and Deerfield Beach, as well as an area of unincorporated Broward County.

The proposed widening of the Sawgrass Expressway within the project limits results in an increased need for water quality and water quantity storage along the corridor. Additionally, several of the interchanges along the corridor are being modified resulting in the need to upgrade and modify the corresponding stormwater management, collection, and conveyance systems. The scope of the stormwater management system improvements will include the following:

- Reshaping of existing roadside dry detention/retention swales to provide storage capacity as needed.
- Reshaping of existing interchange infield areas and wet ponds to provide storage capacity as needed.
- Conversions of Turnpike parcel to either wet or dry detention ponds or floodplain mitigation
- Modification of existing outfall structures as needed.
- Modification of cross drains and stormwater collection and conveyance systems.

The current stormwater management facilities were designed and permitted for an 8-lane typical section. During the design and construction of the widening to 6-lane section, the facilities were regraded to ensure the provision of the original





permitted storage. The preferred alternative was analyzed for stormwater treatment, attenuation, and floodplain compensation. A Pond Siting Report (PSR) and a Location Hydraulics Technical Memorandum (LHTM) have been prepared for the preferred roadway alternative in accordance with Part 2 of the FDOT PD&E Manual.



3.0 **PROJECT DESCRIPTION**

This project proposes improvements to the portion of Sawgrass Expressway from US 441 to Powerline Road, approximately four miles including the interchanges.

The entire Sawgrass Expressway is a tolled, 21-mile limited access facility located in northern Broward County. Most of the facility is located on the western fringe of the Broward urban area. The Sawgrass Expressway is part of the Florida's Strategic Intermodal System (SIS), and the National Highway System (NHS). In addition, Sawgrass Expressway is designated as an evacuation route providing connectivity to other evacuation routes such as I-75, Florida's Turnpike and I-95. The existing limited access right-of-way width varies within the study limits but is generally 300 feet wide. The right-of-way is typical throughout the corridor except at the interchanges, where it varies to accommodate entrance and exit ramps. See **Figure 3.1** for project location map.

SR 869 / Sawgrass Expressway PD&E Study

Draft Pond Siting Report

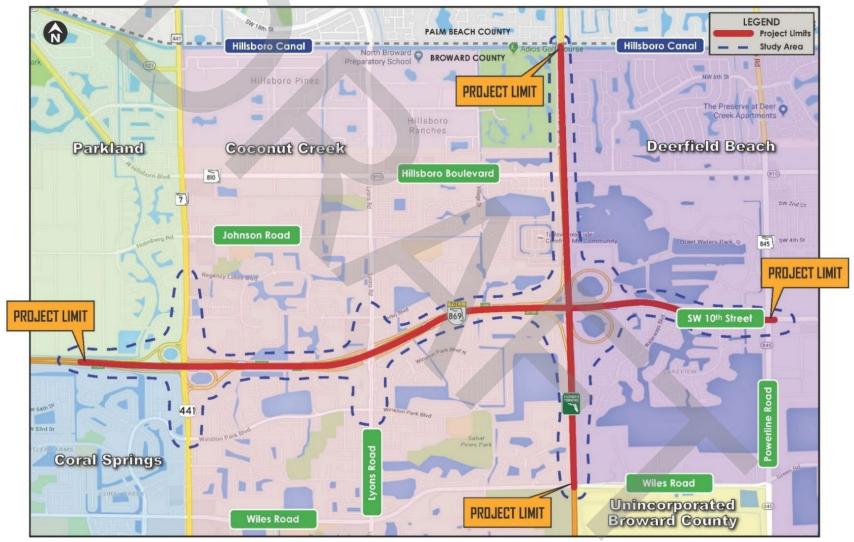


Figure 3.1 - Project Location Map



The vertical datum for this study is NAVD 1988. The conversion from NGVD to NAVD '88 datum is: NAVD'88 = NGVD'29 - 1.529 ft.

3.1 EXISTING TYPICAL SECTION

Sawgrass Expressway – Sawgrass Expressway, between west of US 441 and Florida's Turnpike, consists of four to six 12-foot wide travel lanes (two to three lanes in each direction) with 12-foot wide auxiliary lanes at select locations and 12-foot wide inside and outside shoulders. The median width varies within this segment of the corridor. The section between west of US 441 and east of Lyons Road has a 64-foot wide depressed grassed median separated by guardrail (see *Figure 3.2(a)* and *Figure 3.2(b)*). The section between east of Lyons Road and Florida's Turnpike narrows down to a 2-foot wide median barrier wall (see *Figure 3.2(c)*). This section also has a two-lane collector-distributor roadway system with 12-foot wide travel lanes and 12-foot wide auxiliary lanes on both sides of the corridor providing ramp access to and from the Florida's Turnpike. Inside and outside shoulder widths vary depending on the number on lanes, tolling point locations and ramp gores.

SW 10th Street – SW 10th Street, between Florida's Turnpike and Powerline Road, consists of six 12-foot wide travel lanes (three lanes in each direction) and a raised grassed curbed median. The median width varies between 30-65 feet wide. The median currently accommodates landscape vegetation and exclusive left-turn lanes at the intersections (see *Figure 3.2(d)*).

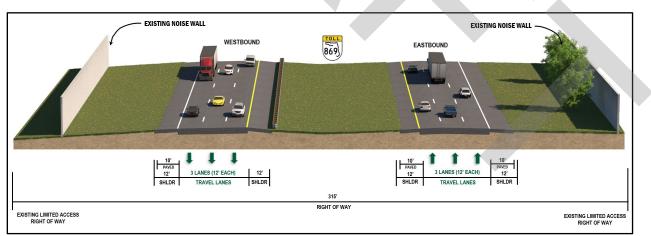


Figure 3.2(a) - Existing Roadway Section of SR 869 West of US 441



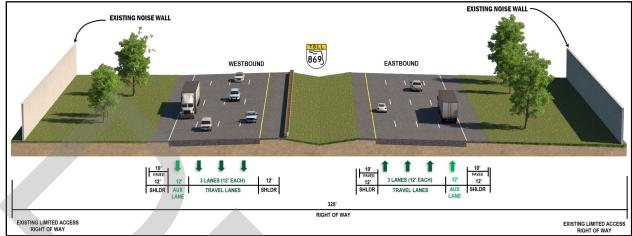


Figure 3.2(b) - Existing Roadway Section of SR 869 between US 441 and Lyons Road

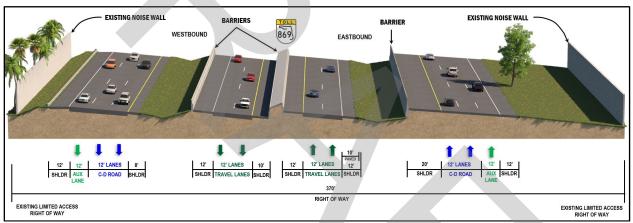
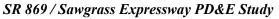
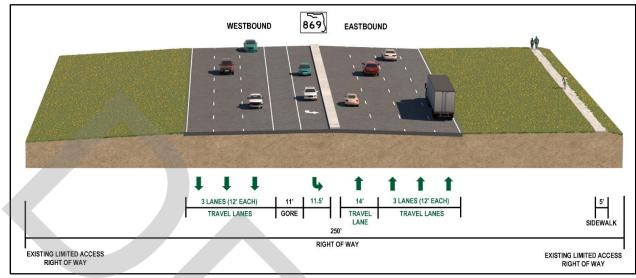


Figure 3.2(c) - Existing Roadway Section of SR 869 between Lyons Road and Florida's Turnpike







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Figure 3.2(d) - Existing Roadway Section of SR 869 between Florida's Turnpike and Powerline Road

Florida's Turnpike – Florida's Turnpike, between Wiles Road and the County Line, consists of six 12-foot wide travel lanes (three lanes in each direction) with 8.5-10-foot wide inside shoulders, 12-foot wide outside shoulders and a 2-foot wide median barrier wall (see *Figure 3.3(a)* and *Figure 3.3(b)*).

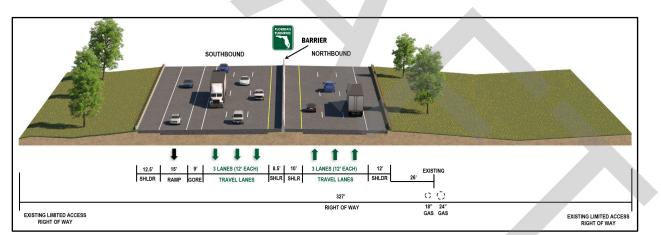


Figure 3.3(a) – Existing Roadway Section of Florida's Turnpike between Wiles Road and Sawgrass Expressway



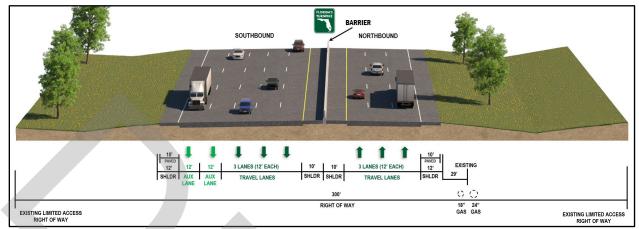


Figure 3.3(b) - Existing Roadway Section of Florida's Turnpike between Sawgrass Expressway and the County Line

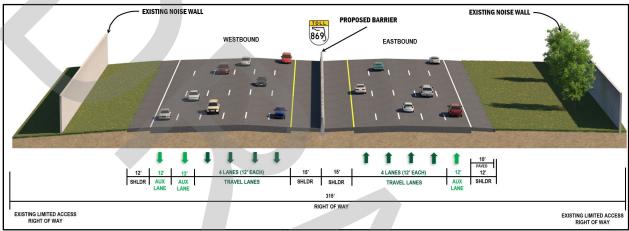
3.2 PROPOSED TYPICAL SECTION

Sawgrass Expressway – The preferred alternative proposes to widen the Sawgrass Expressway to four travel lanes in each direction with auxiliary lanes at select locations. The preferred alternative also includes collector distributor roadway systems on both sides of the corridor. The collector distributor roadway systems will separate local traffic and interchange traffic from the mainline traffic. Separating traffic patterns reduces lane changes, weaving maneuvers, speed differentials and friction along the corridor. The collector distributor roadway systems will be barrier separated from the Sawgrass Expressway mainline lanes.

Sawgrass Expressway, west of US 441, will consist of 12-foot wide travel lanes and auxiliary lanes with 15-foot wide inside shoulders, 12-foot wide outside shoulders and a 2-foot wide median barrier wall (see *Figure 3.4(a)*). This section is consistent with the proposed Sawgrass Expressway widening project to the west between Atlantic Boulevard and west of US 441 (FPID# 435461-1). Between US 441 and Lyons Road, the roadway section will consist of 12-foot wide travel lanes and auxiliary lanes with 15-foot wide inside shoulders, 12-foot wide outside shoulders and a 2-foot wide median barrier wall. The collector distributor roadway systems begin at US 441 and end at the Florida's Turnpike. Between US 441 and Lyons Road, the collector distributor roadway system will consist of two 12-foot wide travel lanes with varying inside and outside shoulders widths between 8-12 feet wide separated from the mainlines lanes with a 2-foot wide barrier wall (see *Figure 3.4(b)*).



Between Lyons Road and Florida's Turnpike, the roadway section will consist of 12foot wide travel lanes and auxiliary lanes with varying inside and outside shoulders widths between 12-14 feet wide and a 2-foot wide median barrier wall. The collector distributor roadway system will consist of two 12-foot wide travel lanes and one auxiliary lane with varying inside and outside shoulders widths between 8-12 feet wide separated from the mainlines lanes with a 2-foot wide barrier wall (see **Figure 3.4(c)**).





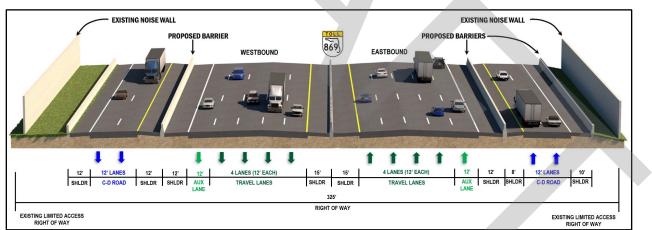


Figure 3.4(b) - Preferred Alternative Roadway Section of SR 869 between US 441 and Lyons Road



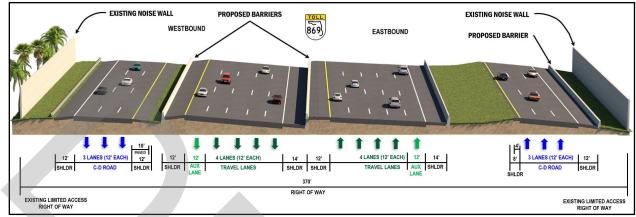


Figure 3.4(c) - Preferred Alternative Roadway Section of SR 869 between Lyons Road and Florida's Turnpike

SW 10th Street – SW 10th Street, between Florida's Turnpike and Powerline Road, will consist of two separate roadway corridors: 1) SW 10th Street and 2) SW 10th Street Connector. This roadway section overlaps with the SW 10th Street project currently underway by FDOT District Four (FPID# 439891-1). This project is proposing to add two limited access connector lanes in each direction on the north side of the existing SW 10th Street corridor between Florida's Turnpike and I-95. The FDOT project is also proposing other corridor improvements along the SW 10th Street existing corridor (see *Figure 3.4(d)*).

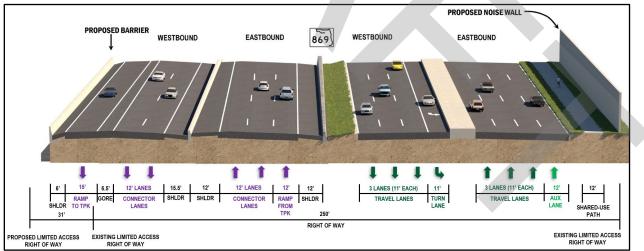


Figure 3.4(d) - Preferred Alternative Roadway of SR 869 between Florida's Turnpike and Powerline Road



Florida's Turnpike – The preferred alternative proposes to widen the Florida's Turnpike between Wiles Road and the County Line to four travel lanes and one thru lane in each direction for a total of ten lanes, with auxiliary lanes at select locations (see *Figure 3.5(a)* and *Figure 3.5(b)*). Thru lanes are additional travel lanes that help provide congestion relief in high traffic areas. These lanes offer customers making longer, more regional trips, the ability to bypass the local traffic entering and exiting the road. Customers pay the same amount to use the thru lanes as they do in any other lane on the toll road. All mainline lanes and shoulders are 12-foot wide, with a 2-foot wide median barrier wall.

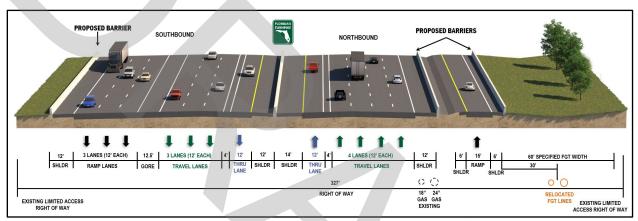


Figure 3.5(a) – Preferred Alternative Roadway Section of Florida's Turnpike between Wiles Road and Sawgrass Expressway

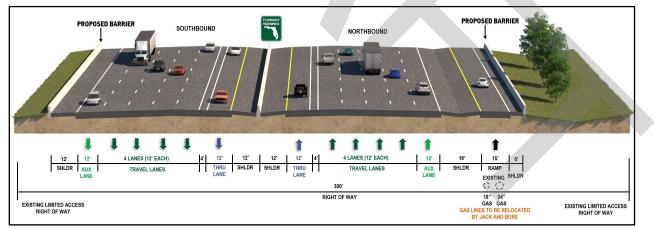


Figure 3.5(b) - Preferred Alternative Roadway Section of Florida's Turnpike between Sawgrass Expressway and the County Line



3.3 ADJACENT STUDIES/PROJECTS

As shown in *Figure 3.6* there are a total of twelve studies/projects within the project area.



ote: Projects 1 and 2 have overlapping limits between Florida's Turnpike and Powerline Road. Projects 1 and 12 have overlapping limits between Sawgrass Expressway and the County Line.

Figure 3.6 – Adjacent Projects



There are four immediate adjacent projects.

North of Project Limits along Florida's Turnpike – FPID#415927-1 – Florida's Turnpike from the Hillsboro Canal to Hillsboro Boulevard PD&E Study:

The roadway segment from the Hillsboro Canal to Hillsboro Boulevard overlaps with this PD&E Study. Stormwater runoff from the segment south of the Hillsboro Canal flows south and into the Sawgrass northeastern interchange pond. The required treatment, attenuation and flood mitigation will be provided within the project.

South of Project Limits along Florida's Turnpike – FPID#-442212-1 – Florida's Turnpike from Atlantic Boulevard to Wiles Road PD&E Study:

Runoff from northern segment of the project, from Sample Road to Wiles Road, is within the Hillsboro Canal basin. Rainfall runoff enters the southern limits of the Turnpike/Sawgrass PD&E project limits at Wiles Road. A PSR prepared by the PD&E Study to the south identified three alternative pond sites. The preferred site is Alternative 1, which is a joint-use stormwater management facility with the Tradewinds Park and entails a land swap in connection with the Sample Road Interchange.

The roadway improvements will also require approximately 17.2 ac-ft of floodplain mitigation as the widening impacts FEMA floodplains on the west side of the Turnpike. Alternatives 2 and 3, in the vicinity of Wiles Road are mentioned in the PSR as potential floodplain mitigation sites. Both sites are not well-suited for floodplain mitigation as Alternative 2 site is a wetland and Alternative 3 site is a landfill and designated as contaminated site.

There is not enough floodplain mitigation volume available within project limits to accommodate required mitigation volume for the project, FPID#406150 (the FTE south of Wiles Road). Floodplain mitigation volume needs to be revisited to accommodate floodplain needs.

East of Project Limits along Sawgrass Expressway – 439891-1 – SW 10th Street from Sawgrass/Turnpike Interchange to Military Trail:

The project overlaps the section from the Florida's Turnpike interchange to Powerline Road. Runoff from the SW 10th Street is designed to flow to the interchange ponds for treatment and attenuation. The interchange ponds also



used for flood mitigation. The overlapped section of Sawgrass Expressway/SW 10th Street will be reconstructed. Therefore, provided volume for this project is not considered for volume calculations of the Sawgrass Expressway PD&E study.

West of Project Limits along Sawgrass Expressway – 435461-5 – Sawgrass Expressway from West of University Drive to US 441 (SR 7) Design:

The roadway segment west of SR 441 overlaps with this PD&E Study. Project, FPID#435461-5 have enough volume capacity within its project limits. Therefore, no additional treatment, attenuation and floodplain mitigation volume is required to accommodate within the Sawgrass Expressway PD&E study.



4.0 DATA COLLECTION

The following documents are the main sources of information consulted and evaluated for the preparation of this PSR.

Permit Research:

- SFWMD Permit No. 06-00629-S for the Sawgrass Expressway in Broward County Issued on February 14, 1985, and further permit modifications.
- SFWMD PERMIT No. 06-10034-S for the Florida's Turnpike in Broward County Issued on - February 14, 1985, and further permit modifications.

Original As-built plans:

- Sawgrass/Deerfield Expressway Section V (Sta. 910+00 to Sta. 1110+00.00) from East of University Drive to East of Lyons Road
- Sawgrass/Deerfield Expressway Section VI (Sta. 1110+00.00 to Sta. 1200+00.00) from East of Lyons Road to East of Powerline Road
- FPID 406153-1-52-01 Sawgrass Expressway Widening from Coral Ridge Drive to Florida's Turnpike (Sta. 786+00 to Sta. 1152+00)
- FPID 412286-2-52-01 Sawgrass Expressway Widening from Coral Ridge Drive to Florida's Turnpike (Sta. 786+00 to Sta. 1152+00)
- FPID 420289-6-52-01 Sawgrass Expressway Deerfield Toll Plaza Modifications 2007 (Sta. 1095+98.12 to Sta. 1178+82.52)
- FPID 431281-1 AET5B
- FPID 406150-152-01, As- built plans for the 6-lane Widening Turnpike:
- FPID 232316-1-52-01 Turnpike North and South of Sawgrass Expressway
- FPID 232233-1-52 Turnpike from North of Sample Road to South of the Hillsboro Canal

Drainage Reports:

• FPI 466150-1 Turnpike widening from Atlantic Boulevard to the Sawgrass Expressway - SFWMD ERP No. 060509-10



5.0 DESIGN CRITERIA AND AGENCY COORDINATION

5.1 DESIGN CRITERIA

The design criteria considered in the development of the drainage for this project are summarized in **Table 5.1**.

Design Element	Design Standard	Source		
Open Channel	10 Year for Ditches/Swales	DM Section 2.2		
Design Frequency	25 Year for Outfall Ditches and Canals	Table 2.1		
Open Channel Minimum Slope	0.0005 ft/ft	DM Section 2.4.2		
Channel Velocity (Maximum)	· ·			
Storm Drain Design Frequency	3 Year for General Design 10 Year for Interstate Facilities	DM Section 3.3 Table 3.1		
Storm Drain Design Tailwater	Stormwater Ponds: Peak stage in the pond during storm drain design event French Drains: Design Head over the outlet control structure Regulated Canals: Agency regulated control elevation	DM Section 3.4		
Minimum Time of Concentration	10 Minutes	DM Section 3.5.1		
Minimum Pipe Slope	Minimum Slope which produces a storm drain velocity of 2.5 fps when full and no greater than 15 fps when the storm drain is flowing full	DM Section 3.6.1		
Hydraulic Gradient	When minor the Hydraulic Grade Line (HGL) energy losses are not considered, HGL shall be 1 ft below the theoretical gutter elevation	DM Section 3.6.2		
Outlet Velocity	When outlet velocity exceeds 6 fps provide special channel lining and/or energy dissipater	DM Section 3.6.3		
Spread Standards	Spread resulting from 4 inches per hour shall be limited to: ½ lane for < 45 MPH 8 ft of lane clear for 45 MPH to 55 MPH No encroachment for > 55 MPH	DM Section 3.9 Table 3.9.1		
Minimum Pipe Size	18 inches	DM Section 3.10.1		

Table 5.1 - Drainage Design Criteria



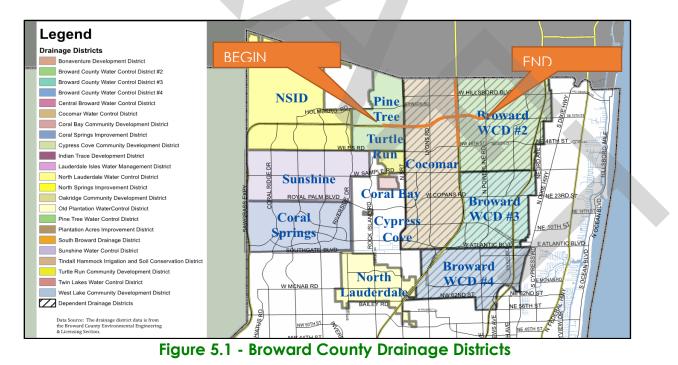
Table 5.1 - Drainage Design Criteria (continued)

Design Element	Design Standard	Source	
Maximum Pipe Length	Pipe without French Drains 300 ft for 18 inches pipes 400 ft for 24 to 36 inches pipes 500 ft for > 42 inches pipes French Drains (Minimum Length from Access) 150 ft for 18 to 30 inches pipes 200 ft for > 36 inches pipes	DM Section 3.10.1	
Cross Drains Design Frequency	50 years for Mainline Interstate and Facilities with projected 20 year ADT > 1500 25 years for Facilities with projected 20 year ADT < 1500 10 years for roadside ditch culverts	DM Section 4.3	
Wet Detention and Retention Ponds	20 ft minimum between top edge of normal pool elevation and right of way line, 15 ft adjacent to	DM Section 5.4.4.2	
Maintenance Berm	the water sloped at 1:8 or flatter	SFWMD ERP Manual Section 7.5	
Detention and Retention Ponds Freeboard	1 ft freeboard required above peak design stage for ponds and 0.5 foot minimum freeboard for linear treatment swales	DM Section 5.4.4.2	
Wet Detention and Retention Ponds	Total Area = 0.5 acre minimum Slopes between control elevation and 2 ft below it	DM Figure 5-1	
Requirements	shall be 1:4 or flatter	SFWMD ERP Manual Section 7.4	
Water Quality Requirements	Wet Detention: Greater of 1 inch over total project area or 2.5 inches over total impervious Dry Detention: 75% of wet detention Wet/Dry Retention: 50% of wet or dry detention accordingly	SFWMD ERP Manual Section 5.2.1	
Water Quality Requirements	Post Development discharge rate equal to or less than pre development discharge rate for 25 year – 3 day storm event, or rates specified in district criteria	SFWMD ERP Manual Section 6.2 and 6.3	
Floodplain Encroachment	No encroachment allowed	SFWMD ERP Manual Section 6.4	
Outfall Structures	Structures shall include baffles systems. Structures shall include bleed down notch or orifice that allows ½ inches of the detention volume to be discharged within 24 hours.	SFWMD ERP Manual Section 7.1 and 7.2	



This study corridor is located within SFWMD Hillsboro Canal Basin and under the local drainage district authorities of Pinetree Drainage District (PTWCD), Cocomar Drainage District (CWCD) and the Broward County Water Control District #2 (BCWCD #2). Refer to *Figure 5.1* below for local drainage districts.

The original SFWMD Permit No. 06-00629-S included 8-lane widening within project corridor for SR 869. However, the proposed improvement will eliminate most of the roadside swales and therefore will require complete redesign of stormwater management system. A new Environmental Resource Permit is anticipated for the Sawgrass Expressway improvement. The Permit Application will involve the agencies with jurisdiction over this project are, a SFWMD, US Army Corps of Engineers (USACE) and the Florida Department of Environmental Protection (FDEP). The SFWMD water quality criteria requires wet detention treatment volume from the first inch of runoff of the total area, or 2.5 inches of runoff from the impervious area, whichever is greater. For dry detention or dry retention 75% or 50% of this treatment volume should be provided respectively. The peak discharge from post development condition cannot exceed pre-development or previously permitted discharges.



The project west of the Florida's Turnpike is in Water Preserve Area (WPA), which requires an additional 50% water quality volume for additional impervious area.



Since required treatment volume for the study is calculated for maximum of project area or 2.5" of total impervious area, the provided treatment volume is larger than required treatment volume. **Table 5.2** shows a comparison of provided treatment volume and required treatment volume for WPA basin (Basin 1 and 2 in this study). Therefore, the WPA water quality criteria is satisfied for the study.

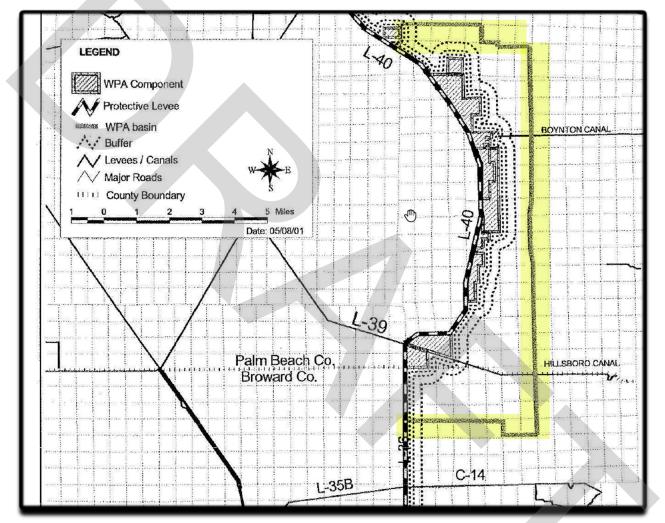


Figure 5.2 - Water Preserve Area Basin



Table 5.2 - Comparison of Provided and Required Treatment Volume with theSFWMD WPA Basins

Basin Name	(1) Total Area (ac)	(2) Imp. Area - Existing (ac)	(3) Imp. Area - Proposed (ac)	(4) Additional Imp. Area (ac)	(5) Required Treatment Vol. 2.5" of Additional Imp. Area (Ac-ft)	(6) Additional 50% Treatment Vol. for Water Preserve Area Basin (ac-ft)	(7) Required Treatment Vol. per Water Preserve Area Basin (ac-ft)	(8) Calculated Required Treatment Vol. (ac-ft)	(9) Provided Treatment Vol. more than Rquired Treatment Vol.?
1	110.64	45.33	86.76	41.43	8.63	4.32	12.95	13.45	Yes
2	85.88	40.29	64.08	23.79	4.96	2.48	7.43	12.05	Yes

Column (1) = From Table 10.1 Water Quality and Quantity Calculation Summary Column (2) = From Table 10.1 Water Quality and Quantity Calculation Summary

Column (3) = From Table 10.1 Water Quality and Quantity Calculation Summary

Column (4) = Column (3) -(2)

Column (5) = 2.5" X Column (4)/12

Column (6) = 50% of Column (5)

Column (7) = Column (5) + (6)

Column (8) = From Table 10.1 Water Quality and Quantity Calculation Summary

5.2 AGENCY COORDINATION

The water quality and quantity criteria and requirements have been reviewed in the pre-application meeting with SFWMD held on August 2, 2018. Since the project primarily discharges to local canals, coordination meetings were held with local drainage districts as well (Refer to **Appendix A**). A new Environmental Permit Application with current permitting criteria is the preferred permitting approach. The latest requirements from the FDOT Drainage Manual and Turnpike Design Handbook has been considered for sizing the water management facilities.



6.0 EXISTING CONDITION

6.1 EXISTING DRAINAGE CONDITION

Information was obtained from field review, planning documents, construction drawings, aerial photographs, and permit applications to identify the existing stormwater management system serving the study corridor, which is 4.68 miles long and covers an area of 430.56 acres.

The Sawgrass Expressway is a divided limited access highway with design speed of 65 miles per hour. The original 21-mile project for a four-lane divided expressway was separated into six sections for design purposes. The original stormwater management system for the Sawgrass Expressway, permitted by SFWMD on February 14, 1985, was approved for an ultimate eight-lane typical section. Roadway runoff water treatment was provided within the median and the roadside swales. Most of the former borrow pits and the interchange infield areas were not designed to provide water quality treatment. Subsequent projects during the 2000 decade widened the Sawgrass Expressway from four to six lanes. The additional travel lanes were added on the median, but the water management facilities remained the same with minor alterations based on the 8lane typical section originally permitted. Improvements to the interchange ramps, the addition of sound barrier walls and landscape have been reducing the original swales gradually.

Basin 1 through 6 are for Sawgrass Expressway.

Basin 1: This basin begins east of Riverside Drive and ends at SR-7/441 including the West side of the SR-7/441 interchange. The existing drainage system consist of catch basins, gutter drains, cross drain and storm sewer that convey the stormwater runoff into roadside swales and ponds. The roadside swales and the interchange ponds discharges into the PTWCD canals. The minimum roadway grade elevation is 16.50 feet NAVD, roadside ditch bottom elevation is 11.00 feet NAVD and PTWCD Control Elevation is 10.05 feet NAVD. There is also an FDOT parcel available the NW quadrant of the interchange beyond Sawgrass right-of-way. Basin 1 within the SFWMD Water Preserve Area (WPA).

Basin 2: This basin begins at US 441 and ends at Lyons Road including the east side of SR7/441 interchange and discharges to the local canal under the jurisdiction



of CWCD. Ditch blocks are regularly located along the ditches dividing them into subsystems. The local canals are interconnected with two 72" cross drains, both located west and east of Lyons Road. The minimum roadway grade elevation is 16.50 feet NAVD, roadside ditch bottom elevation is 11.00 feet NAVD and CWCD Control Elevation is 9.45 feet NAVD. Basin 1 within the SFWMD Water Preserve Area (WPA).

Basin 3: The basin from the Lyons Road to the west of Florida's Turnpike Interchange discharges to the local canal under the jurisdiction of CWCD. The local canals are interconnected through a 72" cross drain located west of the Florida's Turnpike interchange. The local canals ultimately discharge into the Hillsboro canal. The minimum roadway grade elevation is 17.50 ft. NAVD. The roadside ditch bottom elevation is 11.00 FT. NAVD. CWCD Control Elevation is 9.45 ft.

Basin 4: This basin begins east of Florida's Turnpike at Sta. 1145+00.00 and ends up at Sta. 1168+39.00, just west of the Waterways Boulevard along the Sawarass Expressway. Along the Florida Turnpike, the basin begins at Wiles Road at Sta. 5570+00.00 and ends up at Sta. 5665+00.00, just south of Hillsboro Boulevard. It includes the east and westbound to the Sawgrass Expressway and north and southbound of the Florida's Turnpike. The existing drainage system consists of catch basins, gutter drains, cross drains, and storm sewer that convey the runoff into roadside swales and ponds. Treatment for water quality is provided along the roadside swales using ditch blocks and raised ditch bottom inlets. Ditch blocks are regularly located along the ditches dividing them into subsystems. The existing ponds and swales located within the Turnpike right-of-way are all operated and maintained by Florida's Turnpike Enterprise. The runoff that accumulates in the ditches infiltrates into the ground and the excess is discharged by outfall structures, primary earthen weirs, into the receiving ponds at the Sawgrass Expressway Interchange. It then flows through a 48" culvert to the residential lake located at the south-east guadrant of the interchange, it continues within a residential community through a 20-foot drainage easement, and ultimately discharges into the C-3 Canal and Hillsboro Canal. The minimum roadway arade elevation is 17.50 ft. NAVD. The roadside ditch bottom elevation is 11.00 FT. NAVD. CWCD Control Elevation is 9.45 ft. NAVD and BCWCD#2 Control Elevation is 9.45 ft. NAVD.



Basin 5: This existing drainage basin begins east of the Florida's Turnpike at Sta. 1168+39.00 and ends at Sta. 1193+17.00, just west of Independence Drive. It includes the east and westbound roadway of SW 10th Street. It also include half of the road and the roadside swales of westbound SW 10th Street between Sta. 1193+17.00 and Sta. 1209+25.00. The existing drainage systems consist of catch basins, cross drains and storm sewers that convey stormwater runoff into roadside treatment swales along the west and eastbound SW 10th Street into the Florida's Turnpike drainage system at the Southeast Quadrant of the interchange through a raised ditch bottom inlet that is connected to the wet pond. The minimum roadway grade elevation is 12.00 ft. NAVD and roadside ditch bottom elevation is 11.00 FT. NAVD. CWCD Control Elevation is 9.45 ft. NAVD and BCWCD#2 Control Elevation is 9.50 ft. NAVD.

Basin 6: This basin begins west of Independence Drive at Sta. 1193+17.00 and ends up at Sta. 1209+25.00 at the Powerline Road Intersection. It includes the eastbound roadway of SW 10th Street. The existing drainage system consists of catch basins, cross drains and storm sewers that convey stormwater runoff into the roadside swales along the westbound SW 10th Street. The roadside swale along the south side of the road is connected to the local canal/lake through a 30" pipe which is connected to the 2-60" cross drain. This cross drain is west of Powerline Road and is part of the C-3 Canal. This system ultimately discharges into the Hillsboro Canal. The minimum roadway grade elevation is 14.00 ft. NAVD. The roadside ditch bottom elevation is 11.0 FT. NAVD. CWCD Control Elevation is 9.45 ft. NAVD. BCWCD#2 Control Elevation is 9.50 ft. NAVD. Existing drainage features are summarized below in **Table 6.1**.



	Sawgrass Expressway: Permitted Water Management Systems											
		Station		Length			Outfall	5	ss ns	MD n		
Existing Basin	From	То	(ft.)	(mile)	Acres) Acres	(Each)	Weir	Bleeder	Cross Drains	SFWMD Basin	Jurisdiction
1	962+30.00	1060+00.00	9770	1.85	129.73	8	0	0	4		PTWCD, CWCD SFWMD WPA	
2	1060+00.00	1110+00.00	5000	0.95	49.35	2	0	0	2	Hillsboro Canal	CWCD SFWMD WPA	
3	1110+00.00	1145+00.00	3500	0.66	34.70	0	0	0	1	Hillsbo	CWCD	
4	1145+00.00	1168+39.00	2339	0.44	183.90	3	0	0	5	I	CWCD, BCWCD#2	
5	1168+39.00	1193+17.00	2478	0.47	26.66	1	0	0	0		BCWCD#2	
6	1193+17.00	1209+25.00	1608	0.30	6.22	1	0	0	0		BCWCD#2	

Table 6.1 - Existing Outfalls

6.2 CROSS DRAINS

There are several pipes crossing Sawgrass Expressway, but only seven culverts provide conveyance beneath the corridor for off-site runoff: No erosion or structural damages have been reported for the cross drains.



Sawgrass Expressway: Existing Cross Drains								
CROSSDRAIN	MAINLINE STATION	PIPE COUNT	PIPE SIZE (IN)	PIPE LENGTH (FT)	PIPE TYPE	REMARKS		
CD-1A	963+00 (SR 869)	1	60	316	UNKNOWN	Local Canal		
CD-1	989+00 (SR 869)	1	60	307	RCP	Local Canal		
CD-2	1028+25 (SR 869)	2	72	530	RCP	Pine Tree Canal		
CD-3	1070+00 (SR 869)	1	72	324	RCP	Cocomar Canal		
CD-4	1105+58 (SR 869)	1	72	405	RCP	Local Canal		
CD-5	1127+82 (SR 869)	1	72	477	RCP	Local Canal		
CD-6	1158+30 ((SR 869))	1	48	396	RCP	Under SR 869 connecting wet ponds Pond 3B-4 and Pond 3B-5		
CD-7	5652+71 (SR 821)	1	48	185	RCP	Under SR 821		
CD-8	5639+13 (SR 821)	1	48	405	RCP	Under SR 821 connecting wet ponds Pond 3A-2 and Pond 3B-4		
CD-9	5631+00 (SR 821)	1	48	365	RCP	Under SR 821 connecting wet ponds Pond 3B-1 and Pond 3B-5		
CD-10	1203+07 (SR 869)	2	60	294	RCP	C-3 Canal		

Table 6.2 - Existing Cross Drains

6.3 OFF SITE FLOW CONTRIBUTION

The Sawgrass Expressway is mostly flanked by canals, berms, and interchange infield and sound barrier walls. As such, the off-site contributions to the roadway drainage systems are negligible.

6.4 SOILS AND GEOTECHNICAL CHARACTERISTICS

The evaluation of the NRCS Web Soil Survey within the study site reports that the existing soils are Margate fine sands, occasionally ponded, 0 to 1 percent slopes or Hallandale fine sands, 0 to 2 percent slope. The roadway footprint was de-mucked for the initial construction work and replaced with the soils excavated in the adjacent borrow canals. The hydrologic Soils Group is determined as D. Refer **Appendix B Soil Data and Geotechnical Information**. The geotechnical evaluation prepared by GCME, Inc reports Seasonal High Ground Water Table Elevation (SHGWT) varies between 11 ft. NAVD to 8.5 ft. NAVD.

Within the project limit Average Wet Season Water Table Elevation is determined from Broward County Average Wet season water table map. Based on the map from the beginning of the project to US 441 is SHGWT is 10.0 ft. NAVD and from



US 441 to end project is 9.50 ft. NAVD, which is consistent with the previous permit information.



7.0 PROPOSED CONDITION

7.1 PROPOSED DRAINAGE CONDITION

The current PD&E Study includes the last two sections (V and part of VI) of the original corridor improvement approximately four miles in length. The Study performs preliminary drainage analysis to determine the locations and sizes of the required water management facilities for the most critical alternative that will impact the drainage and environmental permitting needs. The use of roadside treatment swales, the former borrow pit areas and the infield areas in the four interchanges (US 441, Lyons Rd., Florida's Turnpike and Powerline Rd.) will be the first option to consider. Off-site ponds requiring right-of-way acquisition would be the last resort.

Four alternatives have been evaluated. The preferred alternative is used for the designing stormwater facilities. Proposed road section for the preferred alternative is shown in **Figure 3.4(a) to 3.4(d)**, **3.5(a) and 3.5(b)**.

7.2 PROPOSED BASINS

The project has been primarily divided into five basins to analyze the proposed condition.

Basin 1: Basin 1 begins from the beginning of the project to US 441, which includes the West side of the SR-7/441 interchange. Roadside swales, dry and wet detention ponds at the interchange are available for stormwater management. A FDOT owned parcel at the NW quadrant of the interchange beyond Sawgrass right-of-way is available, but not included in the calculations. Basin 1 discharges to Pinetree Water Control District Canals and is within the SFWMD Water Preserve Area.

Basin 2: This basin begins at SR-7/441 and ends up at Sta. 1110+00.00, just east of Lyons Road. It includes the East side of the SR-7/441 interchange, the ramps and bridges over Lyons Road, Lyons Road interchange as well as the east and westbound roadway of the Sawgrass Expressway. For stormwater management both dry treatment swales and wet detention ponds are available within this basin. Basin 2 discharges to the Cocomar Water Control District Canals and is within the SFWMD Water Preserve Area.



Basin 3A: This basin begins east of Lyons Road at Sta. 1110+00.00 and ends at Sta. 1153+00.00 at the Florida's Turnpike. It includes the eastbound Sawgrass Expressway, eastbound on ramp from FTE to Sawgrass and a portion of the southbound FTE. For stormwater management both dry treatment swales and wet detention ponds are available within this basin. Basin 3A discharges to Cocomar Water Control District Canals

Basin 3B: This basin begins at east of Lyons Road at Sta. 1145+00.00 and ends, just west of Waterways Boulevard, at Sta. 1168+39.00. It includes the Sawgrass Expressway east and westbound roadway and the Florida's Turnpike north and southbound roadway.

Interchange wet detention ponds are available for treatment and attenuation of this basin, before it discharges to the existing outfall at the southeast corner of the interchange. An FTE-owned parcel at the northeast corner of the interchange is available for stormwater management, which will be converted to a wet detention pond. Basin 3B discharges to BCWCD#2

Basin 3B-1: The basin begins north of Sawgrass Express interchange (Sta. 5652+50) and at Broward/Palm Beach County Line (Sta. 5716+70) along the Florida's Turnpike. The basin flows to the interchange wet pond through a dry swale. The swale also provides partial treatment and attenuation.

Basin 3B-2: The basin begins from Wies Road (Sta. 5575+50) to South of Sawgrass Expressway interchange (Sta. 5622+00) along the Florida's Turnpike. The basin flows to the interchange wet pond through swales where partially treatment and attenuation is provided.

Basin 4: This basin begins west of Independence Drive at Sta. 1193+17.00 and ends at Sta. 1209+50.00 at the Powerline Road intersection. In the proposed condition, existing swales are filled by the widening, therefore runoff from the basin area will be directed to the interchange ponds for treatment and attenuation. Since in the existing conditions runoff from the area does not flow to the interchange pond, no credit is taken for existing treatment and attenuation for the basin. Basin 4 discharges to BCWCD#2

Refer to **Appendix C** for proposed basins and Drainage Maps.



8.0 FLOODPLAIN AND ENVIRONMENTAL INFORMATION

8.1 FLOODPLAIN ANALYSIS

FEMA FIRMS have been reviewed to evaluate the floodplain impacts and required floodplain. In general, Sawgrass Expressway corridor is outside of the FEMA 100-year floodplain; however, some ramps and the interchange ponds and the vicinity of Sawarass Expressway are surrounded by FEMA Special Flood Hazard Areas Zones AE and AH with determined flood elevations (Refer to Appendix D for FEMA Maps). From the US 441 to the Turnpike/Sawgrass Expressway interchange the FEMA flood elevations range from EL. 10 ft. NAVD to EL. 14 ft. NAVD on the south side and from EL. 12 ft. NAVD to EL. 18 ft. NAVD on the north side. Excavation and fill volume between seasonal high water and 100-year flood elevation need to be accounted for at those areas. Some of the FTE Parcel are being converted to wet detention ponds, which will compensate for the loss of storage of 100-year flood plain. Refer to Table 8.1 for the Summary of Floodplain Compensation Calculations and Appendix D for detailed calculations. While the overall project meets floodplain mitigation requirements, there is a floodplain mitigation deficit within Basin 2. This deficit can be reduced by expanding the pond using MSE walls along the ramps.

LOCAL WATER MANAGEMENT DISTRICT	PROJECT BASIN	FLOODPLAIN ENCROACHMENT (AC-FT)	Provided Storage In	FLOODPLAIN COMPENSATIO N (AC-FT)
Pinetree Water Management District	BASIN 1	0.98	POND 1-1	1.26
Cocomar Water Management District	BASIN 2 & 3A	8.50	POND 2-1	5.17
			FLOOD	
Broward County Water Management District # 2	BASIN 3B, 3B-1,	38.76	MITIGATION 3B-1	18.70
Broward County Water Management District # 2	3B-2 & 4	56.70	FLOOD	
			MITIGATION 3B-2	23.26
	PROJECT TOTAL =	<u>48.24</u>		<u>48.39</u>

Table 8.1 - Summary of Floodplain Compensation Calculations

8.2 ENVIRONMENTAL INFORMATION

Wetland and Endangered Species

There is no wetland, archeological or historic sites impact within the project corridor. There is a presence of Bald Eagles nest at the NE corner of Sawgrass



interchange, which is to be protected. No stormwater facility has been proposed at this site.

Contamination

Draft Contamination Screening Evaluation Report (CSER) identifies a total of 16 sites of potential contamination risk. Those include two High Risk, five Medium Risk and nine Low Risk sites. None of the sites will be impacted by the proposed stormwater facilities. Refer to **Appendix E** for contamination map and Excerpts of Draft CSER

Wellfield Protection Zone

There is a wellfield at the North-East quadrant of Sawgrass and Turnpike interchange. The proposed drainage design will avoid any impact to the existing wellfield zone. Refer to **Appendix F** for Wellfield Protection Zone.



9.0 STORMWATER MANAGEMENT ALTERNATIVES

Stormwater management alternatives are considered based on the available options within and around the project corridor. Appropriate agency coordination has also been conducted to evaluate the alternatives. The following alternatives are considered by study.

9.1 DRY AND WET DETENTION

Throughout the project dry retention/detention facilities and wet detention ponds are available within roadway right-of-way, but this condition varies for different basins. Most of the existing roadside swales are diminished by the widening of the roadway except in Basin 1. Interchange wet ponds are large and underused in the existing condition in most cases. Besides, there are few FTE Parcels available for stormwater management. All these conditions made interchange ponds the most viable option for stormwater management. However, high tail water elevations controlled by the canal stages for the outfalls of the ponds and long stormsewer trunk lines to convey stormwater runoff due to lack of roadside swales pose a challenge to full utilization of the interchange ponds.

9.2 EXFILTRATION SYSTEM WITH DRY OR WET DETENTION

Exfiltration system is possible within the project corridor. Geotechnical tests show that the percolation rate varies from 4.72E-05 to 8.73E-06, which can be considered moderate to poor. Dry detention or wet detention facilities needs to be designed along with exfiltration system to provide water quality and attenuation of stormwater runoff. Exfiltration systems can be used to meet partial water quality requirements at the locations where dry or wet detention system is not possible or practical.

9.3 ENVIRONMENTAL LOOK AROUND

Coordination was carried out to investigate if opportunities are available to manage stormwater on a regional or join-use basis which would benefit other stakeholders. During coordination with BCWCD it was revealed that the NE side of the project is actually under welfield zone and it requires water recharge. It was also mentioned that ground water naturally flows from south to north towards Hillsboro Canal on the east side of the Florida's Turnpike. Due to loss of water, Broward County pumps water from Hillsboro Canal to C-3 Canal Basin. After coordination and discussion with the agencies no further concept was



established that would allow stormwater to help recharge the well field. In the agency coordination meeting it was also mentioned that the Cocomar Water Control District has control structures at the ultimate discharge point at Hillsboro Canal, which indicates additional water quality and quantity may be available within the system. However, the C-3 Canal basin does not have a defined water quality mechanism which could provide an opportunity to use C-3 Canal and associated waterbodies directly for stormwater management.

9.4 JOINT-USE POND OPPORTUNITY

An attempt was made to use Independence Bay pond as a join- use facility. The Independence Bay pond serves the entire private development at SE quadrant of Sawgrass Expressway and Turnpike interchange. During coordination meeting BCWCD mentioned that water level on the east side of the Turnpike is about a foot lower than west side and BCWCD always wanted to bring the runoff from the west side to the eastside of the Turnpike. This indicates that additional flow may be accommodated on the east side where Independence Bay pond is located.

9.5 EVALUATION OF ALTERNATIVES AND PREFERRED ALTERNATIVE FOR EACH BASIN

Each proposed basin has been evaluated with multiple viable alternatives for storm water management. Alternatives are numbered below:

- 1. Dry and wet detention systems
- 2. Exfiltration system with dry or wet detention systems
- 3. Offsite Ponds
- 4. Joint use Ponds

Basin 1: In Alternative 1, the storm water runoff from the proposed roadway can be treated and attenuated at the linear dry detention swales SWALE 1-1 to SWALE 1-4, dry detention facility DRY POND 1-1 and wet detention facilities WET POND 1-1 and WET POND 1-2 as depicted in the Drainage Map attached in **Appendix C**. Roadside swales, dry ponds and wet ponds provide treatment and attenuation of roadway runoff from station 962+30 to 1041+00, before discharging to the Pinetree Water Control District Canals. The average swale bottom elevation and wet season water table is 11.0 ft. and 10.0 ft. NAVD respectively. Detailed calculations are attached in **Appendix G**.



The offsite FDOT-owned parcel at the NW quadrant of the interchange beyond Sawgrass right-of-way is available, but not included in the stormwater management calculations.

In Alternative 2, Exfiltration Trench is considered to meet water quality requirement along with dry detention or wet detention systems.

Comparing both the alternatives, Alternative 1 is considered as the preferred one since the runoff from this basin in the proposed condition can be managed within the FTE right-of-way without any foreseeable issues, whereas; in alternative 2 it would require significant amount of exfiltration trench to manage stormwater due to moderate to poor percolation rates. Exfiltration trench is known to be a maintenance intensive stormwater management feature. In addition of exfiltration trench will provide little benefit for the basin. Detailed calculations for preferred alternative is attached in **Appendix G**.

Basin 2:

In Alternative 1, storm water runoff from the proposed roadway can be treated and attenuated at the linear dry detention swales SWALE 2-1 to SWALE 2-6, dry detention facility DRY POND 2-1 to DRY POND 2-5 and wet detention facilities WET POND 2-1, as depicted in the Drainage Map attached in **Appendix C.** Roadside swales, dry ponds and wet ponds provide treatment and attenuation of roadway runoff from Station 1041+00 to 1110+00, before discharging to the Cocomar Water Control District Canals. The average swale bottom elevation and wet season water table is 11.0 ft. and 9.5 ft. NAVD, respectively.

In Alternative 2, Exfiltration trench meets water quality requirements along with dry detention or wet detention systems. Comparing both the alternatives, Alternative 1 is considered as the preferred one since runoff from this basin in the proposed condition can be managed within FTE right-of-way without any foreseeable issues, whereas; In alternative 2 it would require significant amount of exfiltration trench due to moderate to poor percolation rates and exfiltration trench is known to be a maintenance intensive stormwater management feature. In addition, exfiltration trench will provide little benefit to reduce the swale storage in this basin. Detailed calculations for the preferred alternative are attached in **Appendix G**.



Basin 3A:

In Alternative 1, storm water runoff from the proposed roadway can be treated and attenuated at dry detention facility DRY POND 2-1 and DRY POND 2-2 and wet detention facilities WET POND 2-1 and WET POND2-2 as depicted in the Drainage Map attached in **Appendix C.** Roadside swales, dry ponds and wet ponds provide treatment and attenuation of roadway runoff from Station 1110+00 to 1180+00, before discharging to the Cocomar Water Control District Canals. The average swale bottom elevation and wet season water table is 11.0 ft. and 9.5 ft. NAVD, respectively.

In Alternative 2, exfiltration trench meets water quality requirements along with dry detention or wet detention systems.

Comparing both the alternatives, Alternative 1 is the preferred one since the runoff from this basin in the proposed condition can be managed within FTE rightof-way without any probable issues, whereas; In alternative 2 it would require significant amount of exfiltration trench to manage stormwater due to moderate to poor percolation rates and exfiltration Trench is a maintenance intensive stormwater management feature. In addition, exfiltration trench will provide little benefit to reduce the required swale storage in this basin. Detailed calculations are included in **Appendix G**.

Basin 3B, 3B-1 and 3B-2:

In Alternative 1, storm water runoff from the proposed roadway can be treated and attenuated at the linear dry detention swales SWALE 3B-1 and SWALE 3B-2, dry detention facility DRY POND 3B-1 to DRY POND 3B-3 and wet detention facilities WET POND 3B-1 to 3B-4, as depicted in the Drainage Map attached in **Appendix C**. The normal water level at the ponds are 8.40 ft. NAVD and minimum berm elevation is 11.90 ft. NAVD.

In Alternative 2, exfiltration trench meets partial or full water quality requirements along with wet detention systems for attenuation purposes.

Alternative 3 is an offsite pond at northeast corner of Turnpike and Wiles Road. This pond site, proposed by FPID 442212-1 will reduce the need for long stormsewer system to the south of Sawgrass Expressway. However, on the negative side, a new parcel of right-of-way will need to be acquired and pond siting process



needs to be carried out duly. The parcel is also contaminated site. Additional permitting efforts are needed to use the parcel for mitigation.

Alternative 4 is also considered to use the Independence Bay Pond for the stormwater needs for the Turnpike segment south of Sawgrass Expressway. This alternative will also reduce the need for long stormsewer line to the south of Sawgrass Expressway. However, on the negative side, it might introduce a complicated permitting process since the Independence Bay pond is privately owned and serves the subdivision at the SE quadrant of the interchange. Extensive coordination with the agencies and stakeholders would be required.

Comparing the alternatives, Alternative 1 is the preferred one since the runoff from this basin in the proposed condition can be managed within FTE right-ofway. Alternative 2 would require a significant amount of exfiltration trench due to moderate to poor percolation rates and exfiltration trench is a maintenance intensive stormwater management feature, In addition, exfiltration trench alone will not provide full treatment and attenuation for the basin. In alternative 3, a new parcel will need to be acquired through duly carried out pond siting process. The process will need to show that the proposed pond at the parcel is the most cost-effective alternative stormwater management of this basin. In alternative 4 rigorous coordination and complicated analysis needs to be done to use the Independent Bay Pond. An agreement with the Home Owners Association (HOA) needs to be reached should the Independence Bay pond be deemed a feasible site.

As per coordination with consultant of project FPID#406150-1 (the FTE south of Wiles Road) consultant, some alternatives are evaluated to provided required permitting stormwater needs. One of the alternatives is joint use of Tradewinds Park & Stable. If agreement fails, the stormwater storage is required within the PD&E Study limit. The study needs to evaluate to accommodate stormwater needs of the project south of Wiles Road.

Basin 3B discharges to BCWCD#2 Canals. Detailed calculations are attached in **Appendix G**.



Basin 4:

In Alternative 1, roadway runoff west of cross-drain at Sta.1203+00 will be directed to the interchange ponds for treatment and attenuation. Due limited right-of-way and longer hydraulic path, larger and deeper conveyance system is required to satisfy hydraulic gradient criteria. Refer to **Appendix G** for hydraulic calculation summary.

Approximately 600 ft east roadway segment between the cross-drain and the Powerline Road will be discharged to the Canal same as discharging in existing conditions. There is no indication of provided of treatment and attenuation of the roadway segment. It seems runoff from the segment sheet flows to roadside swales and discharges into the canal through Ditch Bottom Inlets (DBI) on both sides of roadway.

In Alternative two, exfiltration trench meets water quality requirements along with wet detention systems.

Alternative 3 develops two offsite dry ponds at the south side of the Quite Waters Park. This alternative reduces the need for long stormsewer system and raising of the Powerline Road Intersection. However, on the negative side, the two new parcels of right-of-way need to be acquired from the Quiet Waters Park and a full pond siting evaluation needs to be carried out.

Comparing the alternatives, alternative 1 is considered the preferred one since the runoff from this basin in the proposed condition can be managed within FTE right-of-way without any foreseeable issues, whereas; In alternative 2 it would require significant amount of exfiltration trench due to moderate to poor percolation rates and exfiltration trench is known to be a maintenance intensive stormwater management feature. In addition, exfiltration trench will provide little benefit to reduce the required swale storage.

In alternative 3 the off-site pond will require coordination and duly done pond siting process to acquire the parcel for the wet pond. Basin 4 discharges to BCWCD#2. Detailed calculations are included in **Appendix G**.



10.0 RESULTS

Preliminary drainage evaluations show that the water treatment and attenuation requirements for the approximately 485-acre study area can be provided within the existing Right-of-Way. Refer to **Table 10-1** for a summary of water quality and quantity calculations. The stormwater storage at each facility per basin is summarized in **Table 10-2**. The required volume of treatment and attenuation for the study corridor is exceeded by the provided storage in wet detention ponds in the interchanges or at the FTE owned parcels, re-graded roadside swales and re-graded interchange infield areas. The surplus areas will be utilized for landscaping and beautification purposes. Refer to **Appendix G** for all calculations and supporting documentation. Design aids are attached in **Appendix H**.



	TABLE 10.1 WATER QUALITY AND QUANTITY CALCULATION SUMMARY													
				Basin	Area Calci	ulation			Water Quality Calculation Summary	Water Quantity Calculation Summary	Required Storage For Stormwater Management	Provided Storage	Surplus Volume	
	Project Basin	Station		Side	Length	Total Area	Impervious Area		Water Quality Treatment Req'd	Net Runoff Increase (Post-Pre)	Water Quality+Water Quantity	(4)	(5)	Remarks
									(1)	(2)	(3)			
No.	Description	From	То	LT/RT	(ft.)	(Ac.)	Exist.	Prop.	(Ac.ft.)	(Ac.ft.)	(Ac.ft.)	(Ac.ft.)	(Ac.ft.)	
1	From West of US-441 to US- 441	962+30	1041+00	LT/RT	7870	110.64	45.33	86.76	13.45	6.29	19.74	24.25	4.51	Dry Detention and Wet Detention
2	From US-441 to East of Lyons Road	1041+00	1110+00	LT/RT	6900	85.88	40.29	64.08	12.04	3.54	15.58	15.65	0.07	Dry Detention and Wet Detention
3A	From East of Lyons Road to FTE	1135+00	1153+00	LT/RT	1800	22.37	12.30	14.92	2.66	0.42	3.08	5.58	2.50	Dry Detention and Wet Detention
3B	Sawgrass Expressway from FTE to Waterways Blvd.	1110+00	1180+00	LT/RT	7000	158.37	68.10	98.91	14.46	4.67	19.13	48.11	28.98	Wet Detention. Required storage for Basin 4 is added in Required Storage
3B-1	FTE from north of the Sawgrass/FTE interchange to Hilsboro Canal	5652+38	5716+70	LT/RT	6432	44.27	18.66	30.89	6.44	1.82	8.25	2.40	-5.85	Dry Detention along right side
3B-2	Tumpike from Wiles Road to Sawgrass/FTE interchange	5572+50	5622+10	LT/RT	4960	36.01	0.00	24.39	5.08	3.71	8.79	4.90	-3.89	Dry Detention along right side
4	From Waterways Blvd. to Powerline Rd.	1180+00	1209+00	LT/RT	2900	17.80	0.00	20.47	3.16	18.55	21.71	0.00	-21.71	Wet Detention. Provided storage is Basin 3B
										Total 3A, 3B, 3B-1 & 4	60.96	60.99	0.03	

Table 10.1 - Water Quality and Quantity Calculation Summary

Notes:

(1) Water Quality Treatment required = Wet detention volume calculated for the first inch of runoff from the developed project, or the total runoff of 2.5 inches times the total impervious area, whichever is greater. See separate spreadsheet for detail calculation.

(2) Water Quantity required = Net Runoff generated from Design Storm (25YR-72HR) = Post Development Runoff - Pre Development Runoff. See separate spreadsheets for detail calculation.

(3) Required Storage for Stormwater management (Water Quality + Water Quantity) = (1) + (2)

(4) From Table 2 Wet and Dry Detention System Storage Calculation

(5) Surplus Volume (5) = (4) -(3). Landscaping needs can be fulfilled at the area where surplus volume is available.



							CONTROL/BOTT		MENT AND	6700405	
BASIN	WATER MANAGEMENT FACILITIES						OM EL	ATTENUATION EL		STORAGE	REMARKS
No.	DESCRIPTION	From Sta.	To Sta.	Side	TREATMENT METHOD	Elev. (ft.)	Area (Ac.)	Elev (ft.)	Area (Ac.)	Volume (Ac-ft)	Minimum Berr EL
	Swale 1-1	962+50	1012+00	LT	Dry Detention	11	0.57	12.5	0.66	0.92	14.50
	Swale 1-2	962+50	1009+00	RT	Dry Detention	11	1.71	12.5	1.86	2.68	13.50
	Swale 1-3	1023+50	1030+50	LT	Dry Detention	11	0.39	12.5	0.47	0.64	13.50
	Swale 1-4	1028+00	1040+00	RT	Dry Detention	11	0.43	12.5	0.51	0.70	13.50
1	Dry Pond 1-1	1030+00	1035+00	LT	Dry Detention	11	1.19	12.5	1.32	1.88	13.50
	Mat Dand 1.1	4020.00	4020.00	1.7	Wet Detention	10	7 7 2	- í	Detention =	6.83	
	Wet Pond 1-1 Wet Pond 1-2	1030+00 1035+00	1039+00 1040+00	LT LT	Wet Detention	10 10	7.73 3.61	11.5 11.5	8.05 3.83	11.84 5.58	14.50 14.50
	Wet Folid 1-2	1033+00	1040+00		Wet Detention	10	3.01		Detention =	17.42	14.50
				OTAL AVAILABLE STORAGE AT BASIN 1 =				24.25 AC-FT			
	Dry Pond 2-1	1042+00	1043+00	RT	Dry Detention	11	0.21	12.5	0.27	0.36	13.50
	Swale 2-1	1042+00	1046+00	RT	Dry Detention	11	0.23	12.5	0.32	0.41	13.20
	Swale 2-2	1042+50	1058+50	LT	Dry Detention	11	0.65	12.5	0.75	1.05	13.20
	Swale 2-3	1047+00	1052+50	RT	Dry Detention	11	0.20	12.5	0.25	0.34	13.20
	Dry Pond 2-2	1049+00	1053+00	RT	Dry Detention	11	0.99	12.5	1.11	1.57	13.45
	Swale 2-4	1076+00	1091+00	RT	Dry Detention	11	0.41	12.5	0.49	0.67	13.20
	Swale 2-5	1077+00	1083+00	LT	Dry Detention	11	0.09	12.5	0.13	0.16	13.45
2	Swale 2-6	1082+50	1092+50	RT	Dry Detention	11	0.32	12.5	0.39	0.53	13.45
	Dry Pond 2-3	1090+50	1092+50	RT	Dry Detention	11	0.21	12.5	0.27	0.36	13.45
	Dry Pond 2-4	1095+00	1097+00	LT	Dry Detention	11	0.17	12.5	0.22	0.29	13.45
	Dry Pond 2-5	1095+00	1098+00	LT	Dry Detention	11	0.13	12.5	0.17	0.23	13.45
	Mat David 2.4	4042.50	4040.50	DT	Web Debendier		6.20	<u> </u>	Detention =	5.98	42.05
	Wet Pond 2-1	1042+50	1048+50	RT	Wet Detention	9.5	6.30	11.0	6.59 Detention =	9.67 <i>9.67</i>	12.95
						тот					15.65 AC-FT
	Wet Pond 3A-1	1147+00	1151+00	LT	Wet Detention	8.4	1.21	9.9	1.27	1.86	12.95
	Wet Pond 3A-2	1149+00	1151+00	LT	Wet Detention	8.4	0.86	9.9	0.91	1.33	12.95
				-				, ,	Detention =	3.19	
3A	Dry Pond 3A-1	1136+00	1143+00	LT	Dry Detention	11	0.64	12.5	0.69	1.00	13.45
	Dry Pond 3A-2	1145+00	1150+00	LT	Dry Detention	11	0.90	12.5	0.96	1.39	13.45
		•						ΣDry	Detention =	2.39	
						тот	AL AVAILA	BLE ST	ORAGE AT	BASIN 3A =	5.58 AC-FT
	Wet Pond 3B-1	1147+50	1151+00	RT	Wet Detention	8.4	0.85	9.9	0.90	1.32	11.9
	Wet Pond 3B-2	1155+00	1162+50	LT	Wet Detention	8.4	6.08	9.9	6.22	9.23	11.9
	Wet Pond 3B-3	1155+00	1159+00	RT	Wet Detention	8.4	12.90	9.9	13.11	19.51	11.9
	Wet Pond 3B-4	1156+00	1163+00	LT	Wet Detention	8.4	2.09	9.9	2.17	3.20	11.9
3B	Wet Pond 3B-5	1155+00	1163+00	RT	Wet Detention	8.4	7.59	9.9	7.75 Detention =	11.50	11.9
30	Dry Pond 3B-1	1090+50	1092+50	RT	Dry Detention	9.4	0.61	10.9	0.70	44.75 0.98	11.90
	Dry Pond 3B-2	1090+50	1092+50	RT	Dry Detention	9.4	0.49	10.9	0.57	0.38	11.90
	Dry Pond 3B-3	1090+50	1092+50	RT	Dry Detention	9.4	0.99	10.9	1.11	1.57	11.90
						1		_	Detention =	3.36	
						тоти	AL AVAILA	BLE ST	ORAGE AT		48.11 AC-FT
3B-1	Swale 3B-1	5651+00	5678+00	RT	Dry Detention	9.4	1.53	10.9	1.68	2.40	11.90
30-1						TOTA	L AVAILAI	BLE STO	ORAGE AT I	BASIN 3B-1	= 2.40 AC-FT
3B-2	Swale 3B-2	5577+00	5609+00	RT	Dry Detention	9.4	3.16	10.9	3.37	4.90	11.90
						ΤΟΤΑ		BLE STO	ORAGE AT I	BASIN 3B-2	= 4.90 AC-FT
Notes:											
					iated Drainage Map		a dway dag	an 600	acconiated	whihit	
					ting plans and conce ales or enlarging inte			ы эее		ANIDIC	
			•		Conversion factor,	-	•	L - 1.529	9'		
	Storage Volume C	alculations in	n Dry/Wet Po	ond							
	\searrow				/						
					B = Ar	ea @ Tr	eatment +	Attenua	ation EL		
	= depth		Storage Volu	ume				\vdash			
		1.									
		(trea	tment + atte	nuatio	A = Area @ Bot						

Table 10.2 - Wet and Dry Detention System Storage Calculation



The preliminary base clearance evaluation was done based on the original roadway profile, Typical Sections and pavement design. The January 2019 FTE Drainage Manual Supplement, Section 5.4.1.1 requires the design high water for base clearance to be set above the outfall weir elevation. If the above criteria cannot be achieved in some roadway stretches, the base clearance could be evaluated, in a case-by-case basis, from the swale bottom elevation or from the SHGWT. Detailed base clearance should be evaluated during final design.



11.0 CONCLUSIONS

The current Sawgrass Expressway corridor is permitted to eight lane section; however, the proposed improvement will eliminate roadside swales. Therefore, a complete preliminary storm water management calculation was done to meet current SFWMD criteria where interchange ponds will be used as stormwater management facilities and several FDOT parcels are available for storm water management use. The calculation shows adequate storage is available at those facilities to meet both FDOT and SFWMD criteria. Long stormsewer conveyance systems are expected in the detailed design. No Right-of-Way acquisition is deemed necessary.



12.0 REFERENCES

SFWMD PERMIT No. 06-00629-S SFWMD PERMIT No. 06-10034-S FDOT DRAINAGE MANUAL (DM) 2023 FDOT Florida Design Manual (FDM) 2023 ERP APPLICANT'S HANDBOOK VOLUME I. FDEP December 2020 ERP APPLICANT'S HANDBOOK VOLUME II. SFWMD May 2016 B.M.P. FOR SOUTH FLORIDA URBAN STORMWATER MANAGEMENT SYSTEMS. SFWMD September 2020 PD&E MANUAL. FDOT June 2023 USDA-NRCS SOIL SURVEY OF BROWARD COUNTY, FL FEMA FLOOD INSURANCE RATE MAPS





Correspondence and Meeting Notes





Coordination with Broward Water Control District #2 & Cocomar Water Control District

Sawgrass Expressway (SR 869) Widening PD&E Study From South of US 441/SR 7 (MP 18.0) to Powerline Road (MP 22.0) Broward County, Florida FPID# 437153-1-22-01 / Contract Number C-9P63

<u>Monday, February 12, 2018</u> 1:30 PM – 2:30 PM Broward Co. Water and Wastewater Services/Water Management Division 2555 West Copans Road Pompano Beach, Fl. 33069

Meeting Minutes

Introductions

Mike Ciscar: We are doing a PD&E Study of the Sawgrass Expressway from just West of US-441 to Turnpike East, to complete the PD&E Study from Sunrise Rd. up to US-441. This is the remaining segment of the corridor from West of SR-441 to the mainline and ultimately down to Powerline Road. The main objective is to continue/complete the Express lanes network, to put Express lanes in the median along the Sawgrass Expressway as well as Interchange modifications necessary at US-441 and Lyons Road. The Mainline Interchange between the Sawgrass and the Turnpike it's a partial interchange from the North to the West as part of this study we are introducing the missing movements running from the East and also Direct Connects Express Lanes connections from the proposed express lanes to the main line because Turnpike also is going to be implementing Express lines on the mainline as well as the Sawgrass. That's the overall scope of our project. One very important component of it, is coordination with other adjacent projects. The Turnpike has projects like I mentioned on the Sawgrass to the west of this job and then on the main line north and south of this job and on the East district 4 (FDOT) is also conducting a PD&E Study along Southwest 10th Street, looking at several options there, one of which is a depressed option. So we have been coordinating closely with them on a bi weekly basis on that. But that's not part of our scope. Our scope is to coordinate with them and tie into them were then tie into us, make a seamless transition. But our objective is express lanes on the Sawgrass and then a mainline Interchange as well as the two service in general.

Mohammad Pervez: For drainage purposes for the corridor we researched the permits and the existing as-built plans. We are going through the basins, where those are, and how it is behaving. From the beginning of the project up to US-441 which is under Pine Tree Water Control District. Then from US-441 it's actually a genuine divide to Turnpike, is under Cocomar Water Control District. From Turnpike to the end of the project it under Broward County Water Control District No. 2. Turnpike interchange it is connected, meaning that both sides of the Turnpike are connected. Is there any physical divide between a Cocomar and Broward County Water Control District No. 2? Outfall 82 on the South East corner discharges to the Broward County Water Control District Canals and Outfall 84 North West Corner discharges to Cocomar Water Control District.

<u>Carl Archie:</u> A connection not included in SFWMD Permits shows a connection between Coco Lakes and the wet pond located SW of the turnpike interchange. Cross culvert No. 9 connects West and East of the Turnpike, which eventually discharges to a lake west of Independence Bay Community. We've always been concerned with the connectivity of these lakes. There were missing pipes, there was a whole lot





missing infrastructure that was never put in. Some of the Sawgrass was over dredged, some was under dredged and there are missing piece. For example outfall 85, we know it was never approved by the adjacent land owner. They used to be a lake and they never obtained an easement to drain. When they developed Independence Bay, the owner got permission to fill in that lake or to relocate it. The pipe goes nowhere because the lake was moved. I think the Sawgrass relocated the outfall between Waterways Blvd. and Independence Dr., you'll need to check our files for that. That's how the system goes. It's a whole series of four separate plans. The lakes (NE and SE of US-441, Cross Culvert No. 3) they were always a problem for us in the county since we need to get the across the states and divide and we regularly use this space and the light to get from over here to over here. There is a connection between these two lakes here,

Mohammad Pervez: I assume those were part of the development plans.

<u>Carl Archie:</u> The Lake (SE of US-441) was used by the Sawgrass, but the storage is owned by a development (NE of US-441). NE Lake is owned by the FDOT now. North and South Lakes are connected. There is an additional outfall underneath Sawgrass that connects North and South Lakes (West of Cross Culvert No. 3).

Mohammad Pervez: So as far as the storm management goes, seems like is mixed by the districts.

<u>Carl Archie:</u> The Cocomar plans were developed way before the Sawgrass Authority developed theirs. We had to have a north-south connectivity so pretty much. The Sawgrass was just in case the secondary user for the entire of things. And back then I think was Broward County was the developing the Sawgrass

<u>Anaily Padron:</u> Before you continue I have a question regarding Cross Culvert No. 4. Per BCPA all the outfalls located in Cocomar WCD are property of the City or County, except for Cross Culvert No. 4. It looks like it is property of Winston Park.

<u>Carl Archie</u>: This development was all coming in at one time and they needed the service. So they installed it.

Susan Juncosa: It's ours.

<u>Mohammad Pervez:</u> For stormwater management purposes, is there a separation a physical separation between East and West of the Turnpike?

<u>Carl Archie:</u> There is a physical divide that's about ten feet from the Turnpike property. The reason for suspicion at the lakes, they just doesn't look alike, it looks like that lake is isolated with the pipes were never maintained.

Mohammad Pervez: We will treat the interchange as a whole, meaning that in that interchange footprint would probably going to bring it in, in those two ponds and whatever the swales we get in between. And then finally we'll find the outfall, fall through the canal. Is it going to be OK to look at one interchange drainage?

<u>Carl Archie</u>: Yes. The divider is outside your right of way in the presumption it is closed. You can presume that the elevation 10.0 NGVD. That was the control elevation for that pond, but it was intended to be a basin divide that we can point.

Fred Gaines: Going back to the control structure. Where does it outfall? Does it the outfall on to the lake?





<u>Carl Archie:</u> It actually followed the fence line and goes into this lake then under the turnpike and then is actually a canal, this was supposed to be an off ramp and way back when they pick it up and that's the outfall to the east. It is piped from the Turnpike and it becomes an open canal.

<u>Ryan Solis-Rios:</u> The northern boundaries of the turnpike are actually changing. Right now the limits ends at the Hillsboro Blvd. We just recently got notice that we have to evaluate another PD&E that's actually 4,000 ft more from the County line. We're basically adding two express lanes of Sawgrass Expressway and two express lanes to FTE, just two lanes only ran into what's out there today and actually there's going to be an additional lane (auxiliary lane). Within the interchange area, we basically have four more lanes.

Mohammad Pervez: As far as the stormwater management goes, Mike already described the roadway improvements in the drainage part or a storm water management part. We would be providing a detention system in the interchanges and we'd be probably providing dry detention or retention in some places. We're explore if there is any way we can provide exfiltration trench. We have the Geotechnical Report and a few other things to do with all those options. We will provide a stormwater treatment attenuation within the limits of the project. So that's of our proposed improvement. For the stormwater management criteria as we always do we will be following south Florida water management district criteria for water quality and water quantity. We know that does project was permitted for 8-lanes, if we can have that storage for eighth lane permitted and then provide for the additional improvement and also analyze to see whether that is beneficial or all the water quality before entire project. And then we would provide the option for the project. For water quantity, we'll analyze Pre VS Post. Post development discharge will be less than Predevelopment discharge without affecting any of the downstream properties.

Susan Juncosa: What would you be considering the project area?

<u>Fred Gaines:</u> For now we will consider from ROW to ROW. May be less, but I don't think there's that much new additional ROW they take them for the most part.

<u>Ryan Solis-Rios:</u> We may impacting the Park on both sides (additional ROW) on FTE and SW 10th St. corridor, but everything else in the project we are basically using the existing ROW.

<u>Carl Archie</u>: We had some plans associated to get more conveyance across FTE South of Sawgrass. I'll have to discuss it with John. In fact, we were planning this whole idea of getting water from the space in addition to independence bay.

Fred Gains: Is that the same area you and Jennifer Hereto talk about in 2006? We're not constructing but it is still an option if you wanted to do that anytime.

<u>Carl Archie:</u> Yes. Broward County Commissioners had some questions about that project and the Turnpike decided to just step back from the design. Secondly, we will have to see the intent with our wellfileld folks, about the lake and area NE FTE Interchange. The Ski Lake and the beach it's not supposed to be mixed with stormwater because of human contact, and so we have always wanted to surround this area with the water from over here, very close, 10 feet in some cases. There were plans associated with getting water over here. I'll get with our well field folks to see if they want to pursue it.

<u>Anaily Padron</u>: I have a question regarding the pipe connections inside Quiet Waters Park from this sketch from the ERP Permit. Looks like all the lakes are interconnected with equalizer pipes and they eventually discharge to C-3 Canal and that ultimately discharges to the Hillsboro Canal via a Control Structure.





Carl Archie: This was never put in place and everyone was very glad that was never put in place, it was installed far too high to be effective. We are not allowed to put water there. That option never came through. The System in the East is so depressed that they never had a flood issue. We have to talk to our wellfield folks, they desperately want to protect the well field. The best idea is to fill the surrounding lakes with surface water.

Fred Gains: All the culverts that are in your system are they free flowing, there are no gates underneath?

Carl Archie: Yes, at least in the Cocomar District.

<u>Anaily Padron:</u> We have no recent permits for SW 10th Street, we located the crossing pipes bases on the Survey and old permits.

Mohammad Pervez: Moving on with the Cross Culvert Extension. The only culvert extension that we have that might be outside FTE ROW is the one connecting the C-3 Canal. In that case do we need to submit permits?

Carl Archie: Yes. If you are impacting and the structure needs to be replaced.

Fred Gains: Do you want to move to question number five?

<u>Mohammad Pervez:</u> So challenging. You know, we don't have much room for stormwater in our area. I'm wondering, is there any storage available in the System that we can use or Cocomar needs more water?

<u>Carl Archie:</u> The district has no surplus left. All of it was sold out. I will need to contact Susan Bodman, Jennifer Hereto (Hydro geologists). I will let them know and we can plan another meeting with them.

Ryan Solis-Rios: You said that you had plans convey stormwater West-East of the FTE Mainline.

<u>Fred Gains:</u> We have done this, as you all know. We did it with Winding Waters, Palm Beach County. There are several instances when folks want stormwater runoff and it's relatively clean.

Mohammad Pervez: Can we treat FTE as a whole basin?

Carl Archie: You'll do it for both, the system are mixed. I suppose you can do it.

Fred Gains: We will make sure that all our coordination's will have NAVD, NGVD references.

Meeting Adjourned: 2:30 PM

Action Items:

- As-Builts from FDOT/FTE required by Susan Juncosa.
- Response from Well field office
- Meeting with Broward County Hydro geologist

Please see sign in sheet for attendance.





Coordination Meeting with South Florida Water Management District Sawgrass Expressway (SR 869) Widening PD&E Study From South of US 441/SR 7 (MP 18.0) to Powerline Road (MP 22.0) Broward County, Florida FPID# 437153-1-22-01 / Contract Number C-9P63

> <u>Thursday, August 2, 2018</u> 10:00 AM – 10:30 AM SFWMD District Headquarter 3301 Gun Club Road West Palm Beach, FL 33406

MINUTES

1. Introduction to the Project

a. Objective and Schedule

The meeting was held to discuss scope of the project and pertinent South Florida Water Management District (SFWMD) drainage design criteria for the project. Fred Gaines opened meeting with a brief description of the project. Mr. Solis-Rios added more project information with discussing the project limits, importance of project and other surrounding FDOT and Turnpike projects within the vicinity of project area.

2. Existing Stormwater Management

a. Local Drainage Districts

There are three local drainage districts with in the project limits. From the beginning of the project to US 441 is within Pine Tree water Control District, from US 441 to FL Turnpike is within Cocomar Water Control District and from FL Turnpike to Powerline Road is within Broward County Water Control District #2.

b. Existing Permits and Pre-development Drainage for this Project

Mr. Pervez mentioned that there is an existing permit approved in 1985 for 8 lane section of the road within the project limit but only 4 lane section was built at that time. In the year 2005, 2 more lanes were added at the median with a letter modification. Mr. Pervez mentioned that the proposed project will have 10 to 12 lane of roadway and there are significant amount of existing roadside swales will be lost due to widening or reconstruction.

3. Stormwater Management Criteria

a. Water Quality and Water Quantity Criteria

Mr. Pervez mentioned that the design team would like to consider providing the storage (current volume) plus the required volume for the treatment of additional impervious area as an option to meet the water quality requirement for the project. The other option is to provide the water quality for entire roadway. Mr. De Rojas from SFWMD explained that since most roadside swale will be used for widening of the road, it is fair to use new permit with current design criteria (1" over the project area or 2.5" over impervious area, whichever the greater) for water quality treatment for new roadway. However, during the final design phase if the project finds that it is beneficial to use previous 8 lane section volume plus the additional volume for additional impervious area, SFWMD would like to evaluate the option at that time.

Mr. Gaines inquired if Independence Bay Ponds can be used for treatment and attenuation to avoid buying additional right of way. *Mr.* De Rojas replied that it depends on local drainage authority.





4. Proposed Improvement

a. Stormwater Management - Post-development

Basins – There are primarily four basins; (1) from beginning of the project to US 441, (2) from US 441 to east of Lyons Road, (3) from east of Lyons Road to waterways Blvd and (4) from waterways Blvd to Powerline Road.

Stormwater Management Options – *stormwater management will be primarily provided by wet detention in the interchange ponds and dry detention in the roadside swales and interchange infield areas.*

5. Cross-Culverts Extensions in WCD's Facilities

a. Culverts and jurisdiction – There are several cross-culverts within the project needs to be extended which is under local drainage district jurisdiction

6. Regional Stormwater Management Opportunities

- a. Potential Wellfield Recharge Activity and Well Wellfield Protection Zones Mr. Pervez mentioned that the project team is coordinating with Local Drainage Districts about to design stormwater system that could help well field recharge located north-east of Turnpike and Sawgrass Expressway interchange. Mr. De Rojas mentioned Department of Environmental Protection (DEP) needs to be contacted in case well field recharge is involved.
- b. Regional Water Quality and Water Quality Basin, Quiet Waters Park and Independence Bay Community – Mr. Rojas mentioned that Cocomar WCD has control structures at the system that regulates the flow to the Hillsboro Canal but BCWCD#2 does not have any defined system. Permitting at BCWCD#2 was delegated to the County

7. Environmental Features and Approach

Rob Myers described the environmental features within the project corridor. They include a bald eagle nest in the Northeast quadrant of the intersection of Sawgrass Expressway and Florida's Turnpike. Mr. Myers stated that nest was active during the 2017/2018 nesting season and produced one chick. Mr. Myers pointed out that the nest is in a different tree than during previous years, but it is still on FDOT right-of-way. Other environmental features within the project area included wetlands, and Mr. Myers noted that wetlands occur on Quiet Waters Park, immediately north of Sawgrass Expressway and east of Florida's turnpike. The SFWMD noted that some of these lakes and wetlands appear to have been cut into uplands and are not natural wetlands. Wetlands and surface waters are also potentially suitable foraging habitat for the federally listed wood stork. Mr. Myers noted that potentially contaminated sites occur in the project area, included several gas stations and a landfill near the southern terminus along Florida's Turnpike.

8. Other

Mr. Pervez inquired if Hillsboro Aquifer Storage and Recovery, a Comprehensive Everglades Restoration Plan (CERP) project will be impacted by the project. SFWMD staff mentioned that CERP is little far north side of project, therefore it may not affect the project.

9. Closing Comments/Questions – Action Items

Meeting was concluded at 11:30 am on Thursday, August 3, 2018.





SFWMD COORDINATION MEETING

Sawgrass Expressway (SR 869) Widening PD&E Study From South of US 441/SR 7 (MP 18.0) to Powerline Road (MP 22.0) Broward County, Florida FPID# 437153-1-22-01 / Contract Number C-9P63

> <u>Thursday, August 2, 2018</u> 10:00 AM – 11:00 AM

SIGN IN SHEET

NAME	AGENCY / COMPANY	TELEPHONE	EMAIL	INITIALS
1) Barb Conmy	SFWMis	561-682-6937	beonmy Ostimed. 900	BC
2) Carlos de Rojas	SFWHD	561-682-6505	cderojas asfumd.gou	A
3) Martin Horwitz	FTE/EMO	407-264-3022	martinhorwitz ado 7. stakes	7.05 Mf
4) Fred Gaines	Atlens	407 264 3689	fred gaines O dot state Clus	Rg
5) hypon Solis-Rio)	Corredino	254-M-6044	robis-vios @ cowedow ion	RIK
6) Mohemmad Perva	HDR	305-728-7446	mohammad, perveze harine com	Sy.
7) Imtyaz Sharkh	HDR	305-728-7434	mtyaz.shaikhehdrinco	n Is
8)				
9)				
10)				
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15)				
16)				





Coordination with Broward Water Control District #2 and Cocomar Water Control District

Sawgrass Expressway (SR 869) Widening PD&E Study From South of US 441/SR 7 (MP 18.0) to Powerline Road (MP 22.0) Broward County, Florida FPID# 437153-1-22-01 / Contract Number C-9P63

<u>Thursday, August 2, 2018</u>2:00 PM – 3:00 PMBroward Co. Water and Wastewater Services/Water Management Division2555 West Copans Road Pompano Beach, Fl. 33069

AGENDA

1. Update on Project

- This is a follow-up meeting with Broward County Water Control District#2 to provide an update on the project and to discuss the opportunity to recharge the wellfields.
- Fred Gaines opened the meeting with a brief description of the project. Mr. Solis-Rios added more project information with discussing the project limits, importance of project and other surrounding FDOT and FL Turnpike projects within the vicinity of project area.
- 2. Update on Stormwater Management Approach
- 1. Proposed Basins- Mohammad Pervez provided an update on the drainage design approach of the project since last meeting.
- 3. Update on Stormwater Management Opportunities
- 2. Wellfield Recharge Possibility Ponds at Quiet Waters Park
 - Broward County Staff mentioned that the ground water flows from south to north through the wellfield zone on the east side of FL Turnpike. Therefore, Broward County has been trying to send water to the south side of Sawgrass Express way and east side of FL Turnpike. Mr. Archie mentioned to have a possible pipe connection under the FL Turnpike to bring water from west to east just north of Wiles Road towards well fields. Mr. Gaines mentioned that the crossing could be a cost sharing opportunity for both FL Turnpike and BCWCD#2 for mutual benefits.
 - Broward County staff mentioned that they do not have any issues on the location of the new ponds in Basin 4 inside the quite waters park. Since the parcels are owned by Broward County Parks and Recreational Department, the staff advised to contact the Parks department. They also mentioned that Quiet Waters Park ponds cannot be used for discharge because of human contact in the water (class I water).





a. Project Discharge in Independence Bay Pond – Compensatory Discharge Opportunity

Carl Archie informed that BCWCD#2 has no objection on sending runoff from Sawgrass FL Turnpike interchange to Independence Bay through the current outfall at SE corner of the interchange. He also mentioned that the existing Sawgrass Express way east of FL Turnpike is also discharging to Independence bay. So Discharge for that part will also be allowed.

b. Potential Alternative Pond at NW Corner of Turnpike and Wiles Road

Regarding the potential offsite pond at the SE corner of FL Turnpike and Wiles Road, Mr. Archie suggested to relocate the pond footprint further north so that it falls in the path of the proposed crossing from Coco Lakes to Independence Bay.

c. Potential Overflow Discharge at Independence Bay from Basin 3B

Broward County staff mentioned to use SFWMD criteria for quality and quantity to release stormwater at Independence Bay. Carl Archie informed that any additional runoff going to the Independence Bay Pond will be subject to a potential Broward County permit. He suggested to have a well-established connection among proposed ponds with lower invert elevation at the pipes to help water to flow towards south of Sawgrass Expressway which may help recharging the well field. Mr. Archie promised to send the legal agreement between the community and the county for receiving water to the lake.

4. Other Items

Broward County staff Ms. Maran suggested to use new Broward County Ground Water Elevation Maps for the design purposes.

Meeting was concluded at 3:30 am on Thursday, August 3, 2018.





COORDINATION WITH BROWARD WATER CONTROL DISTRICT #2 AND COCOMAR WATER CONTROL DISTRICT

Sawgrass Expressway (SR 869) Widening PD&E Study From South of US 441/SR 7 (MP 18.0) to Powerline Road (MP 22.0) Broward County, Florida FPID# 437153-1-22-01 / Contract Number C-9P63

Thursday, August 2, 2018

2:00 PM – 3:00 PM

Broward County Water and Wastewater Services/Water Management Division 2555 West Copans Road Pompano Beach, FL 33069

	NAME	AGENCY / COMPANY	TELEPHONE	EMAIL	INITIALS
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2)	Mike Ciscar	Corradino	(954) 777-0044	MCiscar@corradino.com	
3)	Ryan Solis-Rios	Corradino	(954) 777-0044	rsolis-rios@corradino.com	PSR
4)	Anaily Padron	HDR	(305) 728-7459	Anaily.padron@hdrinc.com	
5)	Mohammad Pervez	HDR	(305) 728-7446	Mohammad.Pervez@hdrinc.com	201
H6)	Martin Horwitz	FTE	(407) 264-3022	Martin.Horwitz@dot.state.fl.us	AH
7)	Erin Yao	FTE	(407) 264-3479	Erin.Yao@dot.state.fl.us	10
8)	Kevin Stewart by phone	FTE/Atkins	(407) 264-3417	Kevin.Stewart@dot.state.fl.us	
9)	Abra Horne	FTE/HNTB	(407) 264-3019	Abra.Horne@dot.state.fl.us	
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17	+) Carolin Mar	an Broward	(954) 519035	56 Cmaran@broward org	. H

SIGN IN SHEET

-1-

From: To:	<u>Ryan Solis-Rios</u> Shaikh, Imtyaz; Lopez, Carlos J.
Subject:	FW: 437153-1 PD&E Widen Sawgrass US441 to Powerline
Date:	Friday, October 6, 2023 9:35:43 AM
Attachments:	image001.png image002.png
Importance:	High

CAUTION: [EXTERNAL] This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Hi Guys:

See below from Turnpike. Please confirm. Thanks,

 From: Stewart, Kevin <Kevin.Stewart@dot.state.fl.us>

 Sent: Friday, October 6, 2023 9:14 AM

 To: Heywood, Jazlyn <Jazlyn.Heywood@dot.state.fl.us>; Ryan Solis-Rios <rsolis-rios@CORRADINO.com>

 Cc: Yao, Erin <Erin.Yao@dot.state.fl.us>; Gaines, Fred <Fred.Gaines@dot.state.fl.us>

 Subject: 437153-1 PD&E Widen Sawgrass US441 to Powerline

Ryan/Jazlyn,

An RAI comment recently came up on the widening project just east of this project. The Sawgrass (west of Turnpike mainline only) is within a WPA (special basin) which has some additional requirements such as an additional 50% water quality volume (for the extra lanes above 8 lanes that were previously permitted). I don't believe this has been accounted for in the volumes for this project. I wanted to pass this information along so it can be included. Thanks! See the SFWMD SWERP manual at erp_swerp_manual.pdf (sfwmd.gov).



Senior Drainage Engineer AtkinsRéalis Engineering, Design and Project Management

Florida's Turnpike Milepost 263, Building 5315, Ocoee, Florida 34761 Tel: 407-264-3417 Mob: 407-448-3616 PLEASE NOTE THAT FLORIDA HAS A BROAD PUBLIC RECORDS LAW, AND THAT ALL CORRESPONDENCE TO ME VIA E-MAIL MAY BE SUBJECT TO DISCLOSURE.



APPENDIX B

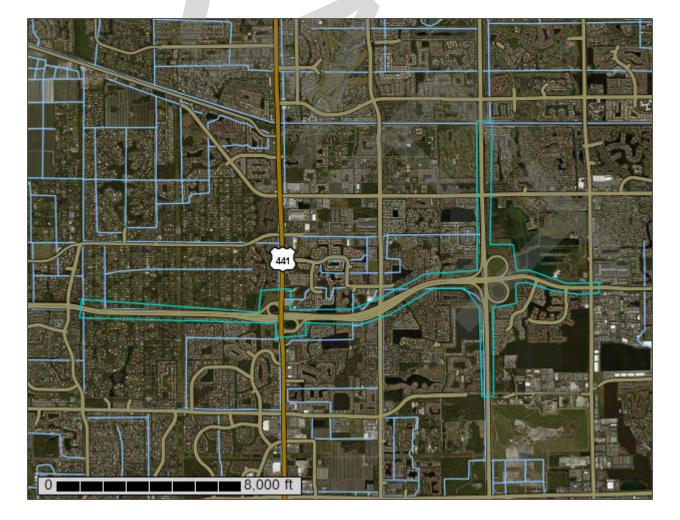
Soil Data & Geotechnical Information



United States Department of Agriculture

Natural

Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Broward County, Florida, East Part; and Palm Beach County Area, Florida



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

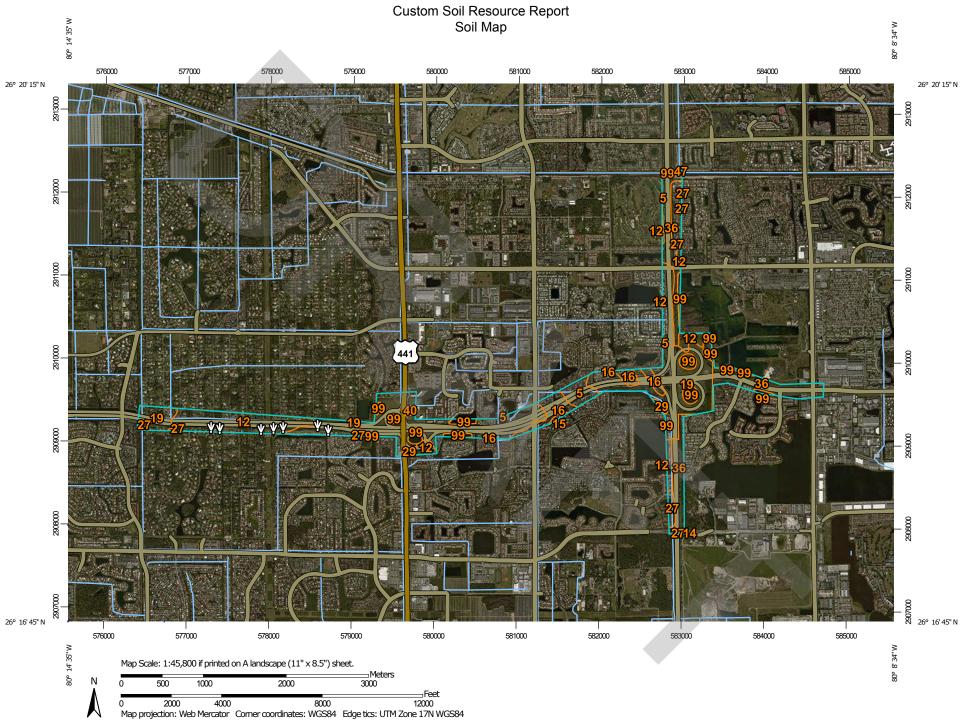
Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

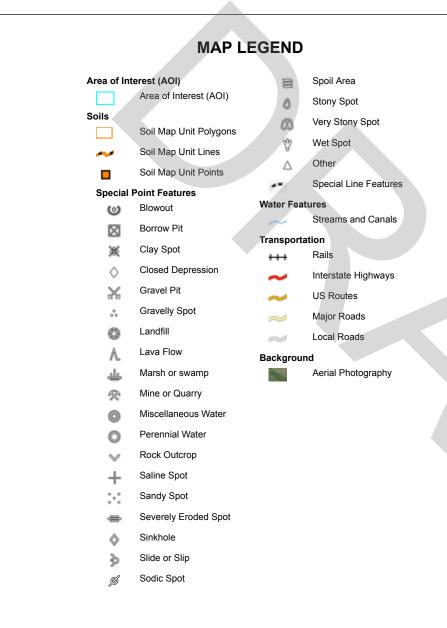
After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.





MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:20,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Broward County, Florida, East Part Survey Area Data: Version 14, Sep 17, 2018

Soil Survey Area: Palm Beach County Area, Florida Survey Area Data: Version 14, Sep 17, 2018

Your area of interest (AOI) includes more than one soil survey area. These survey areas may have been mapped at different scales, with a different land use in mind, at different times, or at different levels of detail. This may result in map unit symbols, soil properties, and interpretations that do not completely agree across soil survey area boundaries.

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Dec 17, 2014—Feb 11, 2015

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background

MAP LEGEND

MAP INFORMATION

imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
5	Boca fine sand, 0 to 2 percent slopes	24.6	2.9%
12	Hallandale fine sand, 0 to 2 percent slopes	180.1	21.1%
14	Matlacha gravelly fine sand, limestone substratum	0.3	0.0%
15	Immokalee fine sand, 0 to 2 percent slopes	2.3	0.3%
16	Immokalee, limestone substratum-Urban land complex	46.9	5.5%
19	Margate fine sand, occasionally ponded, 0 to 1 percent slopes	371.4	43.5%
27	Plantation muck	15.5	1.8%
29	Pompano fine sand, 0 to 2 percent slopes	7.2	0.8%
36	Udorthents	153.5	18.0%
40	Urban land, 0 to 2 percent slopes	3.2	0.4%
99	Water	46.6	5.5%
Subtotals for Soil Survey A	rea	851.6	99.7%
Totals for Area of Interest		854.2	100.0%

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
47	Udorthents, 2 to 35 percent slopes	1.9	0.2%
99	Water	0.7	0.1%
Subtotals for Soil Survey Ar	ea	2.6	0.3%
Totals for Area of Interest		854.2	100.0%
Map U	nit Description	S	

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the

characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered

practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Broward County, Florida, East Part

5—Boca fine sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2svz9 Elevation: 0 to 60 feet Mean annual precipitation: 42 to 56 inches Mean annual air temperature: 70 to 77 degrees F Frost-free period: 350 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Boca and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Boca

Setting

Landform: Drainageways on marine terraces, flats on marine terraces Landform position (three-dimensional): Tread, dip, talf Down-slope shape: Linear, convex Across-slope shape: Concave, linear Parent material: Sandy and loamy marine deposits over limestone

Typical profile

A - 0 to 3 inches: fine sand E - 3 to 14 inches: fine sand E/B - 14 to 25 inches: fine sand Btg - 25 to 30 inches: fine sandy loam 2R - 30 to 40 inches: bedrock

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 8 to 40 inches to lithic bedrock
Natural drainage class: Poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: About 0 to 18 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Calcium carbonate, maximum in profile: 4 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 2.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3w Hydrologic Soil Group: A/D Ecological site: South Florida Flatwoods (R155XY003FL) Forage suitability group: Sandy over loamy soils on flats of hydric or mesic lowlands (G155XB241FL)
 Other vegetative classification: South Florida Flatwoods (R155XY003FL)
 Hydric soil rating: Yes

Minor Components

Hallandale

Percent of map unit: 7 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Linear Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: Yes

Wabasso

Percent of map unit: 6 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Linear, convex Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Ft. drum

Percent of map unit: 2 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

12—Hallandale fine sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2tzx4 Elevation: 0 to 70 feet Mean annual precipitation: 60 to 70 inches Mean annual air temperature: 72 to 79 degrees F Frost-free period: 360 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Hallandale and similar soils: 90 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Hallandale

Setting

Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy marine deposits over limestone

Typical profile

A - 0 to 2 inches: fine sand Eg - 2 to 7 inches: fine sand Bw - 7 to 12 inches: fine sand 2R - 12 to 22 inches: bedrock

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 2 to 20 inches to lithic bedrock
Natural drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 5.95 in/hr)
Depth to water table: About 6 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 0.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4w Hydrologic Soil Group: B/D Forage suitability group: Sandy soils on flats of mesic or hydric lowlands (G155XB141FL) Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: Yes

Minor Components

Dania

Percent of map unit: 5 percent Landform: Marshes on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Concave Across-slope shape: Concave Other vegetative classification: Freshwater Marshes and Ponds (R156AY010FL) Hydric soil rating: Yes

Plantation

Percent of map unit: 5 percent Landform: Depressions on marine terraces, flatwoods on marine terraces Landform position (three-dimensional): Tread, dip, talf Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

14—Matlacha gravelly fine sand, limestone substratum

Map Unit Setting

National map unit symbol: 1hn8t Mean annual precipitation: 60 to 68 inches Mean annual air temperature: 72 to 79 degrees F Frost-free period: 358 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Matlacha, limestone substratum, and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Matlacha, Limestone Substratum

Setting

Landform: Flats on marine terraces Landform position (three-dimensional): Talf Down-slope shape: Convex Across-slope shape: Linear Parent material: Sandy mine spoil or earthy fill

Typical profile

C - 0 to 23 inches: gravelly fine sand 2Ab - 23 to 27 inches: fine sand 2Eb - 27 to 48 inches: fine sand 3R - 48 to 52 inches: unweathered bedrock

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: 40 to 60 inches to lithic bedrock
Natural drainage class: Somewhat poorly drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): High (1.98 to 5.95 in/hr)
Depth to water table: About 24 to 36 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 5 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 2.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: B *Forage suitability group:* Forage suitability group not assigned (G156AC999FL) *Hydric soil rating:* No

Minor Components

Hallandale

Percent of map unit: 5 percent Landform: Flats on marine terraces Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: Yes

Margate

Percent of map unit: 5 percent Landform: Drainageways on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

15—Immokalee fine sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2s3lk Elevation: 0 to 130 feet Mean annual precipitation: 44 to 56 inches Mean annual air temperature: 70 to 77 degrees F Frost-free period: 350 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Immokalee and similar soils: 90 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Immokalee

Setting

Landform: Flatwoods on marine terraces Landform position (three-dimensional): Riser, talf Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy marine deposits

Typical profile

A - 0 to 6 inches: fine sand E - 6 to 35 inches: fine sand Bh - 35 to 54 inches: fine sand BC - 54 to 80 inches: fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 1.98 in/hr)
Depth to water table: About 6 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Low (about 5.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: B/D
Forage suitability group: Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
Other vegetative classification: South Florida Flatwoods (R155XY003FL)
Hydric soil rating: No

Minor Components

Basinger

Percent of map unit: 4 percent Landform: Depressions on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Concave, linear Across-slope shape: Concave, linear Hydric soil rating: Yes

Pomello

Percent of map unit: 2 percent Landform: Knolls on marine terraces, ridges on marine terraces Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, interfluve, riser Down-slope shape: Convex, linear Across-slope shape: Linear Ecological site: Sand Pine Scrub (R155XY001FL) Other vegetative classification: Sand Pine Scrub (R155XY001FL) Hydric soil rating: No

Wabasso

Percent of map unit: 2 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Convex, linear Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Margate

Percent of map unit: 1 percent

Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Placid

Percent of map unit: 1 percent Landform: Drainageways on marine terraces, depressions on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Concave Across-slope shape: Concave Other vegetative classification: Freshwater Marshes and Ponds (R155XY010FL) Hydric soil rating: Yes

16—Immokalee, limestone substratum-Urban land complex

Map Unit Setting

National map unit symbol: 1hn8w Elevation: 10 to 100 feet Mean annual precipitation: 60 to 68 inches Mean annual air temperature: 72 to 79 degrees F Frost-free period: 358 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Immokalee, limestone substratum, and similar soils: 50 percent *Urban land:* 40 percent *Minor components:* 10 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Immokalee, Limestone Substratum

Setting

Landform: Flatwoods on marine terraces Landform position (three-dimensional): Talf Down-slope shape: Convex Across-slope shape: Linear Parent material: Sandy marine deposits

Typical profile

A - 0 to 5 inches: fine sand E - 5 to 48 inches: fine sand Bh - 48 to 58 inches: fine sand 2R - 58 to 62 inches: weathered bedrock

Properties and qualities

Slope: 0 to 2 percent Depth to restrictive feature: 40 to 72 inches to paralithic bedrock Natural drainage class: Poorly drained Runoff class: High

Custom Soil Resource Report

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr) Depth to water table: About 6 to 18 inches

Frequency of flooding: None

Frequency of ponding: None

Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)

Sodium adsorption ratio, maximum in profile: 4.0

Available water storage in profile: Low (about 3.9 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4w Hydrologic Soil Group: A/D Forage suitability group: Forage suitability group not assigned (G156AC999FL) Hydric soil rating: No

Description of Urban Land

Setting

Landform: Marine terraces Landform position (three-dimensional): Interfluve, talf Down-slope shape: Linear Across-slope shape: Linear

Minor Components

Immokalee

Percent of map unit: 3 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Talf Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

Basinger

Percent of map unit: 3 percent Landform: Drainageways on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Margate

Percent of map unit: 2 percent Landform: Drainageways on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Pompano

Percent of map unit: 2 percent Landform: Drainageways on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

19—Margate fine sand, occasionally ponded, 0 to 1 percent slopes

Map Unit Setting

National map unit symbol: 2sm5l Elevation: 0 to 30 feet Mean annual precipitation: 60 to 70 inches Mean annual air temperature: 72 to 81 degrees F Frost-free period: 360 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Margate and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Margate

Setting

Landform: Flats on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Linear Across-slope shape: Concave Parent material: Sandy marine deposits over limestone

Typical profile

A - 0 to 8 inches: fine sand
E - 8 to 16 inches: fine sand
Bw - 16 to 28 inches: fine sand
C - 28 to 32 inches: very gravelly fine sand
2R - 32 to 42 inches: bedrock

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Natural drainage class: Poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (1.98 to 19.98 in/hr)
Depth to water table: About 0 to 18 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Calcium carbonate, maximum in profile: 4 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 2.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4w
Hydrologic Soil Group: A/D
Forage suitability group: Sandy soils on stream terraces, flood plains, or in depressions (G156AC145FL)
Hydric soil rating: Yes

Minor Components

Basinger

Percent of map unit: 5 percent Landform: Flats on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Linear Across-slope shape: Concave Other vegetative classification: Slough (R155XY011FL) Hydric soil rating: Yes

Matlacha

Percent of map unit: 5 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Convex, linear Across-slope shape: Linear Hydric soil rating: No

Plantation

Percent of map unit: 5 percent Landform: Marshes on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

27—Plantation muck

Map Unit Setting

National map unit symbol: 1hn97 Elevation: 0 to 30 feet Mean annual precipitation: 60 to 68 inches Mean annual air temperature: 72 to 79 degrees F Frost-free period: 358 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Plantation, drained, and similar soils: 70 percent Plantation, undrained, and similar soils: 20 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Plantation, Drained

Setting

Landform: Marshes on marine terraces

Landform position (three-dimensional): Talf

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Herbaceous organic material over sandy marine deposits over limestone

Typical profile

Oa - 0 to 10 inches: muck

A - 10 to 28 inches: fine sand

Cg - 28 to 35 inches: fine sandy loam

2R - 35 to 39 inches: unweathered bedrock

Properties and qualities

Slope: 0 to 1 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Natural drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (1.98 to 19.98 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3w Hydrologic Soil Group: A/D Forage suitability group: Organic soils in depressions and on flood plains (G156AC645FL) Hydric soil rating: Yes

Description of Plantation, Undrained

Setting

Landform: Marshes on marine terraces Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Parent material: Herbaceous organic material over sandy marine deposits over limestone

Typical profile

Oa - 0 to 10 inches: muck *A - 10 to 28 inches:* fine sand *Btg - 28 to 35 inches:* fine sandy loam *2R - 35 to 39 inches:* unweathered bedrock

Properties and qualities

Slope: 0 to 1 percent

Custom Soil Resource Report

Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Natural drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (1.98 to 19.98 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: None
Frequency of ponding: Frequent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Low (about 4.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7w Hydrologic Soil Group: A/D Forage suitability group: Organic soils in depressions and on flood plains (G156AC645FL) Hydric soil rating: Yes

Minor Components

Hallandale

Percent of map unit: 3 percent Landform: Flats on marine terraces Landform position (three-dimensional): Talf Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: Yes

Dania

Percent of map unit: 3 percent Landform: Marshes on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

Margate

Percent of map unit: 2 percent Landform: Drainageways on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

Lauderhill

Percent of map unit: 2 percent Landform: Depressions on marine terraces Landform position (three-dimensional): Dip Down-slope shape: Concave Across-slope shape: Concave Hydric soil rating: Yes

29—Pompano fine sand, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2tzw3 Elevation: 0 to 100 feet Mean annual precipitation: 44 to 65 inches Mean annual air temperature: 68 to 77 degrees F Frost-free period: 350 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Pompano and similar soils: 80 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pompano

Setting

Landform: Flats on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Linear Across-slope shape: Linear, concave Parent material: Sandy marine deposits

Typical profile

A - 0 to 4 inches: fine sand C - 4 to 80 inches: fine sand

Properties and qualities

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Poorly drained
Runoff class: Very high
Capacity of the most limiting layer to transmit water (Ksat): High to very high (6.00 to 20.00 in/hr)
Depth to water table: About 3 to 18 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum in profile: 2 percent
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Low (about 4.8 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4w Hydrologic Soil Group: A/D Forage suitability group: Sandy soils on flats of mesic or hydric lowlands (G155XB141FL)
 Other vegetative classification: Slough (R155XY011FL)
 Hydric soil rating: Yes

Minor Components

Anclote

Percent of map unit: 4 percent Landform: Depressions on marine terraces Landform position (three-dimensional): Tread, dip Down-slope shape: Concave, convex Across-slope shape: Concave, linear Other vegetative classification: Freshwater Marshes and Ponds (R155XY010FL) Hydric soil rating: Yes

Valkaria

Percent of map unit: 4 percent Landform: Drainageways on flats on marine terraces Landform position (three-dimensional): Tread, dip, talf Down-slope shape: Linear Across-slope shape: Linear, concave Other vegetative classification: Slough (R155XY011FL) Hydric soil rating: Yes

Malabar

Percent of map unit: 4 percent Landform: — error in exists on — Landform position (three-dimensional): Tread, dip, talf Down-slope shape: Linear, concave Across-slope shape: Linear, concave Ecological site: Slough (R155XY011FL) Other vegetative classification: Slough (R155XY011FL) Hydric soil rating: Yes

Immokalee

Percent of map unit: 3 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Riser, talf Down-slope shape: Linear Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Myakka

Percent of map unit: 3 percent Landform: Drainageways on flatwoods on marine terraces Landform position (three-dimensional): Tread, dip, talf Down-slope shape: Linear Across-slope shape: Linear, concave Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Riviera

Percent of map unit: 2 percent *Landform:* Drainageways on marine terraces, flats on marine terraces *Landform position (three-dimensional):* Tread, dip, talf Down-slope shape: Linear Across-slope shape: Concave, linear Ecological site: Slough (R155XY011FL) Other vegetative classification: Slough (R155XY011FL) Hydric soil rating: Yes

36—Udorthents

Map Unit Setting

National map unit symbol: 1hn9j Mean annual precipitation: 60 to 68 inches Mean annual air temperature: 72 to 79 degrees F Frost-free period: 358 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Udorthents and similar soils: 100 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents

Setting

Landform: Marine terraces Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Linear Parent material: Altered marine deposits

Typical profile

C - 0 to 57 inches: cobbly sand

Properties and qualities

Slope: 2 to 40 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 2.3 inches)

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: A Forage suitability group: Forage suitability group not assigned (G156AC999FL)

Hydric soil rating: No

40—Urban land, 0 to 2 percent slopes

Map Unit Setting

National map unit symbol: 2x9fc Elevation: 0 to 200 feet Mean annual precipitation: 40 to 68 inches Mean annual air temperature: 68 to 79 degrees F Frost-free period: 345 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Urban land: 85 percent *Minor components:* 15 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Urban Land

Setting

Landform: Hills on marine terraces, ridges on marine terraces, knolls on marine terraces, rises on marine terraces, flatwoods on marine terraces
 Landform position (two-dimensional): Backslope, summit
 Landform position (three-dimensional): Interfluve, side slope, riser, rise, talf
 Down-slope shape: Linear, convex
 Across-slope shape: Linear
 Parent material: No parent material

Typical profile

M - 0 to 6 inches: cemented material [^]C - 6 to 36 inches: paragravelly sand 2Ab - 36 to 46 inches: paragravelly fine sand 2Cb - 46 to 80 inches: paragravelly fine sand

Minor Components

Matlacha

Percent of map unit: 3 percent Landform: Flats on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Convex, linear Across-slope shape: Linear Hydric soil rating: No

St. augustine

Percent of map unit: 3 percent Landform: Marine terraces Landform position (three-dimensional): Tread, rise Down-slope shape: Linear Across-slope shape: Convex Hydric soil rating: No

Paola

Percent of map unit: 1 percent

Landform: Ridges on marine terraces, knolls on marine terraces Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, interfluve, riser Down-slope shape: Linear, convex Across-slope shape: Linear Other vegetative classification: Sand Pine Scrub (R155XY001FL) Hydric soil rating: No

Pomello

Percent of map unit: 1 percent Landform: Ridges on marine terraces, knolls on marine terraces Landform position (two-dimensional): Summit, backslope Landform position (three-dimensional): Side slope, interfluve, riser Down-slope shape: Linear, convex Across-slope shape: Linear Other vegetative classification: Sand Pine Scrub (R155XY001FL) Hydric soil rating: No

Adamsville

Percent of map unit: 1 percent Landform: Rises on marine terraces, knolls on marine terraces Landform position (three-dimensional): Tread, rise Down-slope shape: Convex Across-slope shape: Linear Other vegetative classification: Upland Hardwood Hammock (R155XY008FL) Hydric soil rating: No

Boca

Percent of map unit: 1 percent Landform: Flats on marine terraces, drainageways on marine terraces Landform position (three-dimensional): Tread, talf, dip Down-slope shape: Convex, linear Across-slope shape: Linear, concave Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: Yes

Eaugallie

Percent of map unit: 1 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Convex Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Hallandale

Percent of map unit: 1 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Tread, talf Down-slope shape: Linear Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: Yes

Immokalee

Percent of map unit: 1 percent Landform: Flatwoods on marine terraces Landform position (three-dimensional): Riser, talf Down-slope shape: Linear Across-slope shape: Linear Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Myakka

Percent of map unit: 1 percent Landform: Drainageways on flatwoods on marine terraces Landform position (three-dimensional): Tread, dip, talf Down-slope shape: Linear Across-slope shape: Linear, concave Other vegetative classification: South Florida Flatwoods (R155XY003FL) Hydric soil rating: No

Apopka

Percent of map unit: 1 percent Landform: Hills on marine terraces, ridges on marine terraces Landform position (two-dimensional): Backslope, summit Landform position (three-dimensional): Interfluve, side slope, riser Down-slope shape: Convex Across-slope shape: Linear Other vegetative classification: Longleaf Pine-Turkey Oak Hills (R155XY002FL) Hydric soil rating: No

99—Water

Map Unit Composition

Water: 100 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Palm Beach County Area, Florida

47—Udorthents, 2 to 35 percent slopes

Map Unit Setting

National map unit symbol: 1j7dz Mean annual precipitation: 48 to 56 inches Mean annual air temperature: 70 to 77 degrees F Frost-free period: 358 to 365 days Farmland classification: Not prime farmland

Map Unit Composition

Udorthents and similar soils: 95 percent Minor components: 5 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Udorthents

Setting

Landform: Marine terraces Landform position (three-dimensional): Interfluve Down-slope shape: Convex Across-slope shape: Linear Parent material: Altered marine deposits

Typical profile

A - 0 to 7 inches: gravelly sand C1 - 7 to 57 inches: gravelly sand C2 - 57 to 80 inches: gravelly sand

Properties and qualities

Slope: 2 to 65 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Salinity, maximum in profile: Nonsaline to very slightly saline (0.0 to 2.0 mmhos/cm)
Sodium adsorption ratio, maximum in profile: 4.0
Available water storage in profile: Very low (about 2.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: A Forage suitability group: Forage suitability group not assigned (G156AC999FL) Hydric soil rating: No

Minor Components

Riviera

Percent of map unit: 5 percent

Custom Soil Resource Report

Landform: Flats on marine terraces, drainageways on marine terraces Landform position (three-dimensional): Talf, dip Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

99—Water

Map Unit Composition Water: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

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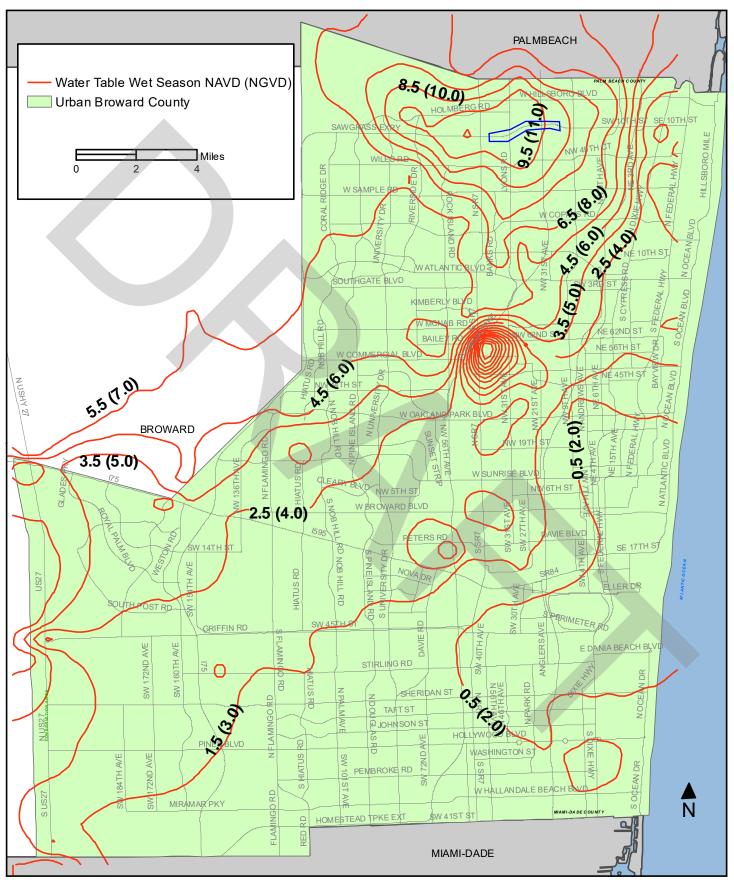
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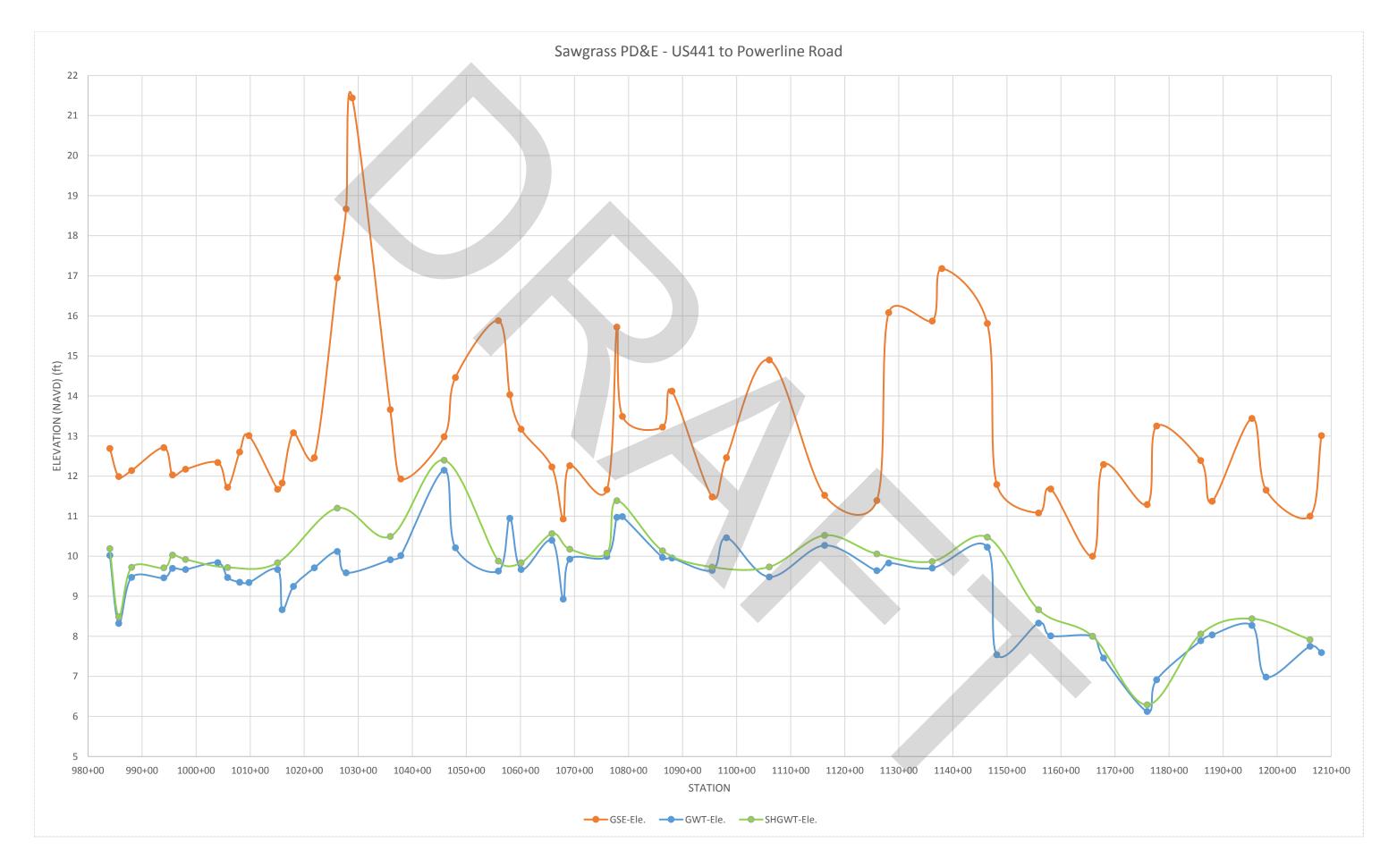
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WATER TABLE MAP - AVERAGE WET SEASON



Division Name: Planning and Environmental Regulation Department Name: Environmental Protection and Growth Management This map is for conceptual purposes only and should not be used for legal boundary determinations. Elevation converted from NGDV to NAVD using the FEMA approved conversion factor for Broward County of (-) 1.5



Boring #	Drilled Date	GWT Depth (ft)	GWT Ele. (ft)	Station	Elevation	Offset	Depth
R-101A	8/18/2017	2.67	10.02	984+03	12.69	7.6	10
R-101B	8/18/2017	3.25	9.46	994+01	12.71	7.05	10
R-101	8/15/2017	2.50	9.84	1003+99	12.34	7.76	10
R-102	8/15/2017	3.67	9.34	1009+77	13.01	10.66	10
R-103	8/15/2017	3.17	8.66	1015+92	11.83	6.91	10
R-104	8/15/2017	2.75	9.71	1021+87	12.46	4.7	10
R-105	8/15/2017	7.08	14.36	1028+89	21.44	11	10
R-106	8/16/2017	GNE	GNE	1053+35	21.89	13.16	10
R-107	8/16/2017	3.50	9.67	1060+14	13.17	9.43	10
R-108	8/16/2017	2.33	9.93	1069+12	12.26	7.8	10
R-109	8/16/2017	4.75	10.97	1077+83	15.72	11.87	10
R-110	8/16/2017	8.58	18.18	1104+84	26.76	-21.88	10
R-201A	8/18/2017	3.67	8.32	985+68	11.99	118.14	10
R-201B	8/18/2017	2 .33	9.70	995+63	12.03	111.53	10
R-201	8/16/2017	2.25	9.47	1005+84	11.72	121.44	10
R-202	8/16/2017	2.00	9.67	1015+06	11.67	120.6	10
R-203	8/16/2017	6.83	10.12	1026+09	16.95	109.72	10
R-204	8/16/2017	3.75	9.91	1035+92	13.66	166.56	10
R-205	8/17/2017	0.83	12.15	1045+88	12.98	196.58	10
R-206	8/17/2017	6.25	9.63	1055+92	15.88	124.35	10
R-207	8/17/2017	1.83	10.40	1065+81	12.23	123.78	10
R-208	8/17/2017	1.67	9.99	1076+00	11.66	182.77	10
R-209	8/17/2017	3.25	9.97	1086+27	13.22	181.27	10
R-210	8/17/2017	1.83	9.65	1095+47	11.48	223.13	10
R-211	8/17/2017	5.42	9.48	1106+04	14.9	176.21	10
R-212	8/17/2017	1.25	10.27	1116+27	11.52	137.24	10
R-213	8/16/2017	1.75	9.64	1125+93	11.39	150.76	10
R-214	8/16/2017	6.17	9.70	1136+18	15.87	57.39	10

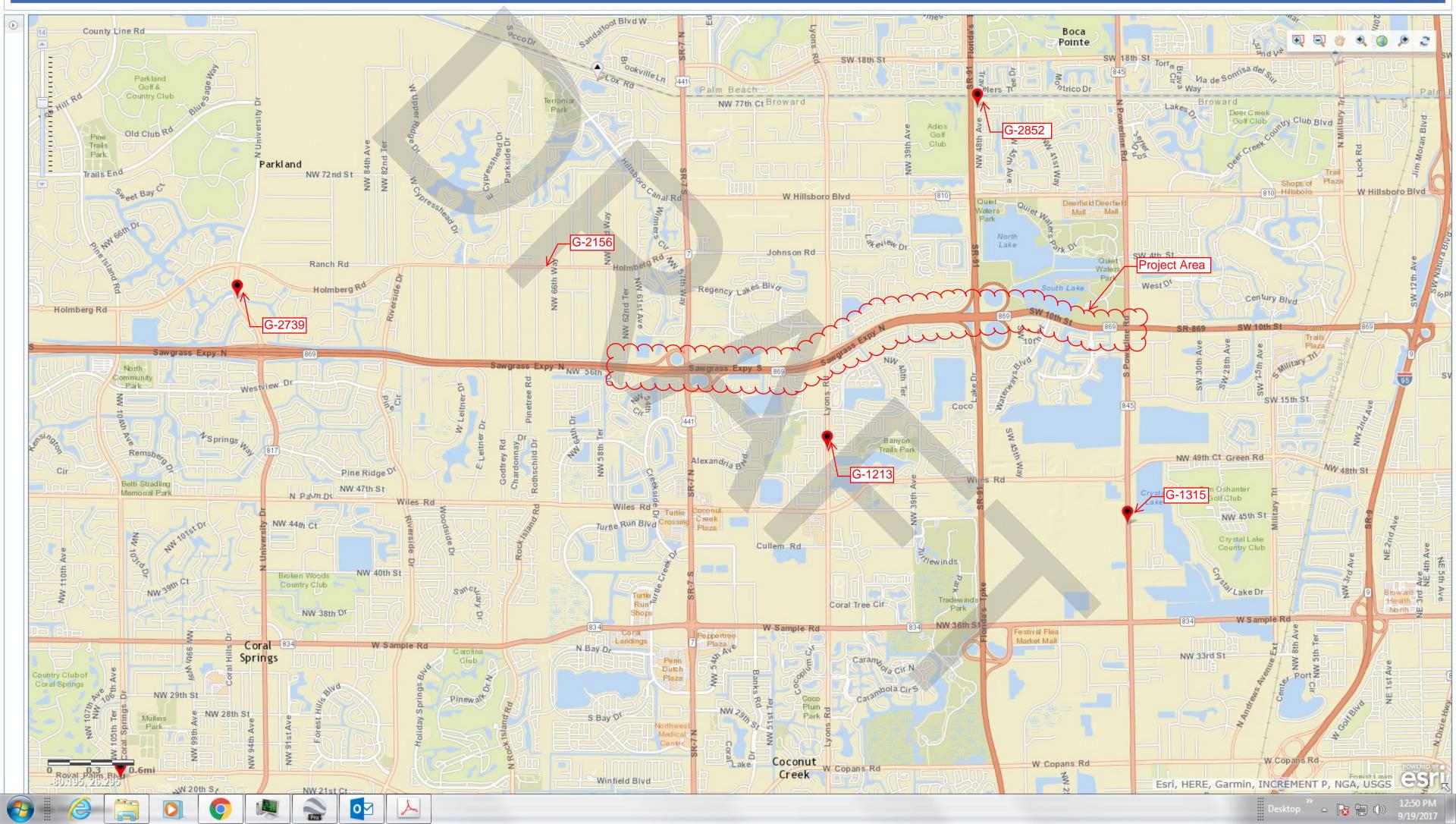
FTE Sawgrass PD&E-US441 to Powerline

Boring #	Drilled Date	GWT Depth (ft)	GWT Ele. (ft)	Station	Elevation	Offset	Depth
R-215	8/16/2017	5.58	10.23	1146+37	15.81	63.12	10
R-216	8/16/2017	2.75	8.33	1155+86	11.08	163.94	10
R-217	8/16/2017	2.00	8.00	54+85	10	69.46	10
R-218	8/17/2017	5.17	6.12	64+95	11.29	57.36	10
R-219	8/17/2017	4.50	7.89	74+86	12.39	21.96	10
R-220	8/17/2017	5.17	8.27	84+34	13.44	38.07	10
R-221	8/17/2017	3.25	7.75	95+07	11	40.79	10
R-301A	8/18/2017	2.67	9.47	988+07	12.14	-112.82	10
R-301B	8/18/2017	2.50	9.67	998+05	12.17	-113.45	10
R-301	8/15/2017	3.25	9.35	1008+04	12.6	-110.83	10
R-302	8/15/2017	3.83	9.25	1018+02	13.08	-123.85	10
R-303	8/15/2017	9.08	9.59	1027+75	18.67	-112.53	10
R-304	8/15/2017	1.92	10.01	1037+85	11.93	-176.19	10
R-305	8/15/2017	4.25	10.21	1047+97	14.46	-158.74	10
R-306	8/15/2017	3.08	10.95	1058+03	14.03	-148.18	10
R-307	8/14/2017	2.00	8.93	1067+90	10.93	-131.59	10
R-308	8/14/2017	2.50	10.99	1078+87	13.49	-147.56	10
R-309	8/14/2017	4.17	9.95	1088+03	14.12	-156.79	10
R-310	8/14/2017	2.00	10.46	1098+12	12.46	-245.15	10
R-313	8/14/2017	6.25	9.83	1128+14	16.08	-108.79	10
R-314	8/14/2017	1.42	15.76	1137+97	17.18	-106	10
R-315	8/14/2017	4.25	7.54	1148+13	11.79	-166.08	10
R-316	8/14/2017	3.67	8.01	1158+12	11.68	-149.1	10
R-317	8/14/2017	4.83	7.46	56+89	12.29	-145	10
R-318	8/14/2017	6.33	6.92	66+69	13.25	-126.75	10
R-319	8/14/2017	3.33	8.04	76+96	11.37	-126.67	10
R-320	8/14/2017	4.67	6.98	86+95	11.65	-119.92	10
R-321	8/14/2017	5.42	7.59	97+19	13.01	-166.83	10

FTE Sawgrass PD&E-US441 to Powerline



National Water Information System: Mapper





Help Info

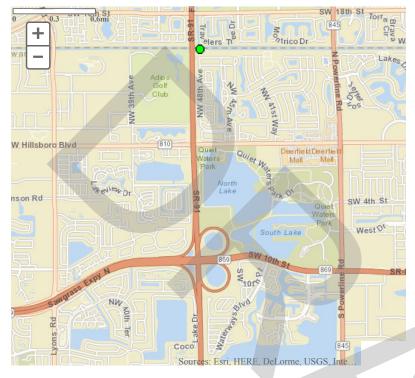
Site Number: 261938080101001 - G -2852



Groundwater Watch

USGS Home Contact USGS Search USGS

Latest News...



Groundwater Watch Help Page

DESCRIPTION: Latitude 26°19'39.6", Longitude 80°10'09.3" NAD83 Broward County, Florida, Hydrologic Unit 03090202 Well depth: 140 feet Hole depth: 221 feet Land surface altitude: 15.82feet above NGVD29. Well completed in "Biscayne aquifer" (N400BISCYN) national aquifer. Well completed in "Biscayne Limestone Aquifer" (112BSCNN) local aquifer

AVAILABLE DATA:

Data Type	Begin Date	End Date	Count
Current / Historical	2007-10-	2017-09-	
Observations	01	16	
Daily Data			
Groundwater level above NGVD	1995-11-	2017-09-	7680
1929, feet	09	15	/000
Field groundwater-level	1988-10-	2017-08-	199
measurements	25	07	199
Field/Lab water-quality samples			
Water-Year Summary	2006	2016	11
Additional Data Sources	Begin Date	End Date	Count
Groundwater Watch **offsite**	1988	2017	199
OPERATION:			
Record for this site is maintain	ed by the I	ISGS Florida	•

Record for this site is maintained by the USGS Florida Water Science Center - Ft. Lauderdale Email questions about this site toFlorida Water-Data Inquiries

Most recent data value: 7.94 on 9/18/2017

Period of Record Monthly Statistics for 261938080101001

Groundwater level above NGVD 1929, feet

All Approved Continuous & Periodic Data Used In Analysis Note: Highlighted values in the table indicate closest statistic to the most

recent data value.

6.90

7.01

6.89

6.80

6.93

7.69

7.60

7.57

7.85

7.90

7.43

7.06

of 9/15/2017 18:33-2

%ile

7.54

7.65

7.70

7.59

7.95

8.70

8.27

8.28

8.26

8.63

8.04

7.66

Month Lowest 10th 25th 50th 75th Median %ile %ile %ile %ile

6.47

6.15

6.07

5.89

5.87

6.38

6.73

7.00

7.29

7.11

7.00

6.58

As

6.00

5.83

5.80

5.09

5.29

5.50

5.98

6.20

6.76

6.69

5.88

5.94

View month/year statistics

Statistics Options

5.84

5.72

5.03

4.82

3.74

3.67

4.66

5.29

6.64

6.47

4.43

3.77

lan

Feb

Mar

Apr

May

Jun

Jul

Aug

Sep

Oct

Nov

Dec

The

Ċ

90th Highest

10.12

10.84

10.27

9.81

8.83

9.79

10.25

11.26

10.54

10.78

9.84

9.82

%ile Median

9.66

9.66

9.19

8.90

8.61

9.24

9.20

9.61

9.16

10.04

9.59

9.29

of

Years

26

26

26

27

29

26

25

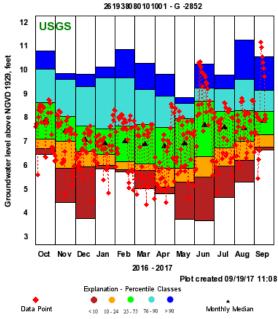
25

26

28

27

25



Daily Groundwater Data

Site Statistics

Most recent Provisional daily data value: 7.94 on 09/18/17

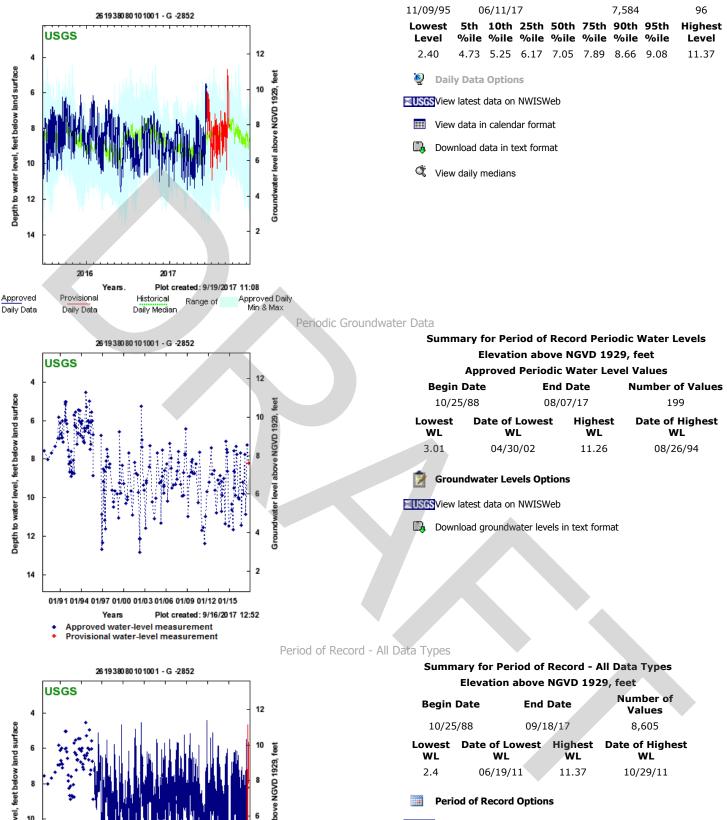
Summary for Period of Continuous Record

Groundwater level above NGVD 1929, feet

Approved Daily Maximum Values Data Used in Analysis

Begin End Date Date	Days	% Complete
------------------------	------	---------------

USGS -- Groundwater Watch



USGSView latest data on NWISWeb for all data types

🔇 View month/year statistics

Download groundwater levels in text format of all data types

Depth to water level, feet below land surface Elevation above NGVD 1929, fee 10 12 2 14 91 94 97 00 03 06 09 12 15 Plot created: 9/19/2017 11:08 Years Water-Level Measurement Approved Provisional Daily Data Daily Data Approved
 Provisional

Return to Groundwater Watch Return to County Page Return to State Page	Return to Groundwater Watch
--	-----------------------------

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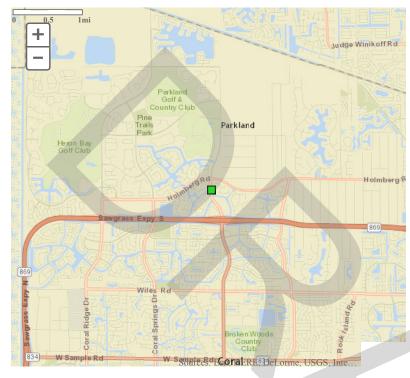


Groundwater Watch

USGS Home Contact USGS Search USGS

Latest News...

Site Number: 261831080151301 - G -2739



DESCRIPTION:

Latitude 26°18'29.7", Longitude 80°15'10.8" NAD83 Broward County, Florida, Hydrologic Unit 03090202 Well depth: 21 feet Land surface altitude: 12.30feet above NGVD29. Well completed in "Biscayne aquifer" (N400BISCYN) national aquifer. Well completed in "Biscayne Limestone Aquifer" (112BSCNN) local aquifer

AVAILABLE DATA:

Data Type	Begin Date	End Date	Count
Current / Historical	2007-10-	2017-08-	
Observations	01	15	
Daily Data			
Groundwater level above NGVD	1991-12-	2017-08-	9033
1929, feet	07	14	5055
Field groundwater-level measurements	1993-11- 23	2017-08- 15	128
Water-Year Summary	2006	2016	11

Begin **Additional Data Sources End Date Count** Date Groundwater Watch ** offsite** 1993 2017 128

OPERATION:

Record for this site is maintained by the USGS Florida Water Science Center - Ft. Lauderdale Email questions about this site toFlorida Water-Data Inquiries

Most recent data value: 7.93 on 8/15/2017

Period of Record Monthly Statistics for 261831080151301 Groundwater level above NGVD 1929, feet

All Approved Continuous & Periodic Data Used In Analysis

Note: Highlighted values in the table indicate closest statistic to the most recent data value.

7.72

7.84

7.83

7.62

7.43

7.89

7.95

8.12

8.19

8.18

7.89

7.80

of 9/15/2017 18:32-2

%ile

8.04

8.24

8.08

7.88

7.80

8.49

8.23

8.37

8.53

8.40

8.10

8.11

Month Lowest 10th 25th 50th 75th Median %ile %ile %ile %ile

7.28

7.20

7.37

7.28

7.05

7.48

7.73

7.89

8.00

7.75

7.60

7.33

As

6.94

6.84

6.66

6.80

6.32

6.75

7.29

7.66

7.78

7.45

7.20

7.09

View month/year statistics

Statistics Options

6.34

6.02

5.66

6.06

5.68

5.87

6.59

7.61

7.68

7.26

6.93

6.82

lan

Feb

Mar

Apr

May

Jun

Jul

Aug

Sep

Oct

Nov

Dec

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90th Highest

8.76

8.82

8.33

8.80

8.96

9.15

8.79

8.64

8.85

8.87

8.67

8.67

%ile Median

8.24

8.38

8.26

8.04

8.22

8.75

8.54

8.52

8.76

8.70

8.53

8.39

of

Years

26

25

26

26

26

26

25

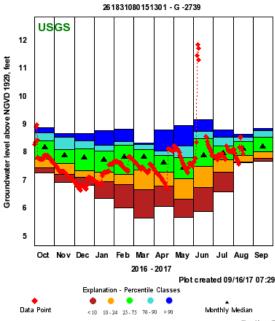
25

25

25

25

26



Groundwater Watch Help Page

Daily Groundwater Data

Site Statistics

Most recent Provisional daily data value: 8.06 on 08/14/17

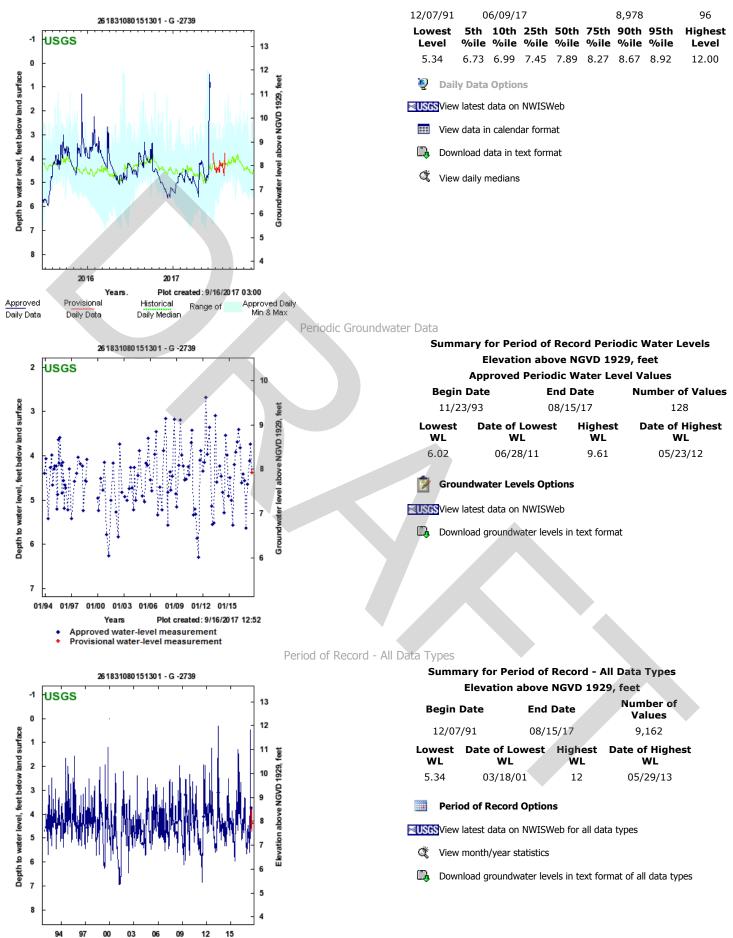
Summary for Period of Continuous Record

Groundwater level above NGVD 1929, feet

Approved Daily Maximum Values Data Used in Analysis

Begin Date	End Date	Days	% Complete
---------------	----------	------	---------------

USGS -- Groundwater Watch



Years

Provisional Daily Data

Approved

Daily Data

Plot created: 9/16/2017 03:17

Water-Level Measurement

Approved
 Provisional

Return to Groundwater Watch Return to County Page Return to State Page	Return to Groundwater Watch
--	-----------------------------

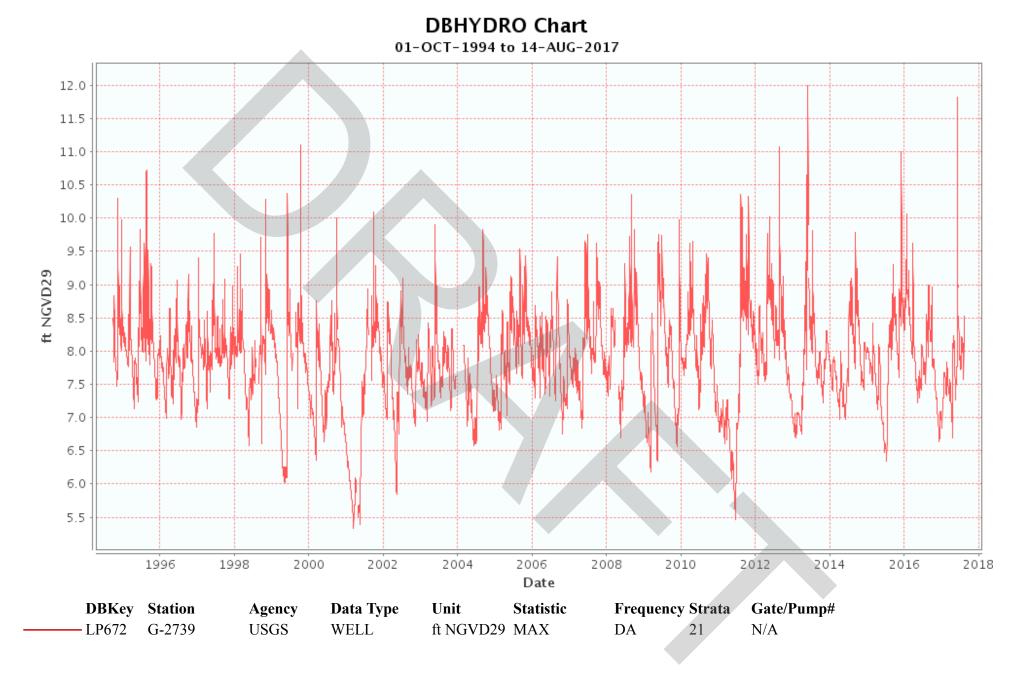
 * References to non-Department of the Interior (DOI) products do not constitute an endorsement by the DOI.

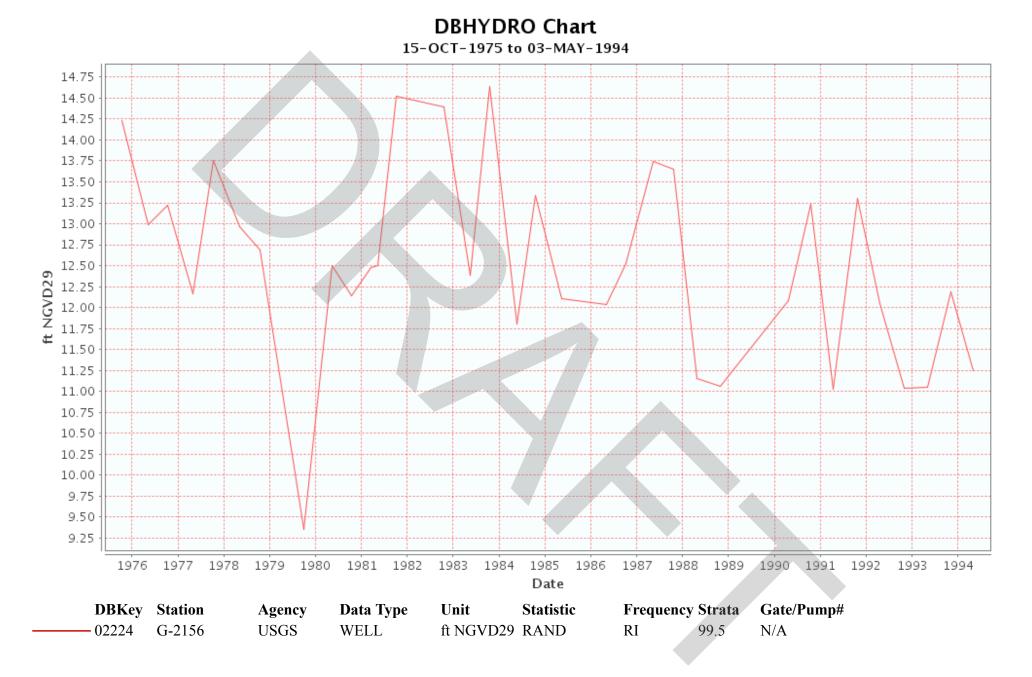
Accessibility FOIA Privacy Policies and Notices

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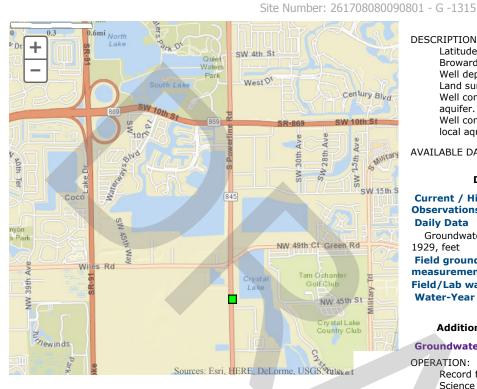




Groundwater Watch

USGS Home Contact USGS Search USGS

Latest News...



Groundwater Watch Help Page

DESCRIPTION:

Latitude 26°17'07.2", Longitude 80°09'08.0" NAD83 Broward County, Florida, Hydrologic Unit 03090202 Well depth: 14 feet

- Land surface altitude: 15.4feet above NGVD29.
- Well completed in "Biscayne aquifer" (N400BISCYN) national aquifer.
- Well completed in "Biscayne Limestone Aquifer" (112BSCNN) local aquifer

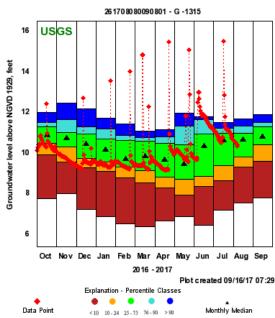
AVAILABLE DATA:

Data Type	Begin Date	End Date	Count
Current / Historical	2007-10-	2017-08-	
Observations	01	07	
Daily Data			
Groundwater level above NGVD 1929, feet	1973-10- 01	2017-08- 06	15364
Field groundwater-level measurements	1978-10- 14	2017-08- 07	149
Field/Lab water-quality samples Water-Year Summary	2006	2016	11
Additional Data Sources	Begin Date	End Date	Count
Groundwater Watch **offsite**	1978	2017	149

OPERATION:

Record for this site is maintained by the USGS Florida Water Science Center - Ft. Lauderdale

Email questions about this site toFlorida Water-Data Inquiries



Most recent data value: 10.32 on 8/7/2017 Period of Record Monthly Statistics for 261708080090801 Groundwater level above NGVD 1929, feet All Approved Continuous & Periodic Data Used In Analysis

ed values in the table indicate closest statistic to the most recent data value. Note: Highligh

Mani	th Lowest	10th	25th	50th	75th	90th	Highest	Number of
MON	Median	%ile	%ile	%ile	%ile	%ile	Median	Years
Jan	6.84	9.08	9.52	10.12	10.65	11.15	11.69	44
Feb	6.48	8.77	9.29	9.67	10.60	10.83	11.40	43
Mar	6.36	8.49	9.11	9.80	10.57	10.75	11.04	42
Apr	6.62	8.26	8.78	9.63	10.45	10.78	11.12	43
May	6.85	7.92	8.68	9.41	10.37	11.30	11.92	43
Jun	6.41	8.32	8.84	10.32	10.92	11.60	11.76	41
Jul	7.18	8.62	9.37	10.59	10.93	11.18	11.46	43
Aug	7.52	9.30	9.77	10.61	10.95	11.30	11.65	43
Sep	7.76	9.55	10.37	10.77	11.25	11.49	11.86	42
Oct	7.72	9.90	10.45	10.85	11.28	11.47	11.98	43
Nov	7.96	9.53	10.27	10.69	10.97	11.63	12.44	44
Dec	7.19	9.25	9.70	10.40	10.90	11.28	12.13	44
			As of	9/15/2017	18:30-2			
	Statisti	ce Onti	one					

Statistics Options

Ć View month/year statistics

Daily Groundwater Data

Site Statistics

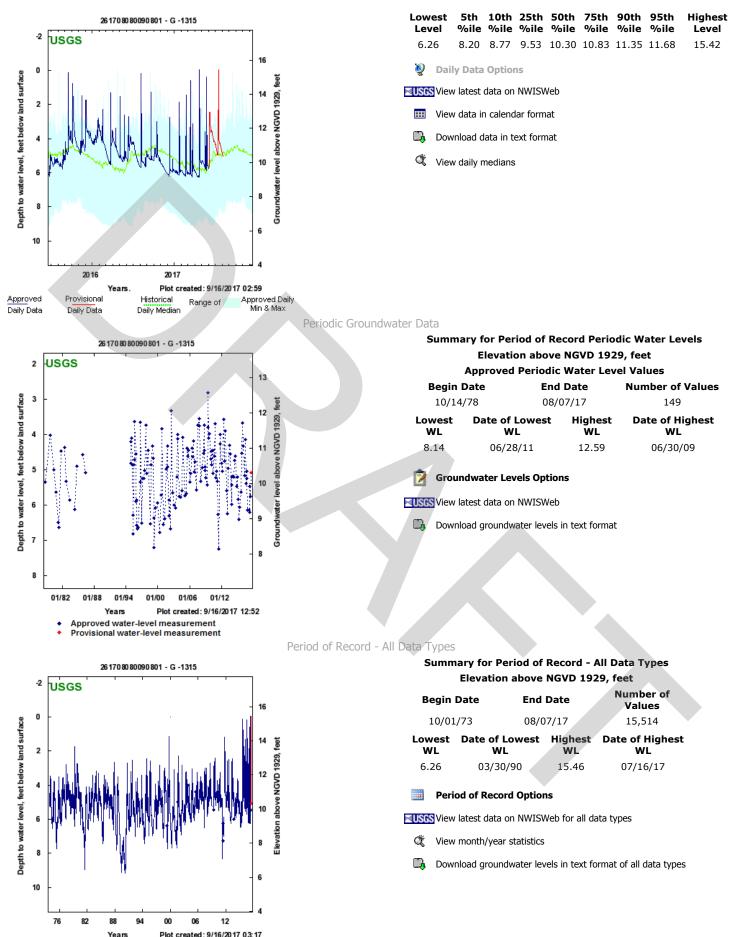
Most recent Provisional daily data value: 10.39 on 08/06/17

Summary for Period of Continuous Record

Groundwater level above NGVD 1929, feet

Approved Daily Maximum Values Data Used in Analysis

Begin Date	End Date	Days	% Complete
10/01/73	06/04/17	15,301	95



Water-Level Measurement

+ Approved + Provisional

Provisional

Daily Data

Approved

Daily Data

USGS -- Groundwater Watch

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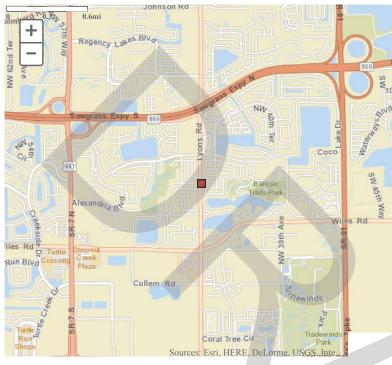
Site Number: 261734080111301 - G -1213



Groundwater Watch

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Latest News...



DESCRIPTION:

Latitude 26°17'34.6", Longitude 80°11'10.5" NAD83 Broward County, Florida, Hydrologic Unit 03090202 Well depth: 15 feet Land surface altitude: 18.3feet above NGVD29. Well completed in "Biscayne aquifer" (N400BISCYN) national aquifer. Well completed in "Biscayne Limestone Aquifer" (112BSCNN) local aquifer

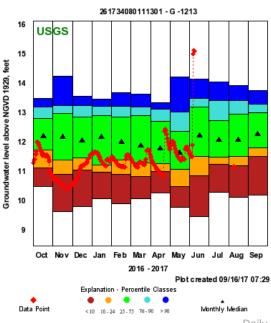
AVAILABLE DATA:

Data Type	Begin Date	End Date	Count
Current / Historical Observations	2007-10- 01	2017-06- 09	
Daily Data			
Groundwater level above NGVD 1929, feet	1963-01- 05	2017-06- 08	16192
Field groundwater-level measurements	1963-01- 04	2017-08- 09	303
Field/Lab water-quality samples			
Water-Year Summary	2006	2016	11
Additional Data Sources	Begin Date	End Date	Count
Groundwater Watch **offsite**	1963	2017	303

OPERATION:

Record for this site is maintained by the USGS Florida Water Science Center - Ft. Lauderdale

Email questions about this site toFlorida Water-Data Inquiries



Groundwater Watch Help Page

> Most recent data value: 11.19 on 8/9/2017 Period of Record Monthly Statistics for 261734080111301 Groundwater level above NGVD 1929, feet All <u>Approved</u> Continuous & Periodic Data Used In Analysis Note: Highlighted values in the table indicate closest statistic to the most recent data value

				uala valu	с.			
	Lowest	10th	25+b	E0th	75th	00th	Highest	Number
Month	Lowest	10th	2501	50th	~ -			of
	Median	%ile	%ile	%ile	%ile	%ile	Median	Years
Jan	10.07	10.98	11.22	12.17	12.92	13.22	13.48	55
Feb	9.92	10.92	11.40	12.01	12.85	13.22	13.69	55
Mar	10.09	10.83	11.31	11.88	12.91	13.20	13.64	55
Apr	10.28	11.01	11.27	11.79	12.72	13.12	13.35	55
May	9.78	10.49	11.08	11.65	12.37	13.02	14.23	55
Jun	9.47	10.85	11.53	12.22	13.20	13.49	14.15	55
Jul	10.29	11.24	11.50	12.09	12.74	13.42	14.05	54
Aug	10.14	11.20	11.52	12.07	12.95	13.43	13.93	54
Sep	10.21	11.52	11.81	12.27	13.00	13.30	13.77	54
Oct	10.49	11.13	11.74	12.20	12.82	13.21	13.49	54
Nov	9.64	10.92	11.39	12.19	12.99	13.25	14.24	54
Dec	9.80	11.06	11.46	12.05	12.86	13.24	13.57	53
			As of	9/15/2017	18:31-2			
annual lines	Ctatiati	an Ontin	n 0					

Statistics Options

View month/year statistics

Daily Groundwater Data

Site Statistics

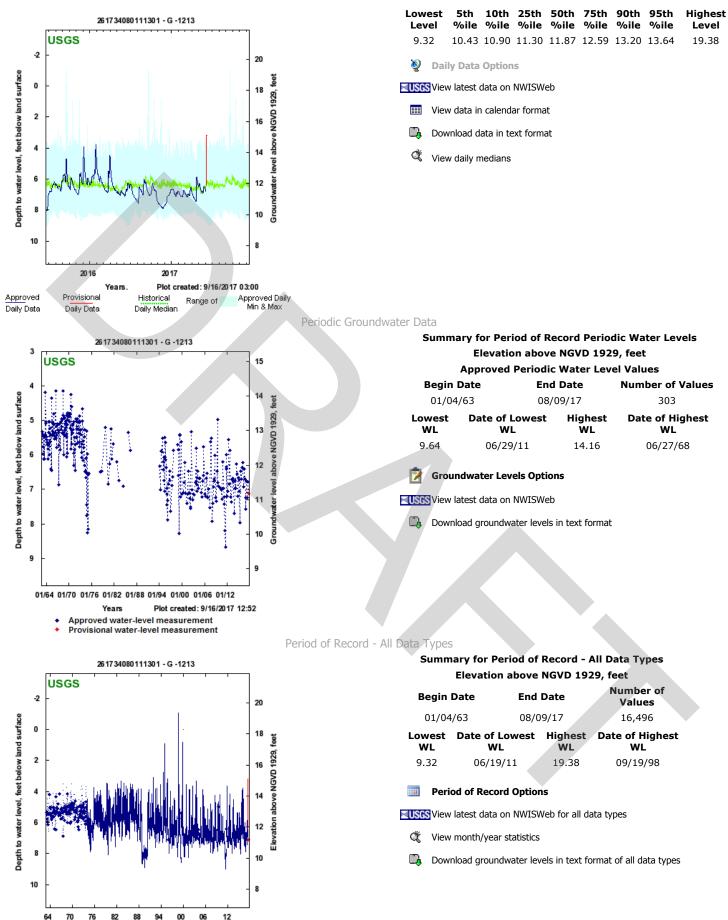
Most recent Provisional daily data value: 15.11 on 06/08/17

Summary for Period of Continuous Record

Groundwater level above NGVD 1929, feet Approved Daily Maximum Values Data Used in Analysis

Begin Date	End Date	Days	% Complete
01/05/63	06/04/17	16,188	81

USGS -- Groundwater Watch



Years

Provisional Daily Data

Approved

Daily Data

Plot created: 9/16/2017 03:17

Water-Level Measurement

+ Approved + Provisional

USGS -- Groundwater Watch

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<u> TABLE - 1</u>

BOREHOLE PERMEABILITY TEST RESULTS

Project: Sawgrass Expressway, from US-441 to Powerline Rd.

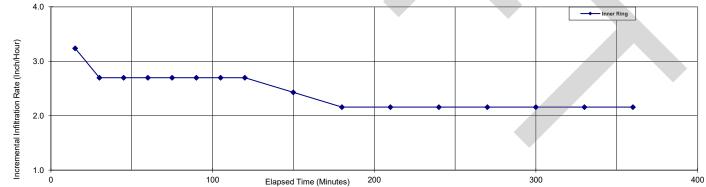
SFWMD METHOD

BHP No.	Date	Approx. Station	Approx. Offset	Bore Hole Dia. (in)	Depth of Hole (ft)	GWT Depth (ft)	Flow Rate Q [gal/min]	K [cfs/ft ²]	K [ft/day]
	•								
BHP-1	02/12/18			8.00	10.0	2.83	0.3000	1.29E-05	1.11
BHP-2	02/14/18			8.00	10.0	3.42	0.3000	1.10E-05	0.95
BHP-3	02/14/18			8.00	10.0	2.75	1.0000	4.40E-05	3.80
BHP-4	02/12/18			8.00	10.0	3.50	0.5000	1.81E-05	1.56
BHP-5	02/14/18			8.00	10.0	4.17	0.5000	1.58E-05	1.36
BHP-6	02/09/18			8.00	10.0	4.67	0.3000	8.73E-06	0.75
BHP-7	02/09/18			8.00	10.0	3.67	0.5000	1.74E-05	1.50
BHP-8	02/12/18			8.00	10.0	3.75	1.3800	4.72E-05	4.08
BHP-9	02/12/18			8.00	10.0	7.00	1.0000	2.28E-05	1.97
						~			

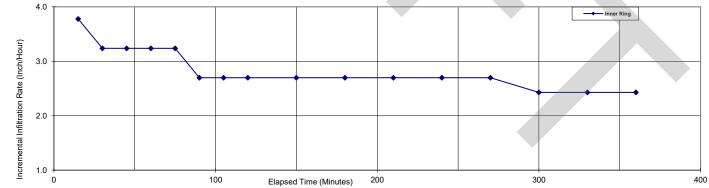
TABLE - D1 Project: 2000-01-16001 (FTE Sawgrass PD&E-US441 to Powerline-BC_Corradino) DOUBLE RING INFILTRATION TEST RESULTS SUMMARY

TEST NUMBER	BASELINE	APPROX. STATION	APPROX. OFFSET	GWT (ft)	Infiltration Rate Summary [inch/hour]	Infiltration Rate Summary [ft/day]
DRIT-1				GNE	2.2	4.3
DRIT-2				GNE	2.4	4.9
DRIT-3				GNE	1.1	2.2
DRIT-4				GNE	1.3	2.7
DRIT-5				2.5	1.9	3.8
DRIT-6				3.5	1.3	2.7
DRIT-7				2.5	4.3	8.6
DRIT-8				GNE	3.2	6.5
DRIT-9				GNE	2.7	5.4
DRIT-10						
DRIT-11				1.0	1.1	2.2
DRIT-12				3.0	1.1	2.2
DRIT-13				1.0	1.1	2.2
DRIT-14				GNE	1.3	2.7
DRIT-15				5.0	9.7	19.4
DRIT-16				3.5	6.5	12.9
DRIT-17				GNE	8.6	17.3
DRIT-18				GNE	4.3	8.6
DRIT-19				GNE	10.0	20.0
DRIT-20				GNE	8.1	16.2

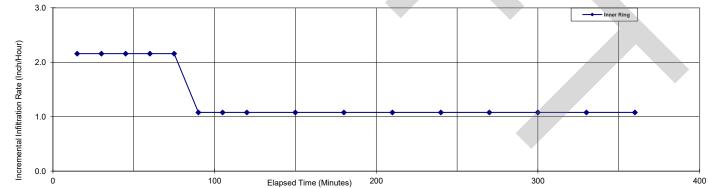
TEST No.:	DRIT-1	1	PROJECT NAME:	Sawgrass Expy	1 1	GENE	RAL SUBSURFACE PF	OFILE
DATE:	1/31/2018		PROJECT NAME:	26°18'3.64"N		DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'3.64"N 80°13'20.48"W				
LIQUID USED:	Water		STATION: OFFSET:		-			
pH: GROUND	, i i i i i i i i i i i i i i i i i i i		GROUND		-			
TEMPERATURE (°F):	71		ELEVATION:					
DEPTH TO WATER TAB		N/A						
PENETRATION OF RING		ches):		INNER: INNER:	3 12			6 24
THICKNESS OF RING W				INNER:	0.125			0.125
AREA OF RINGS (Inches	s ^ 2):			INNER:	113.10		ANNULAR:	339.29
			FLOW REA	DINGS (ml)			FILTRATION RATE	
INCREMENT No.	ELAPSED TIME (MIN.)	TOTAL TIME (MIN.)			LIQUID TEMPERATURE (°F)		IOUR)	REMARKS
	· · · ·		INNER RING	ANNULAR SPACE		INNER RING	ANNULAR SPACE	
0		0						
1	15	15	1500	3750		3.24	2.70	Partly Cloudy
2	15	30	1250	3750		2.70	2.70	Partly Cloudy
3	15	45	1250	3750		2.70	2.70	Partly Cloudy
4	15	60	1250	3500		2.70	2.52	Partly Cloudy
5	15	75	1250	3500		2.70	2.52	Partly Cloudy
6	15	90	1250	3500		2.70	2.52	Partly Cloudy
7	15	105	1250	3000		2.70	2.16	Partly Cloudy
8	15	120	1250	3000		2.70	2.16	Partly Cloudy
9	30	150	2250	6000		2.43	2.16	Partly Cloudy
10	30	180	2000	6000		2.16	2.16	Partly Cloudy
11	30	210	2000	6000		2.16	2.16	Partly Cloudy
12	30	240	2000	6000		2.16	2.16	Partly Cloudy
13	30	270	2000	6000		2.16	2.16	Partly Cloudy
14	30	300	2000	6000		2.16	2.16	Partly Cloudy
15	30	330	2000	6000		2.16	2.16	Partly Cloudy
16	30	360	2000	6000		2.16	2.16	Partly Cloudy



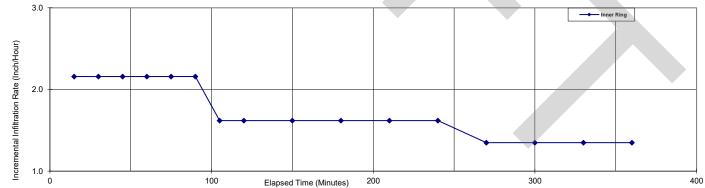
TEST No.:	DRIT-2		PROJECT NAME:		1 1	GENE	RAL SUBSURFACE PF	ROFILE
DATE:	2/1/2018		PROJECT NAME:	Sawgrass Expy 26°18'1.27"N	_	DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'1.27"N 80°13'20.84"W				
LIQUID USED:	Water		STATION:		-			
pH: GROUND	6		OFFSET: GROUND		-			
TEMPERATURE (°F):	70	1	ELEVATION:					
DEPTH TO WATER TAE		N/A						
PENETRATION OF RING		ches):		INNER: INNER:	3 12			6 24
THICKNESS OF RING V				INNER:	0.125			0.125
AREA OF RINGS (Inche	s ^ 2):			INNER:	113.10		ANNULAR:	339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INCREMENT NO.	(MIN.)	TOTAL TIME (MIN.)	INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	REWARKS
0		0						
1	15	15	1750	4500		3.78	3.24	Sunny
2	15	30	1500	4500		3.24	3.24	Sunny
3	15	45	1500	4500		3.24	3.24	Sunny
4	15	60	1500	4500	4	3.24	3.24	Sunny
5	15	75	1500	4250		3.24	3.06	Sunny
6	15	90	1250	4250		2.70	3.06	Sunny
7	15	105	1250	4250		2.70	3.06	Sunny
8	15	120	1250	4250		2.70	3.06	Sunny
9	30	150	2500	8000		2.70	2.88	Sunny
10	30	180	2500	8000		2.70	2.88	Sunny
11	30	210	2500	8000		2.70	2.88	Sunny
12	30	240	2500	8000		2.70	2.88	Sunny
13	30	270	2500	8000		2.70	2.88	Sunny
14	30	300	2250	8000		2.43	2.88	Sunny
15	30	330	2250	8000		2.43	2.88	Sunny
16	30	360	2250	8000		2.43	2.88	Sunny



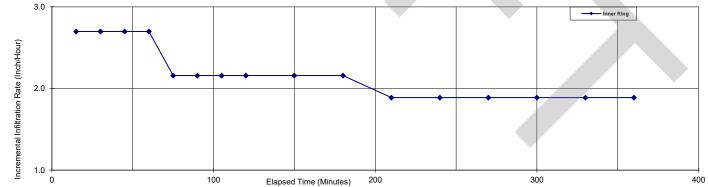
TEST No.:	DRIT-3				1 r	GENE	RAL SUBSURFACE PR	OFILE	
DATE:	1/29/2018		PROJECT NAME:	Sawgrass Expy		DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.	
PID No.:	437153-1-22-01		TEST LOCATION:	26°18'3.40"N 80°12'58.40"W					
IQUID USED:	Water		STATION:	00 12 00.10 11					
H: GROUND	6		OFFSET: GROUND		-				
TEMPERATURE (°F):	72		ELEVATION:						
DEPTH TO WATER THE PENETRATION OF RIN		N/A		INNER:	3		OUTER:	6	
NTERNAL DIAMETER		cries).		INNER:	12			24	
HICKNESS OF RING V				INNER:	0.125			0.125	
REA OF RINGS (Inche	es ^ 2):			INNER:	113.10		ANNULAR:	339.29	
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS	
	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE		
0		0							
1	15	15	1000	2000		2.16	1.44	Sunny	
2	15	30	1000	2000		2.16	1.44	Sunny	
3	15	45	1000	2000	71	2.16	1.44	Sunny	
4	15	60	1000	2000	4	2.16	1.44	Sunny	
5	15	75	1000	2000		2.16	1.44	Sunny	
6	15	90	500	1000		1.08	0.72	Sunny	
7	15	105	500	1000		1.08	0.72	Sunny	
8	15	120	500	1000		1.08	0.72	Sunny	
9	30	150	1000	2000		1.08	0.72	Sunny	
10	30	180	1000	2000		1.08	0.72	Sunny	
11	30	210	1000	2000		1.08	0.72	Sunny	
12	30	240	1000	2000		1.08	0.72	Sunny	
13	30	270	1000	2000		1.08	0.72	Sunny	
14	30	300	1000	2000		1.08	0.72	Sunny	
15	30	330	1000	2000		1.08	0.72	Sunny	
16	30	360	1000	2000		1.08	0.72	Sunny	



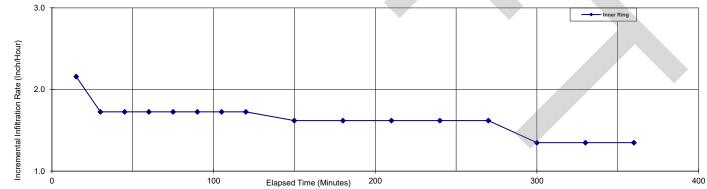
TEST No.:	DRIT-4	1	PROJECT NAME:	Sawgrass Expy	ו ר	GENE	RAL SUBSURFACE PF	ROFILE
DATE:	2/2/2018		PROJECT NAME.	26°18'0.97"N	_	DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	20 10 0.97 N 80°12'58.83"W				
LIQUID USED:	Water		STATION: OFFSET:					
pH: GROUND	ů		GROUND					
TEMPERATURE (°F):	72		ELEVATION:					
DEPTH TO WATER TAB	LÉ (Feet):	N/A						
PENETRATION OF RING		ches):		INNER:	3		OUTER:	6
INTERNAL DIAMETER O THICKNESS OF RING W				INNER: INNER:	12 0.125		OUTER: OUTER:	24 0.125
AREA OF RINGS (Inches				INNER:	113.10		ANNULAR:	339.29
						INCREMENTAL IN	FILTRATION RATE	
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (MI)		(IN/H	OUR)	REMARKS
	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	
0		0						
1	15	15	1000	3500		2.16	2.52	Overcast
2	15	30	1000	3250		2.16	2.34	Overcast
3	15	45	1000	3250		2.16	2.34	Overcast
4	15	60	1000	3000		2.16	2.16	Overcast
5	15	75	1000	3000		2.16	2.16	Overcast
6	15	90	1000	2500		2.16	1.80	Overcast
7	15	105	750	2500		1.62	1.80	Overcast
8	15	120	750	2500		1.62	1.80	Overcast
9	30	150	1500	4750		1.62	1.71	Overcast
10	30	180	1500	4750		1.62	1.71	Overcast
11	30	210	1500	4500		1.62	1.62	Sunny
12	30	240	1500	4500		1.62	1.62	Sunny
13	30	270	1250	4500		1.35	1.62	Sunny
14	30	300	1250	4500		1.35	1.62	Sunny
15	30	330	1250	4500		1.35	1.62	Sunny
16	30	360	1250	4500		1.35	1.62	Sunny



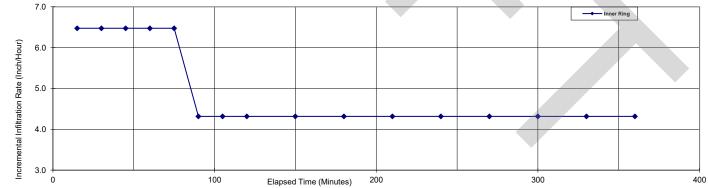
TEST No.: DATE: FPID No.:	DRIT-5 1/29/2018 437153-1-22-01		PROJECT NAME:	Sawgrass Expy 26°18′2.33″N		GENE DEPTH (FEET)	RAL SUBSURFACE PR	ROFILE STRATUM No.
LIQUID USED: pH: GROUND TEMPERATURE (°F):	Water 6 71		STATION: OFFSET: GROUND ELEVATION:	80°12'25.31"W				
DEPTH TO WATER TAB PENETRATION OF RING INTERNAL DIAMETER O THICKNESS OF RING W AREA OF RINGS (Inches	GS INTO GROUND (Ind DF RINGS (Inches): VALL (Inches):	2.5		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INORCEMENT NO.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	
0		0						
1	15	15	1250	3500		2.70	2.52	Overcast
2	15	30	1250	3250		2.70	2.34	Overcast
3	15	45	1250	3250		2.70	2.34	Overcast
4	15	60	1250	3000		2.70	2.16	Overcast
5	15	75	1000	3000		2.16	2.16	Overcast
6	15	90	1000	3000		2.16	2.16	Overcast
7	15	105	1000	3000		2.16	2.16	Overcast
8	15	120	1000	3000		2.16	2.16	Overcast
9	30	150	2000	5500		2.16	1.98	Overcast
10	30	180	2000	5500		2.16	1.98	Overcast
11	30	210	1750	5500		1.89	1.98	Overcast
12	30	240	1750	5500		1.89	1.98	Overcast
13	30	270	1750	5500		1.89	1.98	Overcast
14	30	300	1750	5500		1.89	1.98	Overcast
15	30	330	1750	5500		1.89	1.98	Overcast
16	30	360	1750	5500		1.89	1.98	Overcast



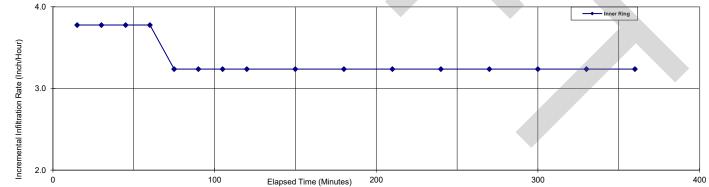
TEST No.: DATE:	DRIT-6		PROJECT NAME:	Sawgrass Expy] [GENE DEPTH (FEET)	RAL SUBSURFACE PR	OFILE STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°17'58.69"N 80°12'20.98"W				
LIQUID USED:	Water		STATION:	80 12 20.98 W				
pH: GROUND	5		OFFSET: GROUND					
TEMPERATURE (°F):	73		ELEVATION:					
DEPTH TO WATER TAB	BLE (Feet):	3.5						
PENETRATION OF RING INTERNAL DIAMETER (THICKNESS OF RING W AREA OF RINGS (Inchest	GS INTO GROUND (Ind DF RINGS (Inches): VALL (Inches):			INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)			FILTRATION RATE	REMARKS
	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	
0		0						
1	15	15	1000	2750		2.16	1.98	Sunny
2	15	30	800	2750		1.73	1.98	Sunny
3	15	45	800	2750		1.73	1.98	Sunny
4	15	60	800	2750		1.73	1.98	Sunny
5	15	75	800	2750		1.73	1.98	Sunny
6	15	90	800	2500		1.73	1.80	Sunny
7	15	105	800	2500		1.73	1.80	Sunny
8	15	120	800	2500		1.73	1.80	Sunny
9	30	150	1500	4500		1.62	1.62	Sunny
10	30	180	1500	4500		1.62	1.62	Sunny
11	30	210	1500	4500		1.62	1.62	Sunny
12	30	240	1500	4250		1.62	1.53	Sunny
13	30	270	1500	4250		1.62	1.53	Sunny
14	30	300	1250	4250		1.35	1.53	Sunny
15	30	330	1250	4250		1.35	1.53	Sunny
16	30	360	1250	4250		1.35	1.53	Sunny



TEST No.: DATE: FPID No.:	DRIT-7 1/25/2018 437153-1-22-01		PROJECT NAME: TEST LOCATION:	Sawgrass Expy 26°18'3.20"N		GENE DEPTH (FEET)	RAL SUBSURFACE PR	ROFILE STRATUM No.
LIQUID USED: pH: GROUND TEMPERATURE (°F):	Water 6 70		STATION: OFFSET: GROUND ELEVATION:	80°12'17.30"W				
DEPTH TO WATER TAB PENETRATION OF RING INTERNAL DIAMETER O THICKNESS OF RING W AREA OF RINGS (Inches	GS INTO GROUND (Ind OF RINGS (Inches): /ALL (Inches):	2.5		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INCREMENT NO.	(MIN.)	TOTAL TIME (MIN.)	INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	REMARKS
0		0						
1	15	15	3000	5000	72	6.47	3.60	Sunny
2	15	30	3000	5000		6.47	3.60	Sunny
3	15	45	3000	5000		6.47	3.60	Sunny
4	15	60	3000	5000		6.47	3.60	Sunny
5	15	75	3000	5000		6.47	3.60	Sunny
6	15	90	2000	4000		4.32	2.88	Sunny
7	15	105	2000	4000		4.32	2.88	Sunny
8	15	120	2000	4000		4.32	2.88	Sunny
9	30	150	4000	8000		4.32	2.88	Sunny
10	30	180	4000	8000		4.32	2.88	Sunny
11	30	210	4000	8000		4.32	2.88	Sunny
12	30	240	4000	8000		4.32	2.88	Sunny
13	30	270	4000	8000		4.32	2.88	Sunny
14	30	300	4000	8000		4.32	2.88	Sunny
15	30	330	4000	8000		4.32	2.88	Sunny
16	30	360	4000	8000		4.32	2.88	Sunny

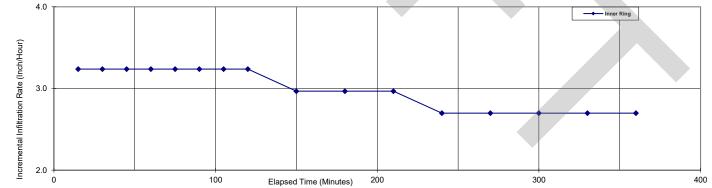


TEST No.:	DRIT-8	1		0 F	1	GENE	RAL SUBSURFACE PR	ROFILE
DATE:	1/25/2018		PROJECT NAME:	Sawgrass Expy		DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'8.63"N 80°12'9.97"W				
LIQUID USED:	Water		STATION:					
pH: GROUND	6		OFFSET: GROUND					
TEMPERATURE (°F):	70		ELEVATION:					
DEPTH TO WATER TAB	BLE (Feet):	N/A						
PENETRATION OF RING		ches):		INNER: INNER:	3			6
INTERNAL DIAMETER O THICKNESS OF RING W				INNER: INNER:	12 0.125			24 0.125
AREA OF RINGS (Inches				INNER:	113.10		ANNULAR:	339.29
			FLOW REAL				FILTRATION RATE	
INCREMENT No.	ELAPSED TIME (MIN.)	TOTAL TIME (MIN.)	FLOW REA		LIQUID TEMPERATURE (°F)	(IN/H	OUR)	REMARKS
	(IVIIIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (F)	INNER RING	ANNULAR SPACE	
0		0			72			
1	15	15	1750	6000		3.78	4.32	Overcast
2	15	30	1750	6000		3.78	4.32	Overcast
3	15	45	1750	5500		3.78	3.96	Overcast
4	15	60	1750	5500		3.78	3.96	Overcast
5	15	75	1500	5500		3.24	3.96	Overcast
6	15	90	1500	5500		3.24	3.96	Overcast
7	15	105	1500	5500		3.24	3.96	Overcast
8	15	120	1500	5500		3.24	3.96	Overcast
9	30	150	3000	10500		3.24	3.78	Overcast
10	30	180	3000	10500		3.24	3.78	Overcast
11	30	210	3000	10000		3.24	3.60	Overcast
12	30	240	3000	10000		3.24	3.60	Overcast
13	30	270	3000	10000		3.24	3.60	Overcast
14	30	300	3000	10000		3.24	3.60	Overcast
15	30	330	3000	10000		3.24	3.60	Overcast
16	30	360	3000	10000		3.24	3.60	Overcast

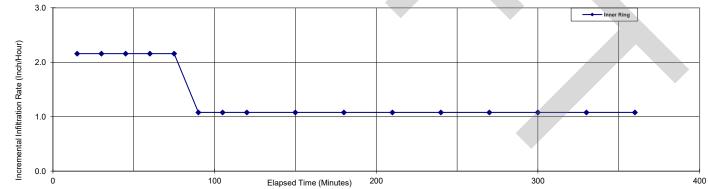


TEST No.:	DRIT-9	1	PROJECT NAME:	Sawgrass Expy	1	GENE	RAL SUBSURFACE PF	ROFILE
DATE:	2/8/2018			26°17'58.34N	-	DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01			26°17'58.34N 80°12'9.69"W				
LIQUID USED:	Water		STATION: OFFSET:		-			
pH: GROUND	5		GROUND					
TEMPERATURE (°F):	74		ELEVATION:					
DEPTH TO WATER TAB	LE (Feet):	N/A						
PENETRATION OF RING		ches):		INNER:	3		OUTER:	6
INTERNAL DIAMETER C THICKNESS OF RING W				INNER: INNER:	12 0.125		OUTER: OUTER:	24 0.125
AREA OF RINGS (Inches				INNER:	113.10		ANNULAR:	339.29
			FLOW REA			INCREMENTAL IN	FILTRATION RATE	
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REAL	DINGS (mi)		(IN/H	IOUR)	REMARKS
	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	
0		0						Sunny
1	15	15	1500	4500		3.24	3.24	Sunny
2	15	30	1500	4500		3.24	3.24	Sunny
3	15	45	1500	4500		3.24	3.24	Sunny
4	15	60	1500	4500		3.24	3.24	Sunny
5	15	75	1500	4500		3.24	3.24	Sunny
6	15	90	1500	4250		3.24	3.06	Sunny
7	15	105	1500	4250		3.24	3.06	Sunny
8	15	120	1500	4250		3.24	3.06	Sunny
9	30	150	2750	8000		2.97	2.88	Sunny
10	30	180	2750	8000		2.97	2.88	Sunny
11	30	210	2750	8000		2.97	2.88	Sunny
12	30	240	2500	8000		2.70	2.88	Sunny
13	30	270	2500	8000		2.70	2.88	Sunny
14	30	300	2500	8000		2.70	2.88	Sunny
15	30	330	2500	8000		2.70	2.88	Sunny
16	30	360	2500	8000		2.70	2.88	Sunny

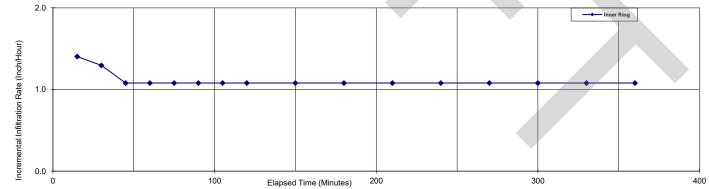




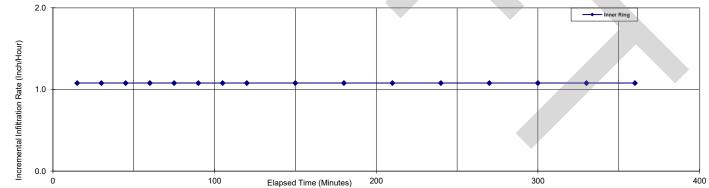
TEST No.: DATE:	DRIT-11 1/24/2018		PROJECT NAME:	Sawgrass Expy] [GENE DEPTH (FEET)	RAL SUBSURFACE PR	OFILE STRATUM No.
FPID No.: LIQUID USED: pH:	437153-1-22-01 Water 6		TEST LOCATION: STATION: OFFSET:	26°18'2.10"N 80°11'57.90"W	-			
GROUND TEMPERATURE (°F):	70		GROUND ELEVATION:] [
DEPTH TO WATER TAB PENETRATION OF RING INTERNAL DIAMETER O THICKNESS OF RING W AREA OF RINGS (Inches	GS INTO GROUND (Ind OF RINGS (Inches): /ALL (Inches):	1.0 1.0		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INCREMENT NO.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	REWIARKS
0		0						
1	15	15	1000	2000	76	2.16	1.44	Sunny
2	15	30	1000	2000		2.16	1.44	Sunny
3	15	45	1000	2000		2.16	1.44	Sunny
4	15	60	1000	2000		2.16	1.44	Sunny
5	15	75	1000	2000		2.16	1.44	Sunny
6	15	90	500	1000		1.08	0.72	Sunny
7	15	105	500	1000		1.08	0.72	Sunny
8	15	120	500	1000		1.08	0.72	Sunny
9	30	150	1000	2000		1.08	0.72	Sunny
10	30	180	1000	2000		1.08	0.72	Sunny
11	30	210	1000	2000		1.08	0.72	Sunny
12	30	240	1000	2000	80	1.08	0.72	Sunny
13	30	270	1000	2000		1.08	0.72	Sunny
14	30	300	1000	2000		1.08	0.72	Sunny
15	30	330	1000	2000		1.08	0.72	Sunny
16	30	360	1000	2000		1.08	0.72	Sunny



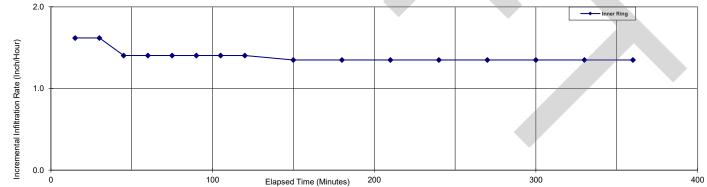
TEST No.: DATE:	DRIT-12 2/9/2018		PROJECT NAME:	Sawgrass Expy 26°17'57.08"N		GENE DEPTH (FEET)	RAL SUBSURFACE PR	OFILE STRATUM No.
FPID No.: LIQUID USED: pH: GROUND	437153-1-22-01 Water 5	-	TEST LOCATION: STATION: OFFSET: GROUND	80°11'57.31"W	-			
TEMPERATURE (°F):	74	ł	ELEVATION:					
DEPTH TO WATER TAE PENETRATION OF RING INTERNAL DIAMETER (THICKNESS OF RING V AREA OF RINGS (Inchest)	GS INTO GROUND (Ind DF RINGS (Inches): VALL (Inches):	3.0 ches):		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INCREMENT NO.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	ILEMAKKS
0		0						
1	15	15	650	2000	76	1.40	1.44	Sunny
2	15	30	600	2000		1.29	1.44	Sunny
3	15	45	500	2000		1.08	1.44	Sunny
4	15	60	500	2000		1.08	1.44	Sunny
5	15	75	500	1750		1.08	1.26	Sunny
6	15	90	500	1750		1.08	1.26	Sunny
7	15	105	500	1750		1.08	1.26	Sunny
8	15	120	500	1750		1.08	1.26	Sunny
9	30	150	1000	3250		1.08	1.17	Sunny
10	30	180	1000	3250		1.08	1.17	Sunny
11	30	210	1000	3250		1.08	1.17	Sunny
12	30	240	1000	3250	80	1.08	1.17	Sunny
13	30	270	1000	3250		1.08	1.17	Sunny
14	30	300	1000	3250		1.08	1.17	Sunny
15	30	330	1000	3250		1.08	1.17	Sunny
16	30	360	1000	3250		1.08	1.17	Sunny



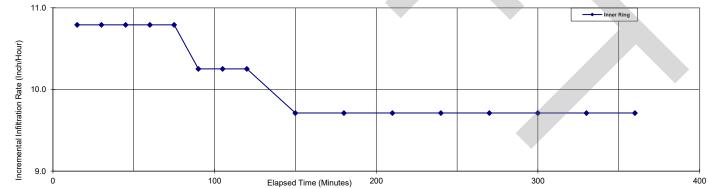
TEST No.: DATE:	DRIT-13 1/24/2018		PROJECT NAME:	Sawgrass Expy 26°18'3.06"N		GENE DEPTH (FEET)	RAL SUBSURFACE PR	ROFILE STRATUM №.
FPID No.: LIQUID USED: pH: GROUND TEMPERATURE (°F):	437153-1-22-01 Water 6 71		TEST LOCATION: STATION: OFFSET: GROUND ELEVATION:	80°11'21.91"W				
DEPTH TO WATER TAB PENETRATION OF RING INTERNAL DIAMETER O THICKNESS OF RING W AREA OF RINGS (Inchest	GS INTO GROUND (Ind DF RINGS (Inches): VALL (Inches):	1.0		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
indicement no.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	REMARKO
0		0			72			
1	15	15	500	1750		1.08	1.26	Overcast
2	15	30	500	1750		1.08	1.26	Overcast
3	15	45	500	1750		1.08	1.26	Overcast
4	15	60	500	1750		1.08	1.26	Overcast
5	15	75	500	1500		1.08	1.08	Overcast
6	15	90	500	1500		1.08	1.08	Overcast
7	15	105	500	1500		1.08	1.08	Overcast
8	15	120	500	1500		1.08	1.08	Overcast
9	30	150	1000	3000		1.08	1.08	Overcast
10	30	180	1000	3000		1.08	1.08	Overcast
11	30	210	1000	3000		1.08	1.08	Overcast
12	30	240	1000	3000		1.08	1.08	Overcast
13	30	270	1000	3000		1.08	1.08	Overcast
14	30	300	1000	3000		1.08	1.08	Overcast
15	30	330	1000	3000		1.08	1.08	Overcast
16	30	360	1000	3000		1.08	1.08	Overcast



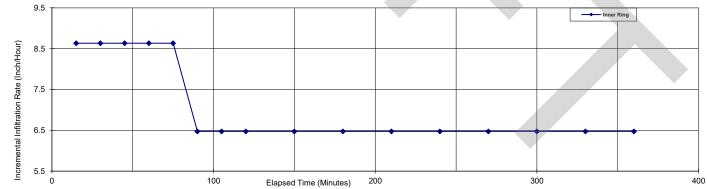
TEST No.:	DRIT-14	1		0	ז ר	GENE	RAL SUBSURFACE PF	OFILE
DATE:	2/12/2018		PROJECT NAME:	Sawgrass Expy		DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'0.01"N 80°11'19.30"W				
LIQUID USED:	Water		STATION:					
pH: GROUND	6		OFFSET: GROUND		-			
TEMPERATURE (°F):	76		ELEVATION:		J			
DEPTH TO WATER TAB	ILE (Feet):	N/A						
PENETRATION OF RING	GS INTO GROUND (Ind	ches):		INNER:	3			6
INTERNAL DIAMETER OF THICKNESS OF RING W				INNER: INNER:	12 0.125			24 0.125
AREA OF RINGS (Inches				INNER:	113.10			339.29
						INCREMENTAL IN	FILTRATION RATE	
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID	(IN/H	OUR)	REMARKS
	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	
0		0						
1	15	15	750	2750		1.62	1.98	Sunny
2	15	30	750	2750		1.62	1.98	Sunny
3	15	45	650	2750		1.40	1.98	Sunny
4	15	60	650	2750		1.40	1.98	Sunny
5	15	75	650	2500		1.40	1.80	Sunny
6	15	90	650	2500		1.40	1.80	Sunny
7	15	105	650	2400		1.40	1.73	Sunny
8	15	120	650	2400		1.40	1.73	Sunny
9	30	150	1250	4500		1.35	1.62	Sunny
10	30	180	1250	4500		1.35	1.62	Sunny
11	30	210	1250	4500		1.35	1.62	Sunny
12	30	240	1250	4500		1.35	1.62	Sunny
13	30	270	1250	4500		1.35	1.62	Sunny
14	30	300	1250	4500		1.35	1.62	Sunny
15	30	330	1250	4500		1.35	1.62	Sunny
16	30	360	1250	4500		1.35	1.62	Sunny



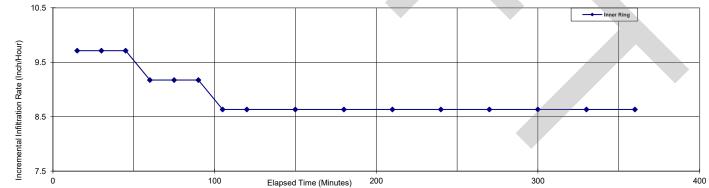
TEST No.: DATE:	DRIT-15 2/13/2018		PROJECT NAME:	Sawgrass Expy]	GENE DEPTH (FEET)	RAL SUBSURFACE PR	ROFILE STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'0.49"N	-			on the low red.
LIQUID USED:	Water		STATION:	80°11'12.96"W	-			
pH:	6		OFFSET:					
GROUND TEMPERATURE (°F):	76		GROUND ELEVATION:					
DEPTH TO WATER TAE PENETRATION OF RING INTERNAL DIAMETER (THICKNESS OF RING V AREA OF RINGS (Inche	GS INTO GROUND (In OF RINGS (Inches): VALL (Inches):	5.0 5.0		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INCIVEMENT NO.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	ILLIMAILING
0		0						
1	15	15	5000	14750		10.79	10.61	Sunny
2	15	30	5000	14500		10.79	10.43	Sunny
3	15	45	5000	14500		10.79	10.43	Sunny
4	15	60	5000	14500		10.79	10.43	Sunny
5	15	75	5000	14500		10.79	10.43	Sunny
6	15	90	4750	14000		10.25	10.07	Sunny
7	15	105	4750	14000		10.25	10.07	Sunny
8	15	120	4750	14000		10.25	10.07	Sunny
9	30	150	9000	27750		9.71	9.98	Sunny
10	30	180	9000	27750		9.71	9.98	Sunny
11	30	210	9000	27750		9.71	9.98	Sunny
12	30	240	9000	27500		9.71	9.89	Sunny
13	30	270	9000	27500		9.71	9.89	Sunny
14	30	300	9000	27500		9.71	9.89	Sunny
15	30	330	9000	27500		9.71	9.89	Sunny
16	30	360	9000	27500		9.71	9.89	Sunny



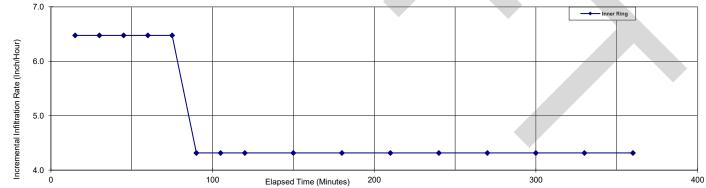
TEST No.: DATE:	DRIT-16 1/23/2018		PROJECT NAME:	Sawgrass Expy] [GENE DEPTH (FEET)	RAL SUBSURFACE PR	OFILE STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'8.40"N				onteriowine.
LIQUID USED:	Water		STATION:	80°11'9.70"W	-			
pH:	6		OFFSET:					
GROUND TEMPERATURE (°F):	72		GROUND ELEVATION:					
					-			
DEPTH TO WATER TAE PENETRATION OF RING INTERNAL DIAMETER (THICKNESS OF RING V AREA OF RINGS (Inchest	GS INTO GROUND (Ind DF RINGS (Inches): VALL (Inches):	3.5 ches):		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
INCREMENT No.	ELAPSED TIME	TOTAL TIME (MIN.)	FLOW REA	DINGS (ml)	LIQUID		FILTRATION RATE	REMARKS
INCICEMENT NO.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	ILLIMATING
0		0						
1	15	15	4000	12000		8.63	8.63	Sunny
2	15	30	4000	12000		8.63	8.63	Sunny
3	15	45	4000	12000		8.63	8.63	Sunny
4	15	60	4000	12000		8.63	8.63	Sunny
5	15	75	4000	12000		8.63	8.63	Sunny
6	15	90	3000	10000		6.47	7.19	Sunny
7	15	105	3000	10000		6.47	7.19	Sunny
8	15	120	3000	10000		6.47	7.19	Sunny
9	30	150	6000	20000		6.47	7.19	Sunny
10	30	180	6000	20000		6.47	7.19	Sunny
11	30	210	6000	20000		6.47	7.19	Sunny
12	30	240	6000	20000		6.47	7.19	Sunny
13	30	270	6000	20000		6.47	7.19	Sunny
14	30	300	6000	20000		6.47	7.19	Sunny
15	30	330	6000	20000		6.47	7.19	Sunny
16	30	360	6000	20000		6.47	7.19	Sunny



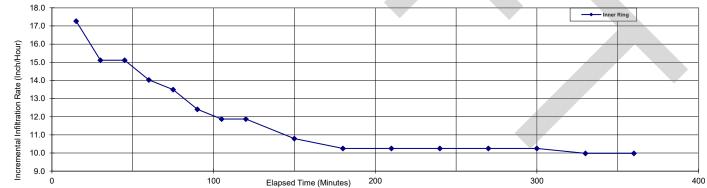
TEST No.:	DRIT-17	1	PROJECT NAME:	Sawgrass Expy	1	GENE	ERAL SUBSURFACE PROFILE	
DATE:	2/14/2018		PROJECT MAINE.	26°18'5.7"N		DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	80°11'4.83"W				
LIQUID USED:	Water 6		STATION: OFFSET:		-			
pH: GROUND			GROUND		-			
TEMPERATURE (°F):	75	1	ELEVATION:					
DEPTH TO WATER TAE		N/A						
PENETRATION OF RING		ches):		INNER: INNER:	3 12			6 24
THICKNESS OF RING V				INNER:	0.125			0.125
AREA OF RINGS (Inches	s ^ 2):			INNER:	113.10		ANNULAR:	339.29
	ELAPSED TIME		FLOW READINGS (ml)		LIQUID	INCREMENTAL INFILTRATION RATE (IN/HOUR)		
INCREMENT No.	(MIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	REMARKS
0		0						
1	15	15	4500	13750		9.71	9.89	Sunny
2	15	30	4500	13750		9.71	9.89	Sunny
3	15	45	4500	13750		9.71	9.89	Sunny
4	15	60	4250	13750		9.17	9.89	Sunny
5	15	75	4250	13750		9.17	9.89	Sunny
6	15	90	4250	13500		9.17	9.71	Sunny
7	15	105	4000	13500		8.63	9.71	Sunny
8	15	120	4000	13500		8.63	9.71	Sunny
9	30	150	8000	26000		8.63	9.35	Sunny
10	30	180	8000	26000		8.63	9.35	Sunny
11	30	210	8000	26000		8.63	9.35	Sunny
12	30	240	8000	25500		8.63	9.17	Sunny
13	30	270	8000	25500		8.63	9.17	Sunny
14	30	300	8000	25500		8.63	9.17	Sunny
15	30	330	8000	25500		8.63	9.17	Sunny
16	30	360	8000	25500		8.63	9.17	Sunny



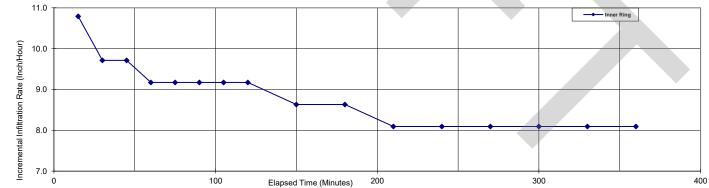
EST No.:	DRIT-18	a			ı r	GENERAL SUBSURFACE PROFILE		
DATE:	1/22/2018		PROJECT NAME:	Sawgrass Expy		DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
PID No.:	437153-1-22-01		TEST LOCATION:	26°18'14.12"N 80° 9'4.65"W				
IQUID USED:	Water		STATION:					
H: GROUND	6		OFFSET: GROUND		-			
EMPERATURE (°F):	70		ELEVATION:					
EPTH TO WATER TAB	LE (East):	N/A						
PENETRATION OF RING	GS INTO GROUND (In			INNER:	3			6
NTERNAL DIAMETER C				INNER:	12			24
HICKNESS OF RING W REA OF RINGS (Inches				INNER: INNER:	0.125 113.10			0.125 339.29
INCA OF ININGS (Inches	5 2).			INNER.	115.10			555.25
INCREMENT No.	ELAPSED TIME (MIN.)	TOTAL TIME (MIN.)	FLOW READINGS (ml)		LIQUID	INCREMENTAL INFILTRATION RATE (IN/HOUR)		REMARKS
INORCEMENT NO.			INNER RING	ANNULAR SPACE	TEMPERATURE (°F)	INNER RING	ANNULAR SPACE	
0		0						Sunny
1	15	15	3000	6000		6.47	4.32	Sunny
2	15	30	3000	6000		6.47	4.32	Sunny
3	15	45	3000	6000		6.47	4.32	Sunny
4	15	60	3000	6000	75	6.47	4.32	Sunny
5	15	75	3000	6000		6.47	4.32	Sunny
6	15	90	2000	5000		4.32	3.60	Sunny
7	15	105	2000	5000		4.32	3.60	Sunny
8	15	120	2000	5000		4.32	3.60	Sunny
9	30	150	4000	10000		4.32	3.60	Sunny
10	30	180	4000	10000		4.32	3.60	Sunny
11	30	210	4000	10000	78	4.32	3.60	Sunny
12	30	240	4000	10000		4.32	3.60	Sunny
13	30	270	4000	10000		4.32	3.60	Sunny
	30	300	4000	10000		4.32	3.60	Sunny
14								
14 15	30	330	4000	10000		4.32	3.60	Sunny



TEST No.:	DRIT-19	1	PROJECT NAME: Sawgrass Expy			GENE	RAL SUBSURFACE PR	ROFILE
DATE:	1/23/2018		PROJECT MAINE.	26°18'16.36"N	-	DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
PID No.:	437153-1-22-01		TEST LOCATION:	26°18'16.36"N 80°11'52.04"W				
IQUID USED:	Water		STATION:					
H: GROUND	6		OFFSET: GROUND					
TEMPERATURE (°F):	71		ELEVATION:		J			
DEPTH TO WATER TAE DENETRATION OF RING NTERNAL DIAMETER (HICKNESS OF RING V REA OF RINGS (Inche	GS INTO GROUND (In OF RINGS (Inches): VALL (Inches):	N/A ches):		INNER: INNER: INNER: INNER:	3 12 0.125 113.10		OUTER: OUTER:	6 24 0.125 339.29
		TOTAL TIME (MIN.)	FLOW READINGS (ml)			INCREMENTAL INFILTRATION RATE		
INCREMENT No.	ELAPSED TIME (MIN.)				LIQUID TEMPERATURE (°F)	•	IOUR)	REMARKS
	(IVIIN.)		INNER RING	ANNULAR SPACE	TEMPERATORE (T)	INNER RING	ANNULAR SPACE	
0		0						
1	15	15	8000	26000		17.27	18.71	Sunny
2	15	30	7000	26000		15.11	18.71	Sunny
3	15	45	7000	24500		15.11	17.63	Sunny
4	15	60	6500	24000		14.03	17.27	Sunny
5	15	75	6250	23000		13.49	16.55	Sunny
6	15	90	5750	22500		12.41	16.19	Sunny
7	15	105	5500	22500		11.87	16.19	Sunny
8	15	120	5500	21500		11.87	15.47	Sunny
9	30	150	10000	39000		10.79	14.03	Sunny
10	30	180	9500	38000		10.25	13.67	Sunny
11	30	210	9500	38000		10.25	13.67	Sunny
12	30	240	9500	37000		10.25	13.31	Sunny
13	30	270	9500	37000		10.25	13.31	Sunny
14	30	300	9500	37000		10.25	13.31	Sunny
15	30	330	9250	37000		9.98	13.31	Sunny
16	30	360	9250	37000		9.98	13.31	Sunny



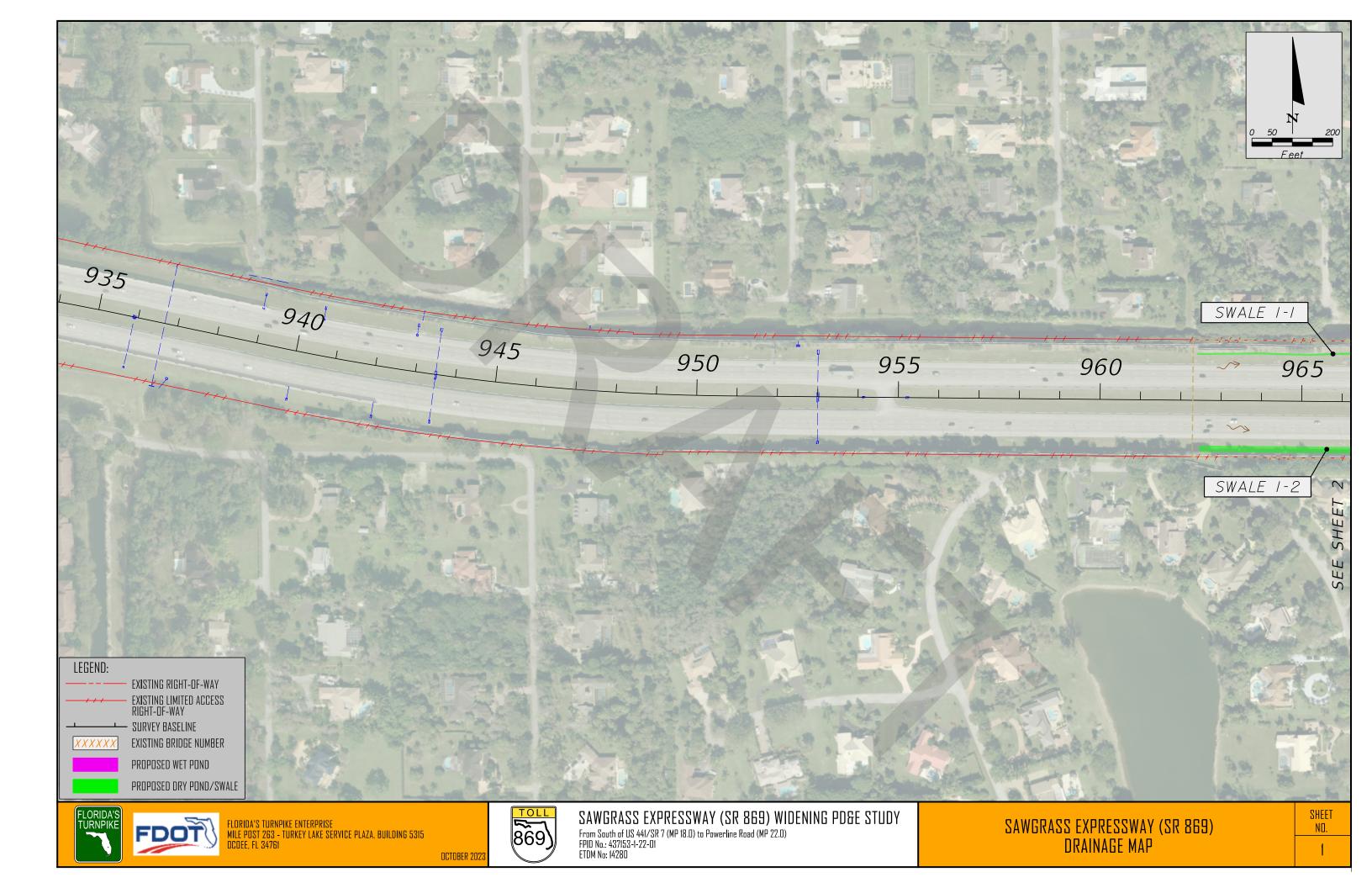
TEST No.:	DRIT-20	1	PROJECT NAME:	0 F	ו ר	GENE	ERAL SUBSURFACE PROFILE	
DATE:	1/22/2018		PROJECT NAME:	Sawgrass Expy	_	DEPTH (FEET)	SOIL DESCRIPTION	STRATUM No.
FPID No.:	437153-1-22-01		TEST LOCATION:	26°18'13.43"N 80°8'48.26"W				
LIQUID USED:	Water		STATION: OFFSET:		-			
pH: GROUND			GROUND		-			
TEMPERATURE (°F):	70	1	ELEVATION:					
DEPTH TO WATER TAE		N/A						
PENETRATION OF RING		ches):		INNER: INNER:	3 12			6 24
THICKNESS OF RING V				INNER:	0.125			0.125
AREA OF RINGS (Inche	s ^ 2):			INNER:	113.10		ANNULAR:	339.29
		TOTAL TIME (MIN.)	FLOW READINGS (ml)			INCREMENTAL INFILTRATION RATE		REMARKS
INCREMENT No.	ELAPSED TIME (MIN.)				LIQUID TEMPERATURE (°F)	(IN/HOUR)		
	(IVIIN.)		INNER RING	ANNULAR SPACE	TEMPERATURE (F)	INNER RING	ANNULAR SPACE	
0		0						
1	15	15	5000	17000		10.79	12.23	Sunny
2	15	30	4500	17000		9.71	12.23	Sunny
3	15	45	4500	16500		9.71	11.87	Sunny
4	15	60	4250	16000		9.17	11.51	Sunny
5	15	75	4250	15750		9.17	11.33	Sunny
6	15	90	4250	15250		9.17	10.97	Sunny
7	15	105	4250	15250		9.17	10.97	Sunny
8	15	120	4250	15000		9.17	10.79	Sunny
9	30	150	8000	29000		8.63	10.43	Sunny
10	30	180	8000	28000		8.63	10.07	Sunny
11	30	210	7500	28000		8.09	10.07	Sunny
12	30	240	7500	27500		8.09	9.89	Sunny
13	30	270	7500	27500		8.09	9.89	Sunny
14	30	300	7500	27000		8.09	9.71	Sunny
15	30	330	7500	27000		8.09	9.71	Sunny
16	30	360	7500	27000		8.09	9.71	Sunny

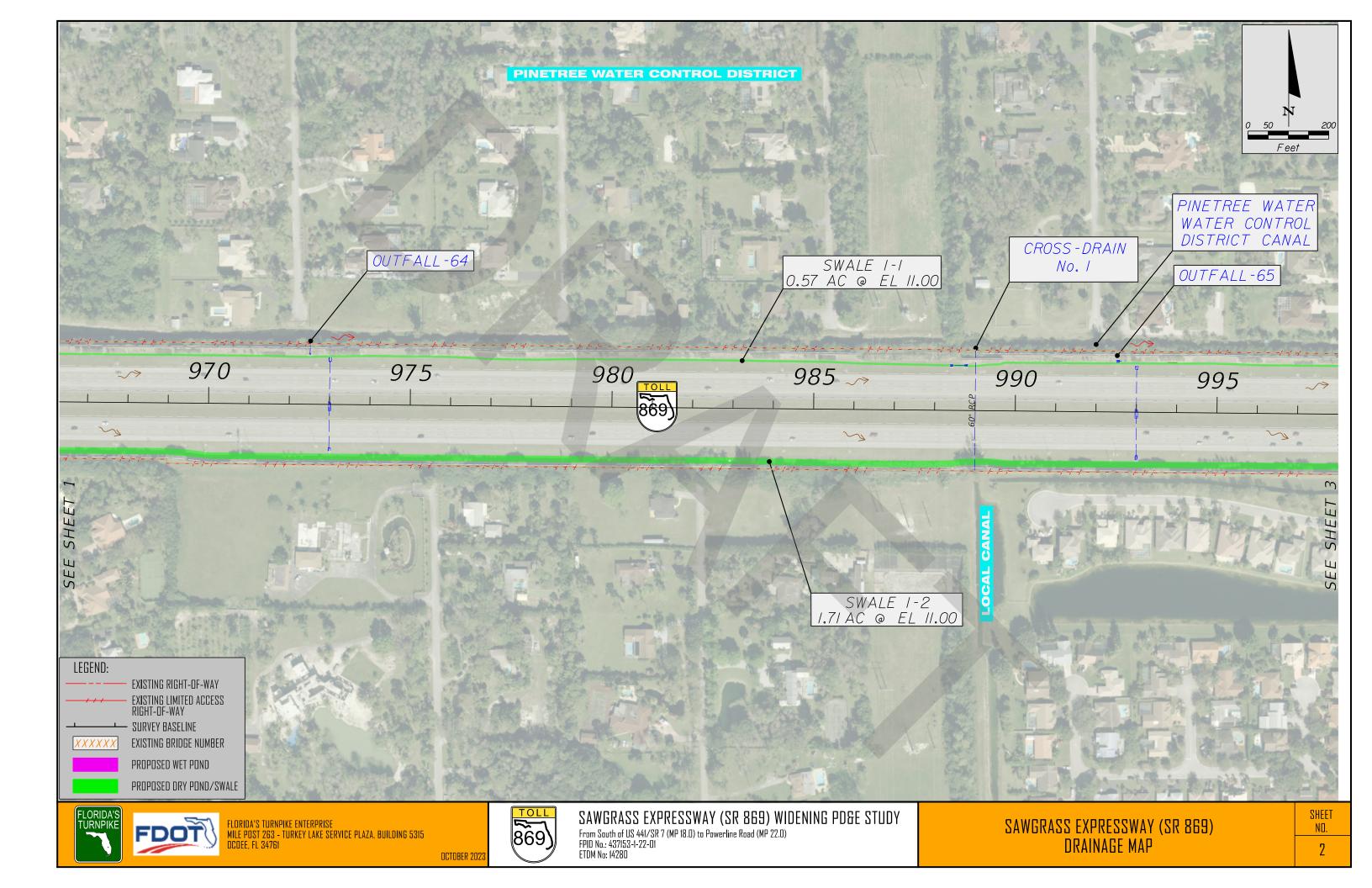


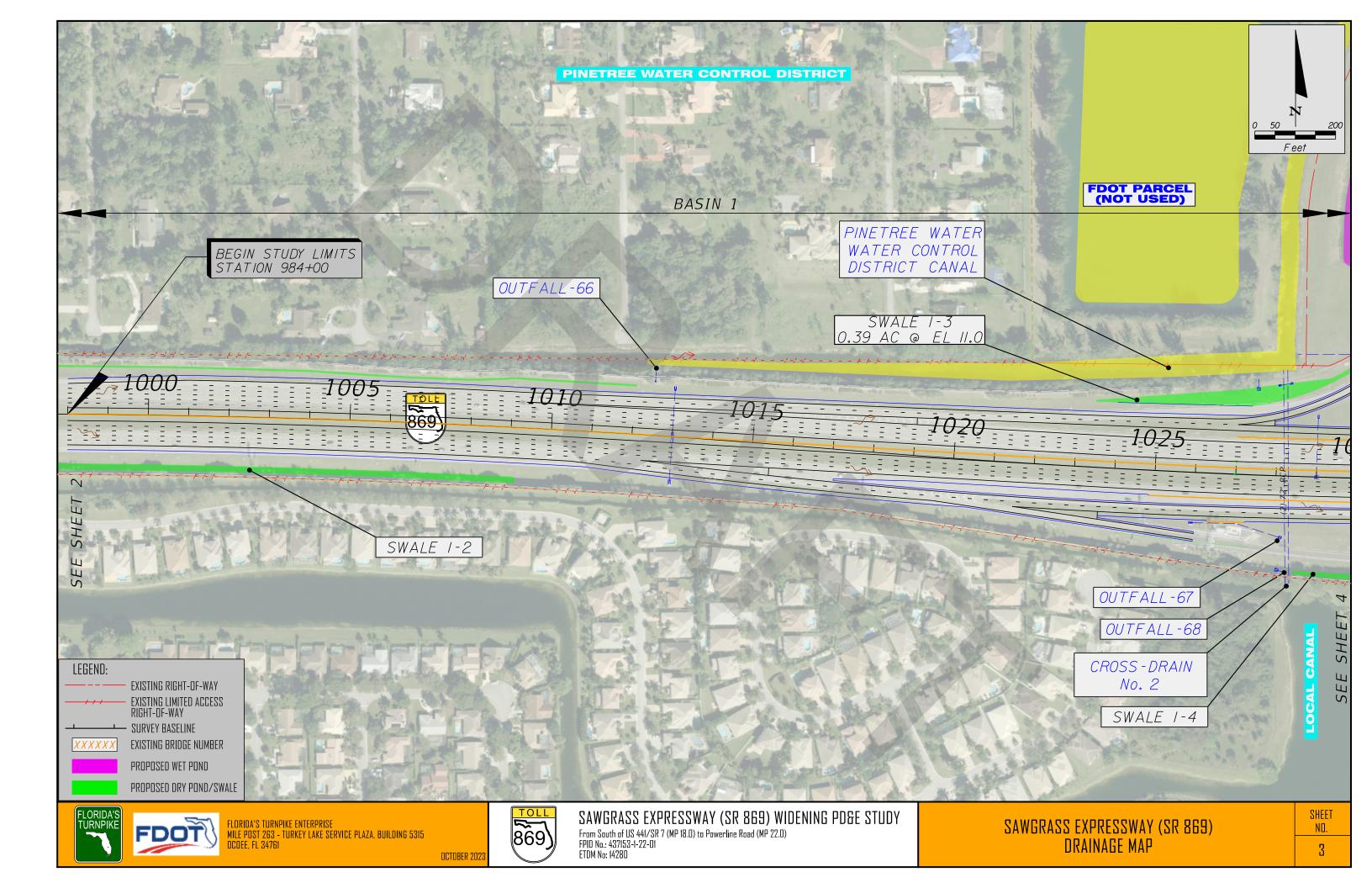
Draft Pond Siting Report

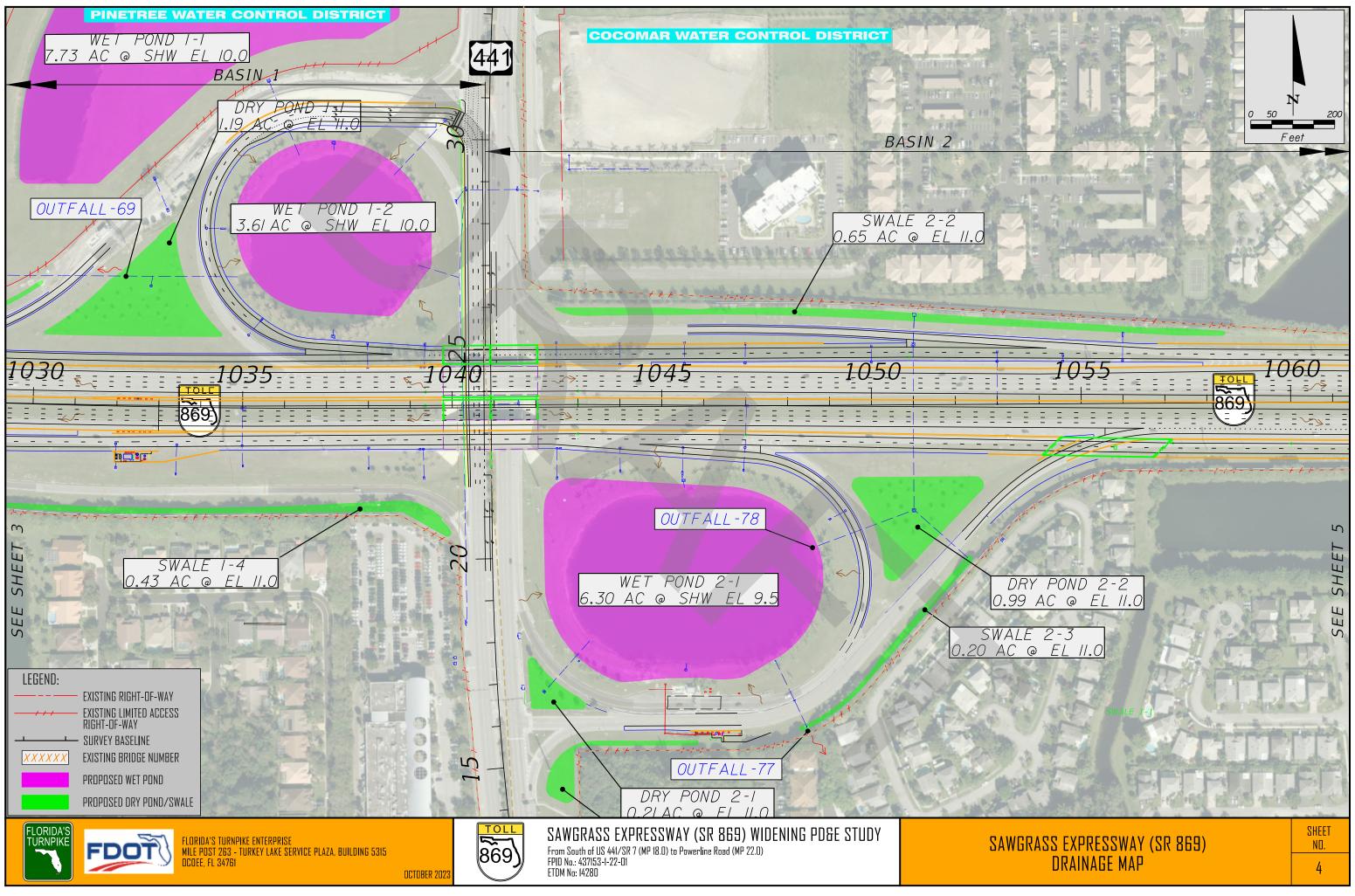
APPENDIX C

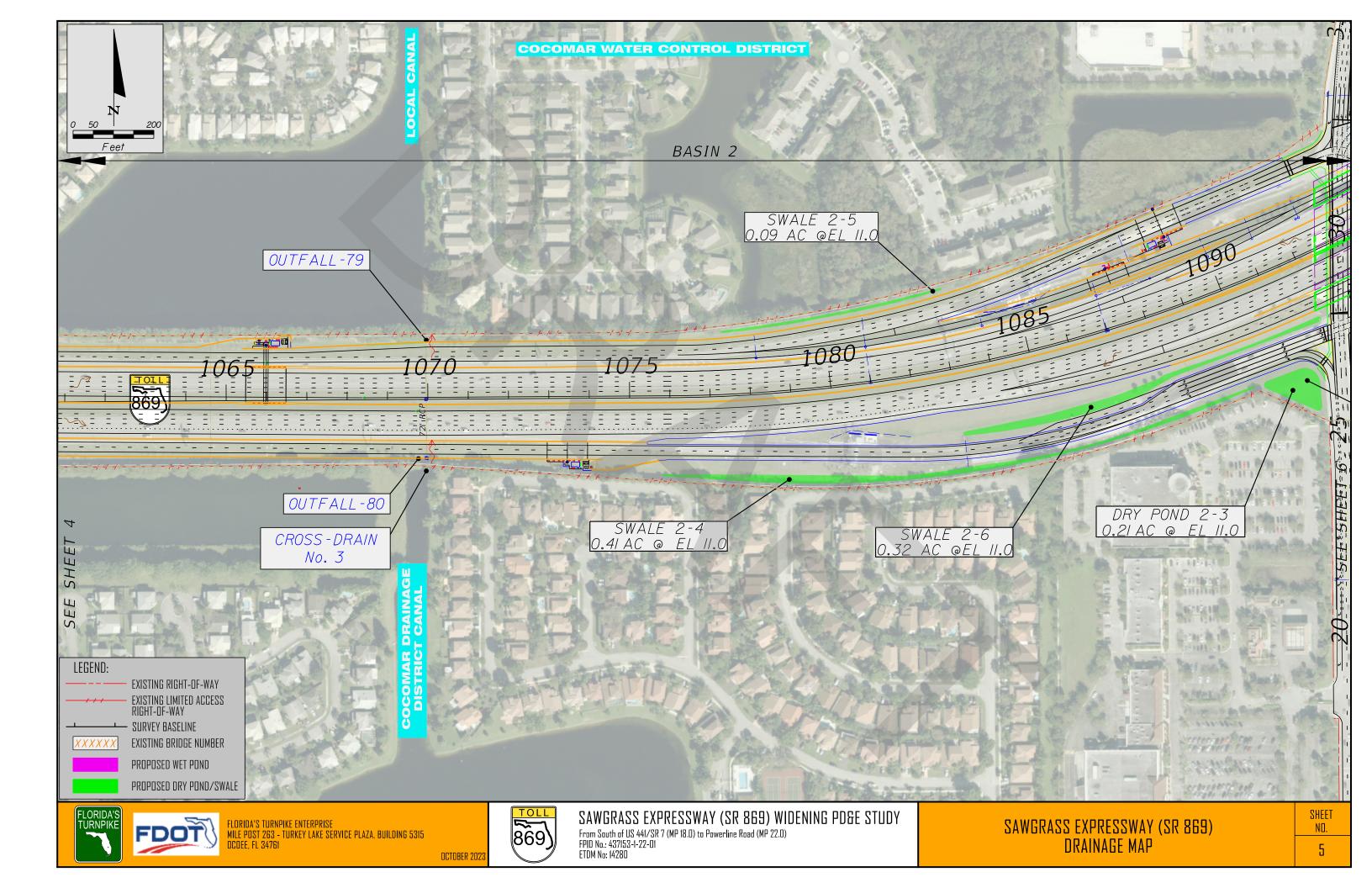
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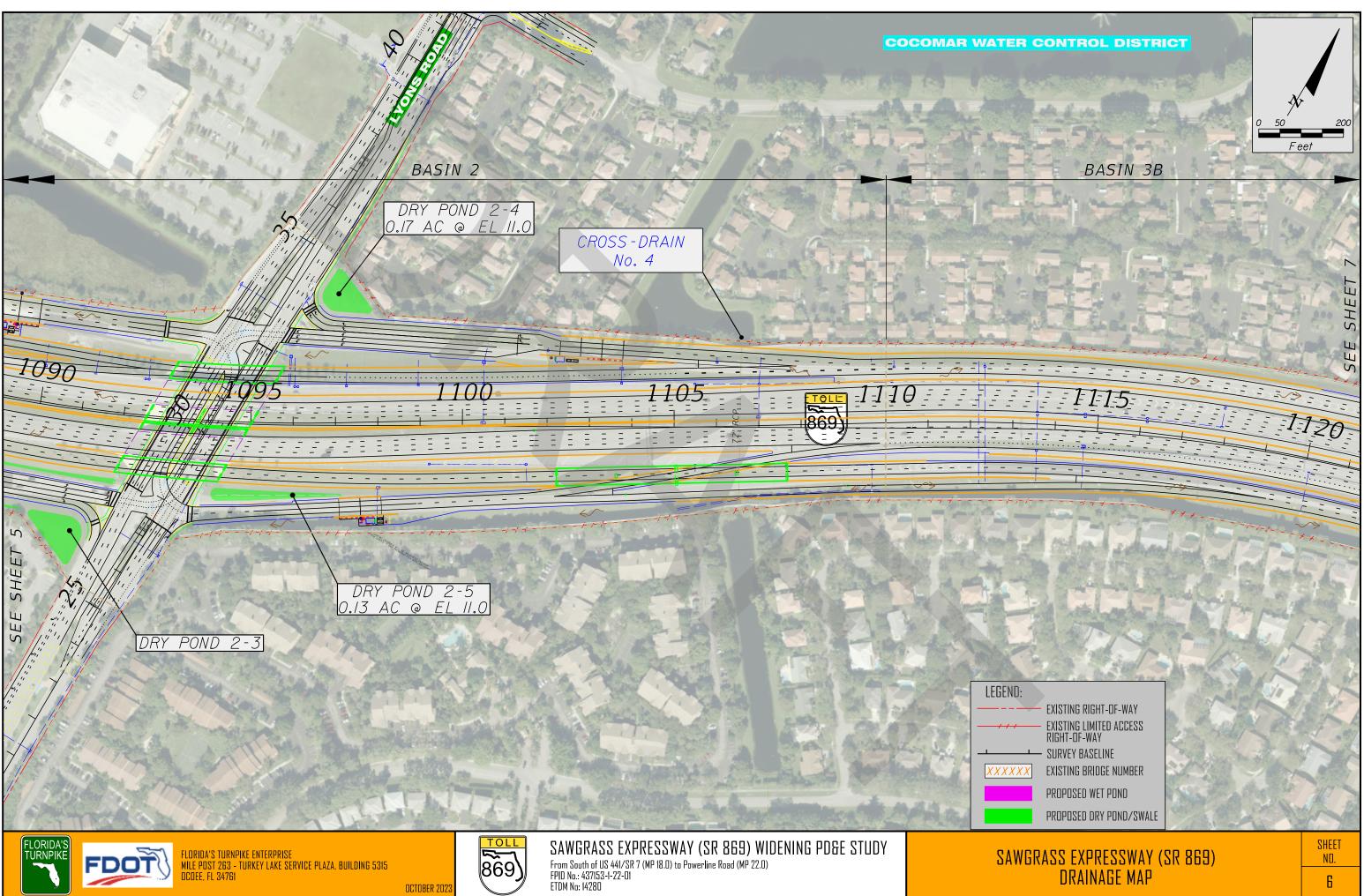




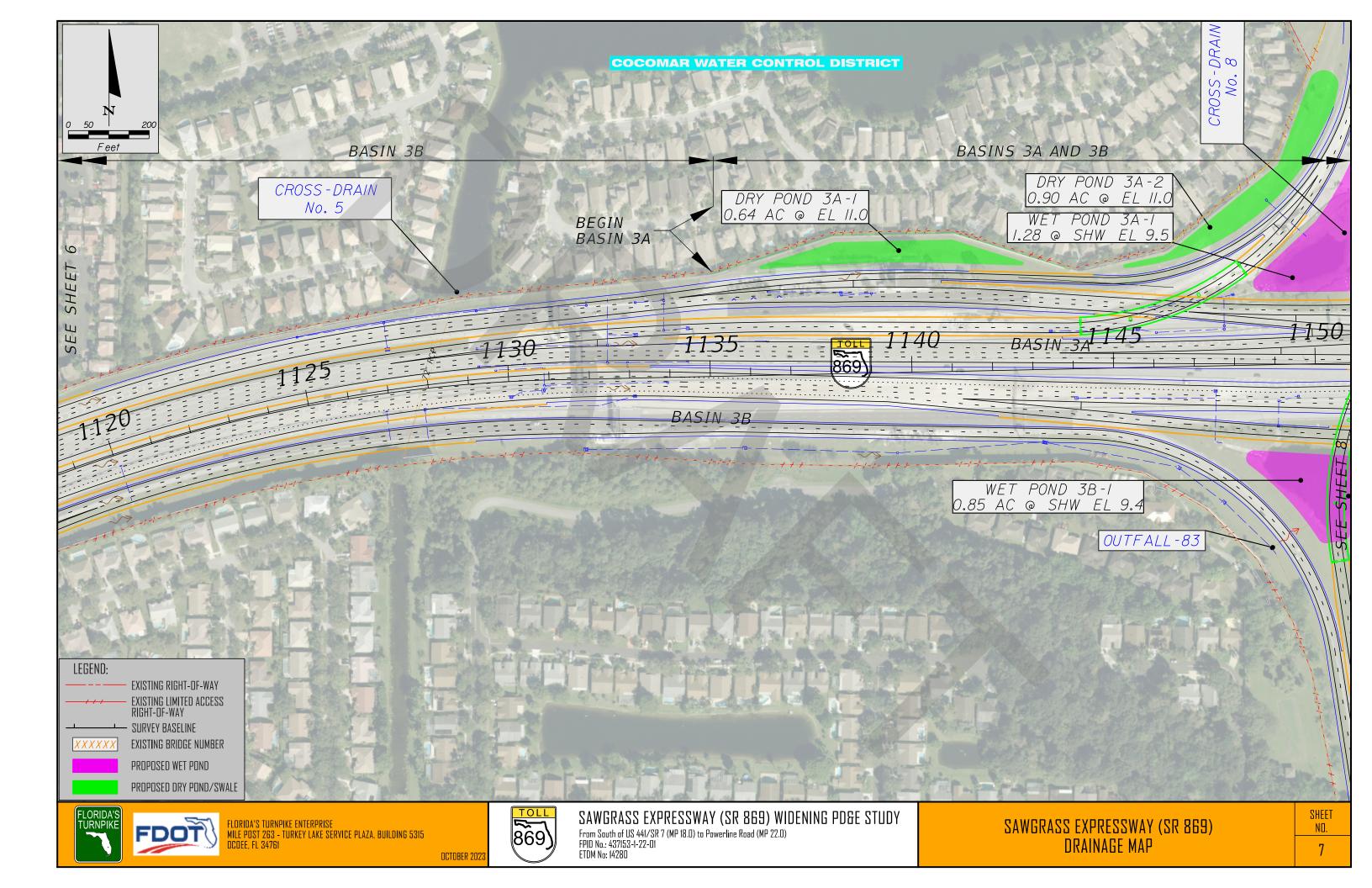


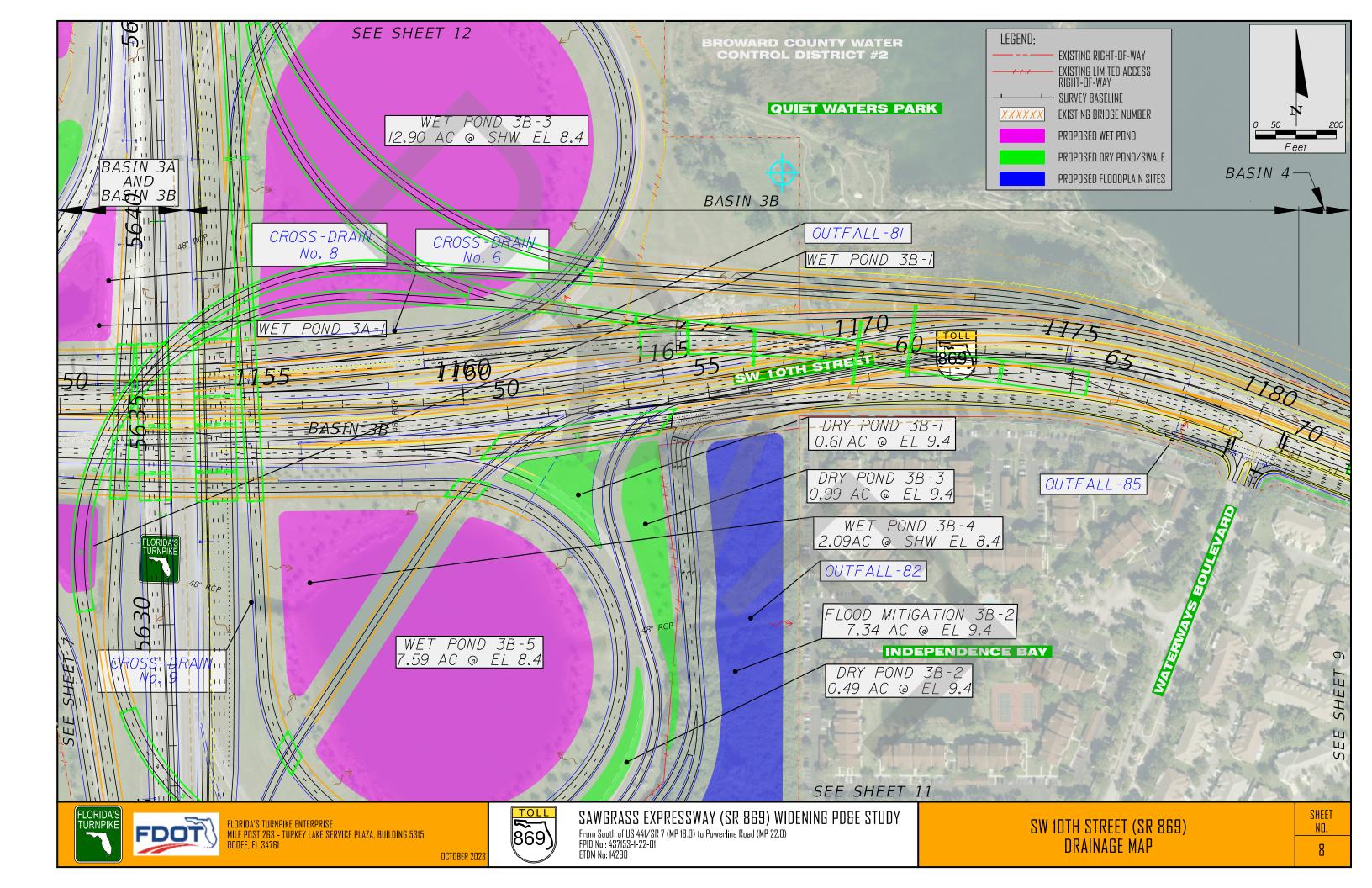


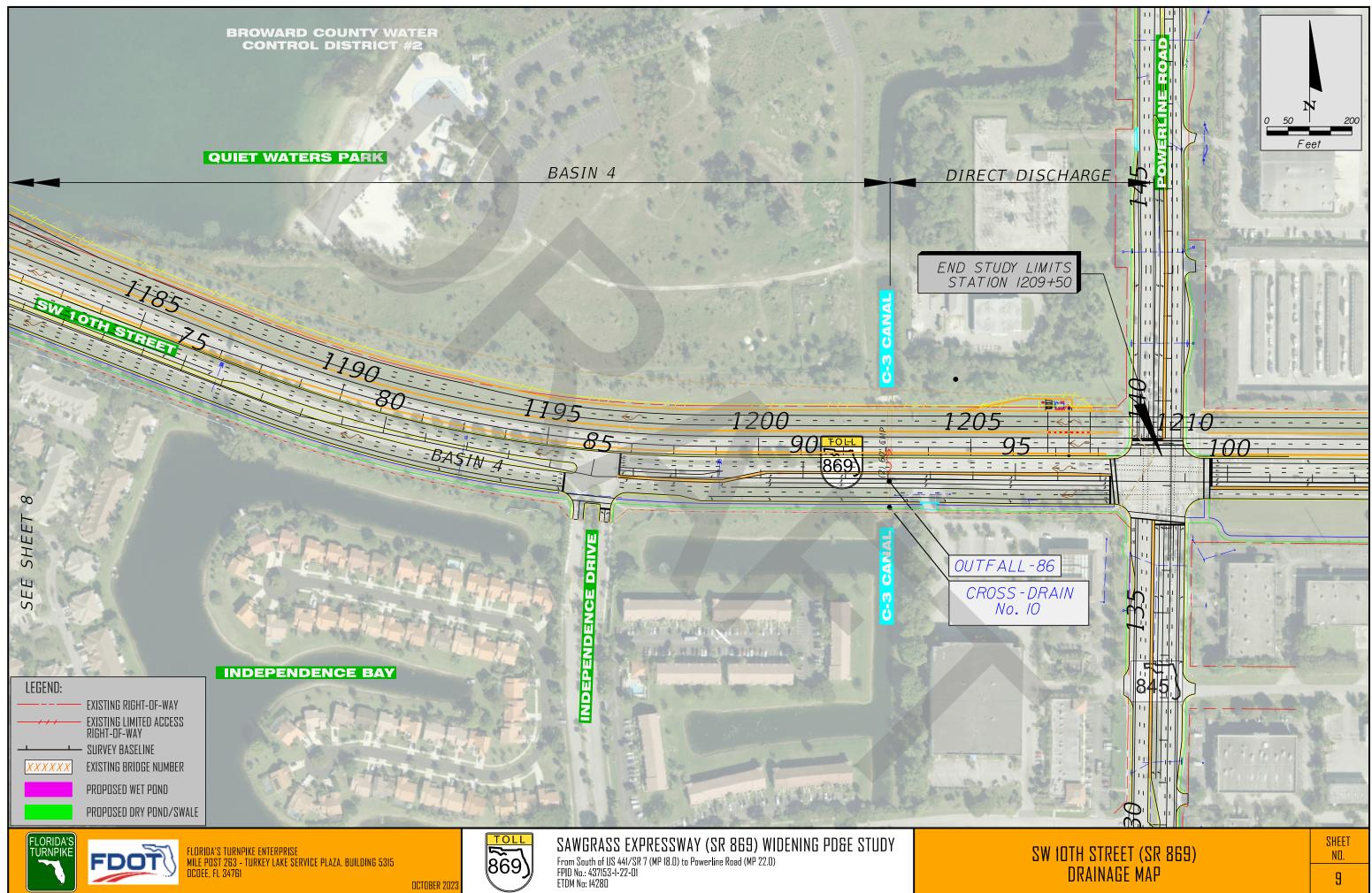


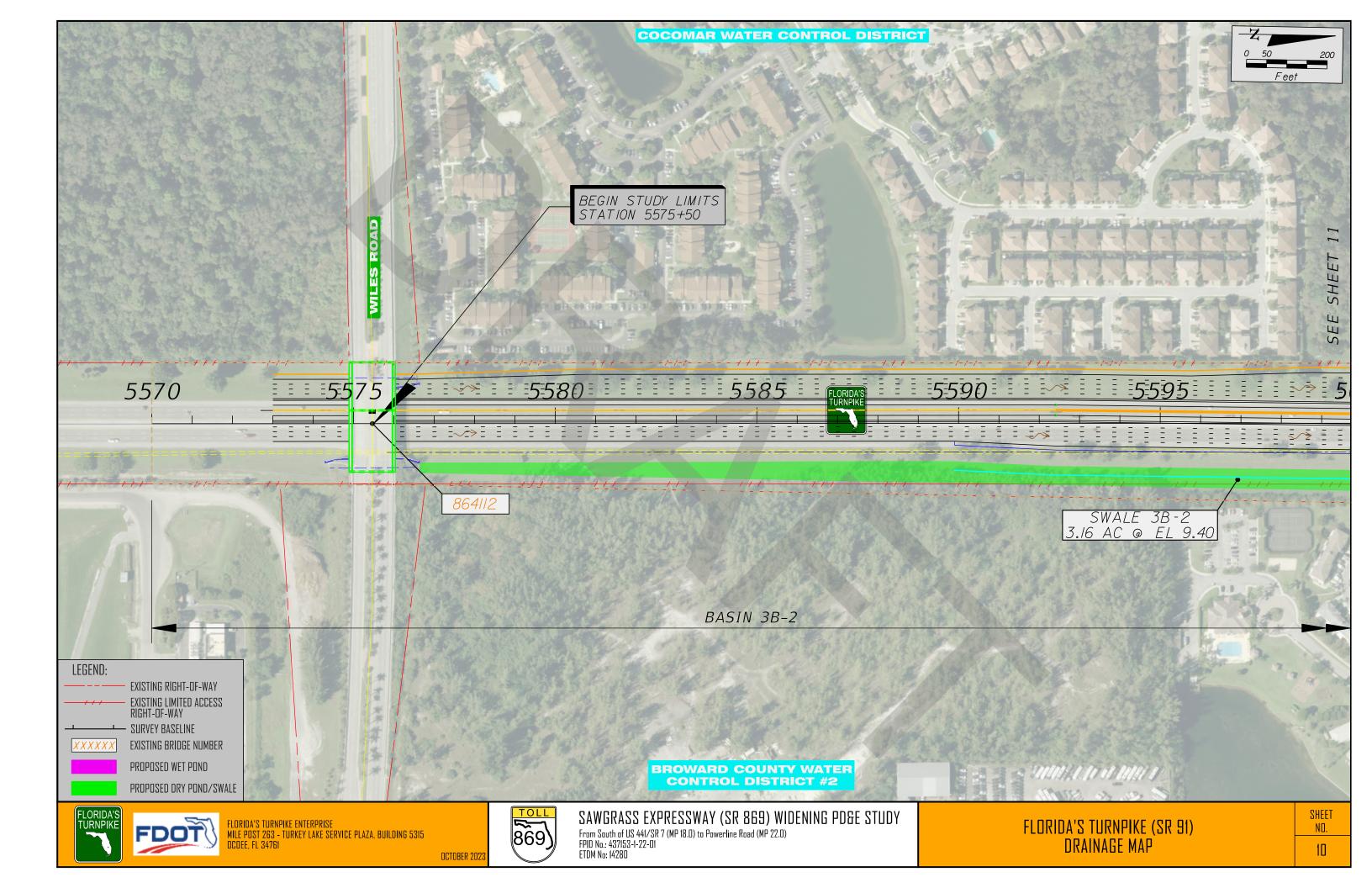


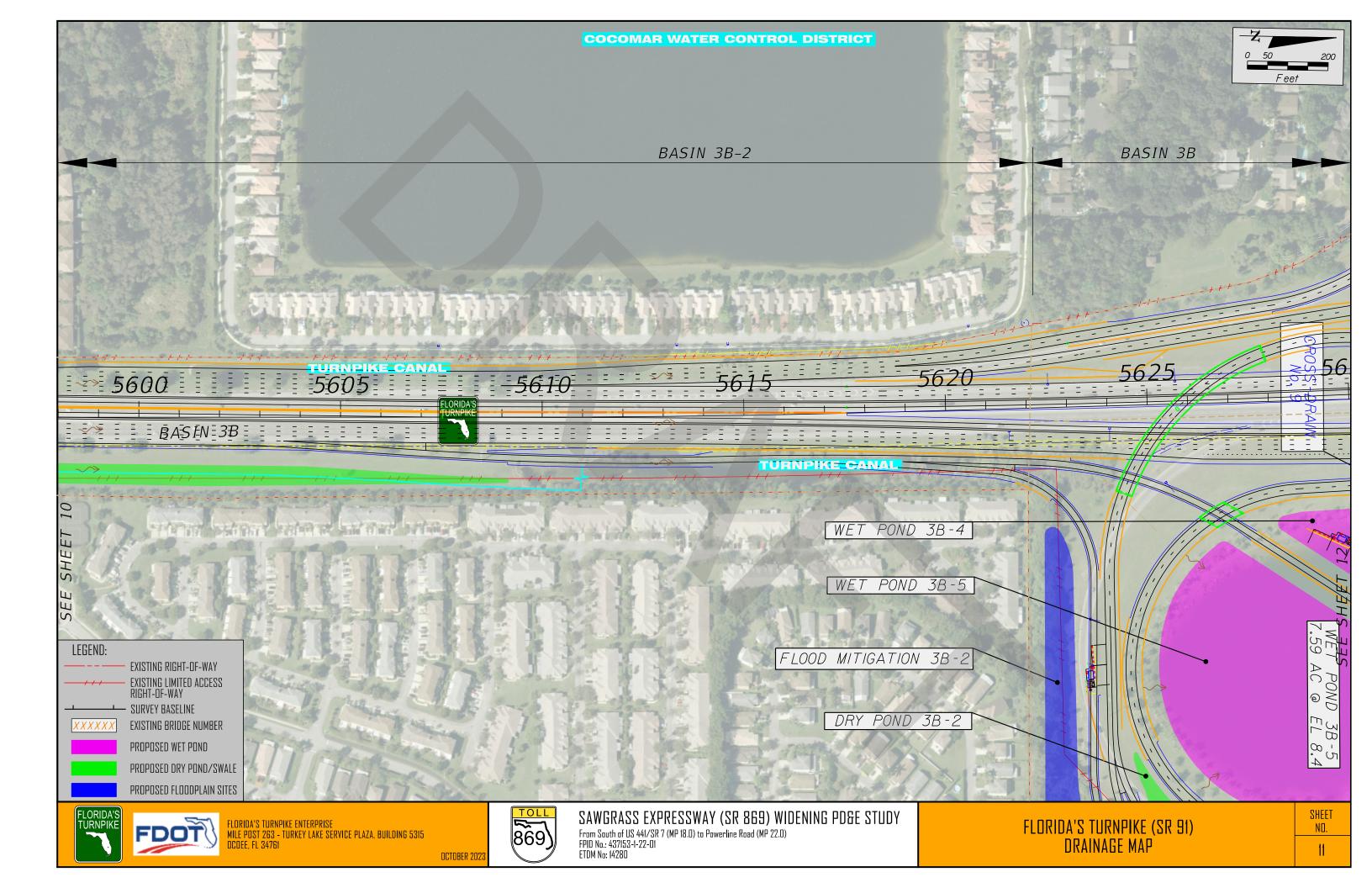
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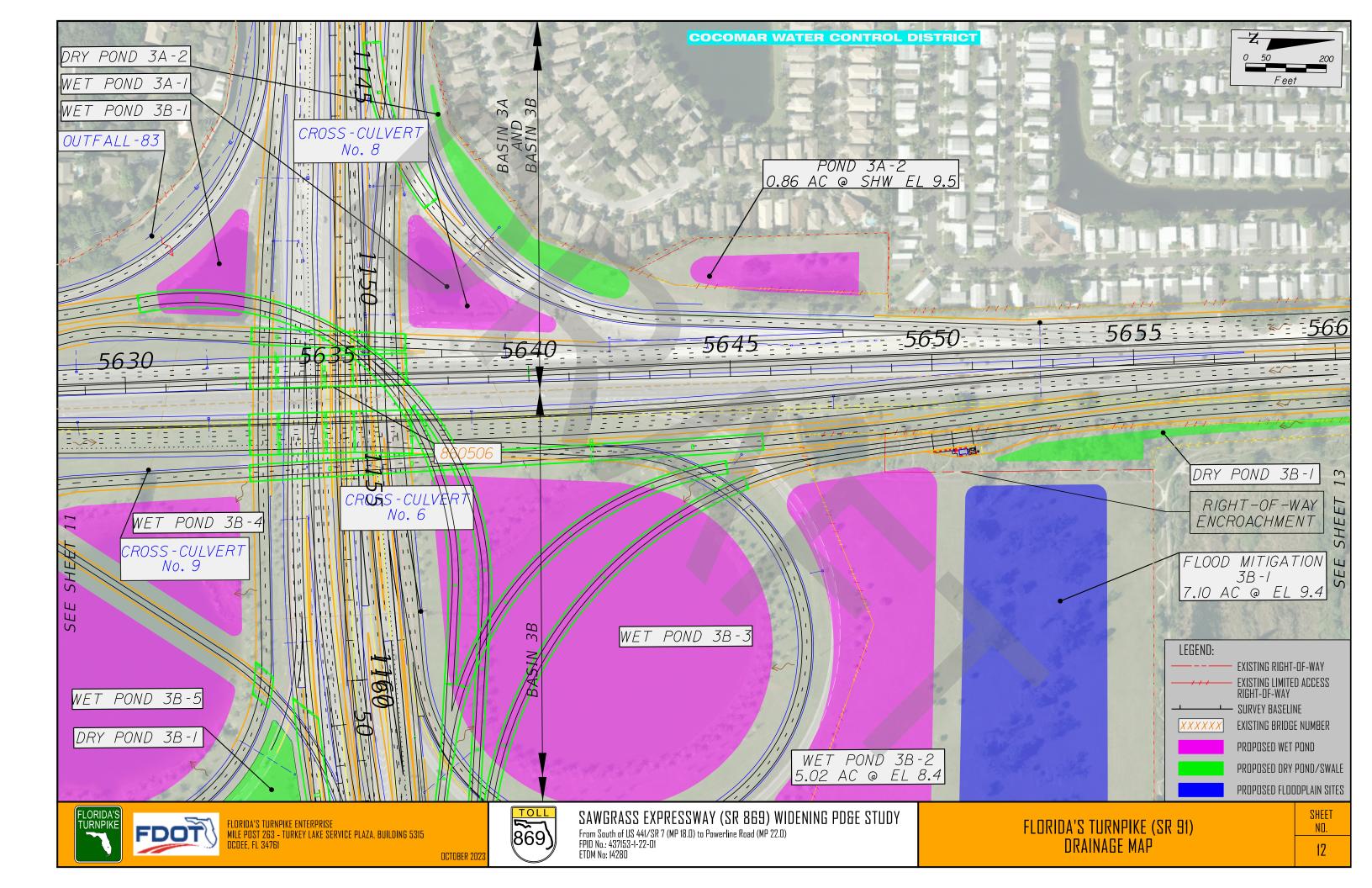


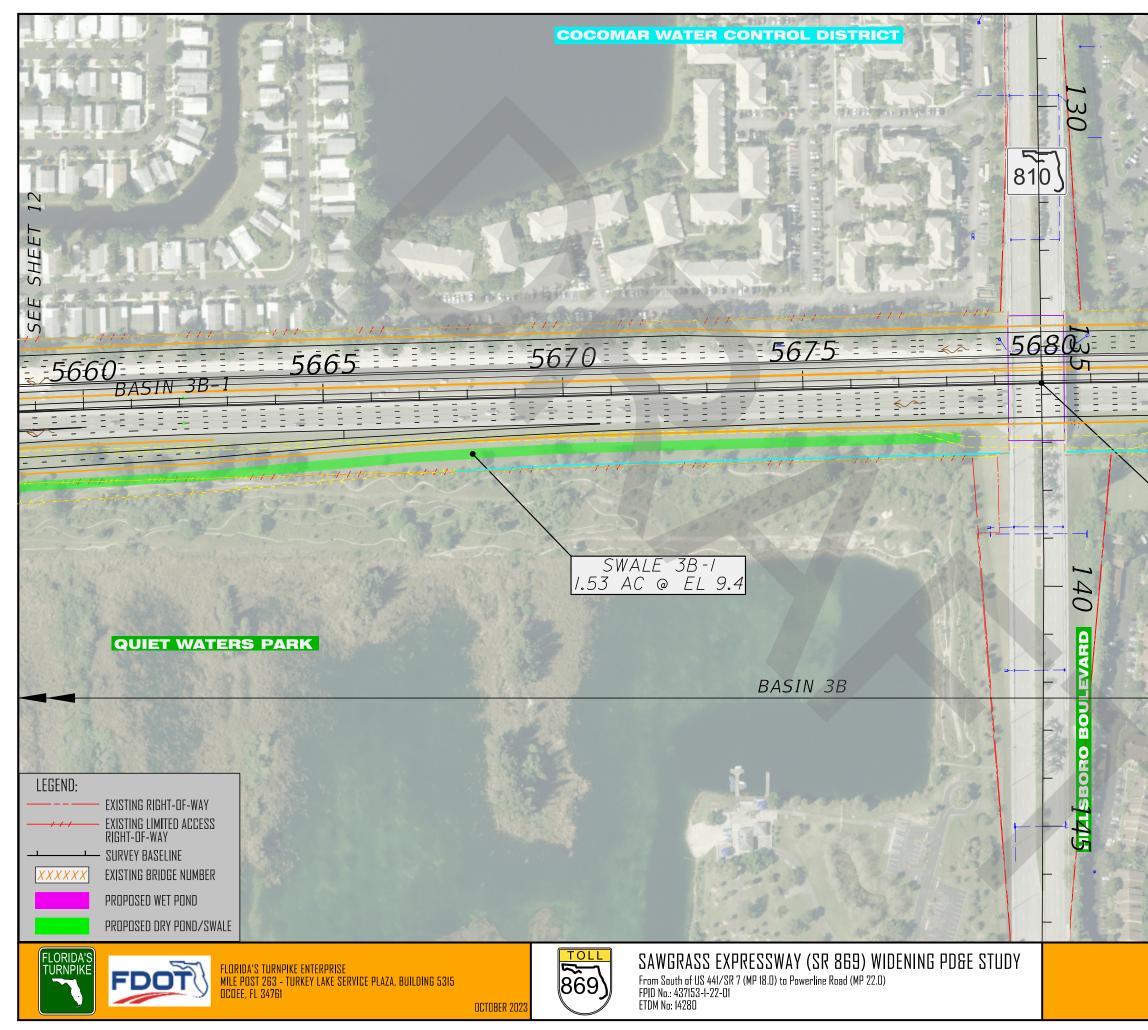




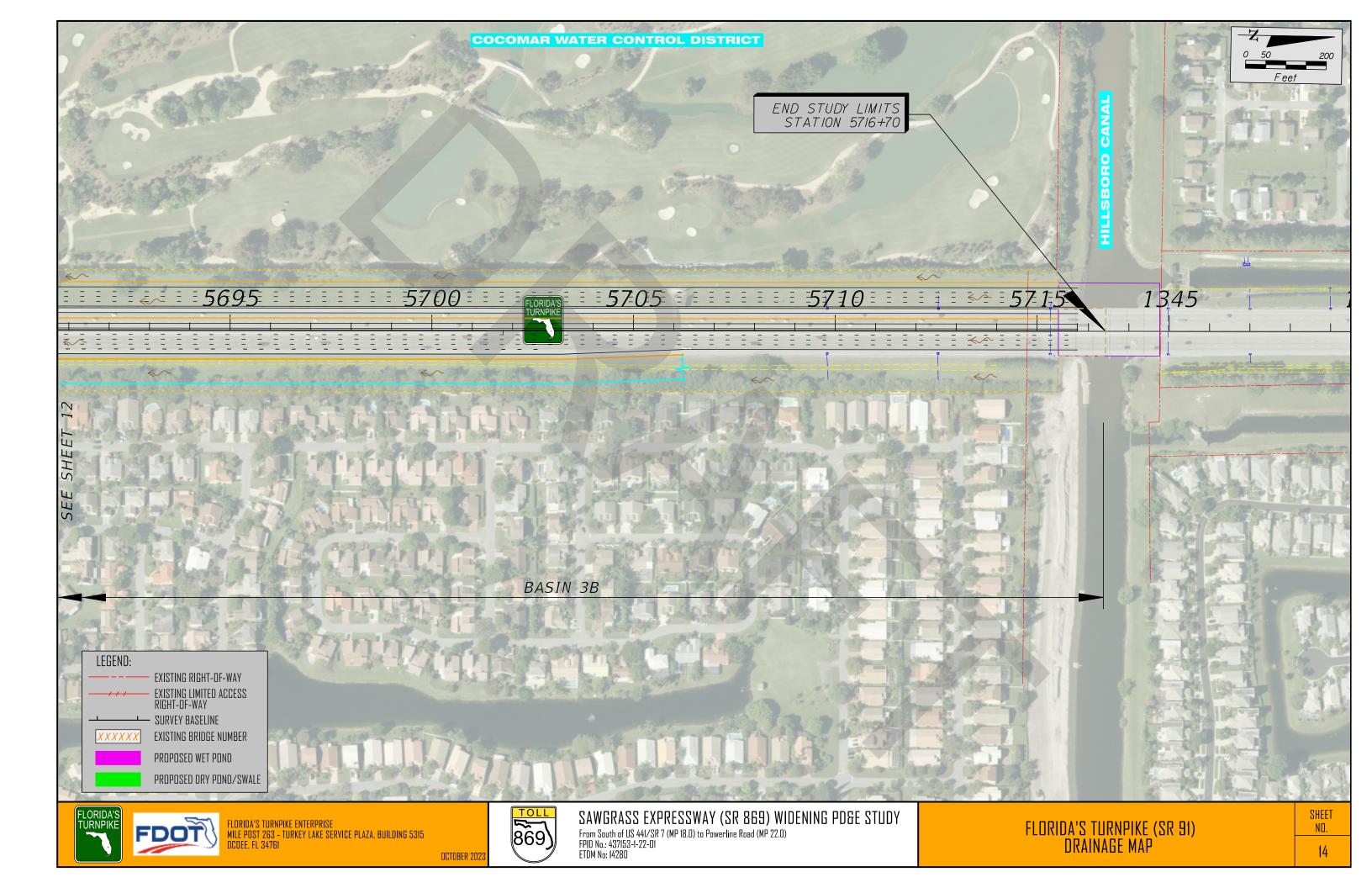








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FLORIDA'S TURNPIKE (SR 91) DRAINAGE MAP	SHEET ND. 13

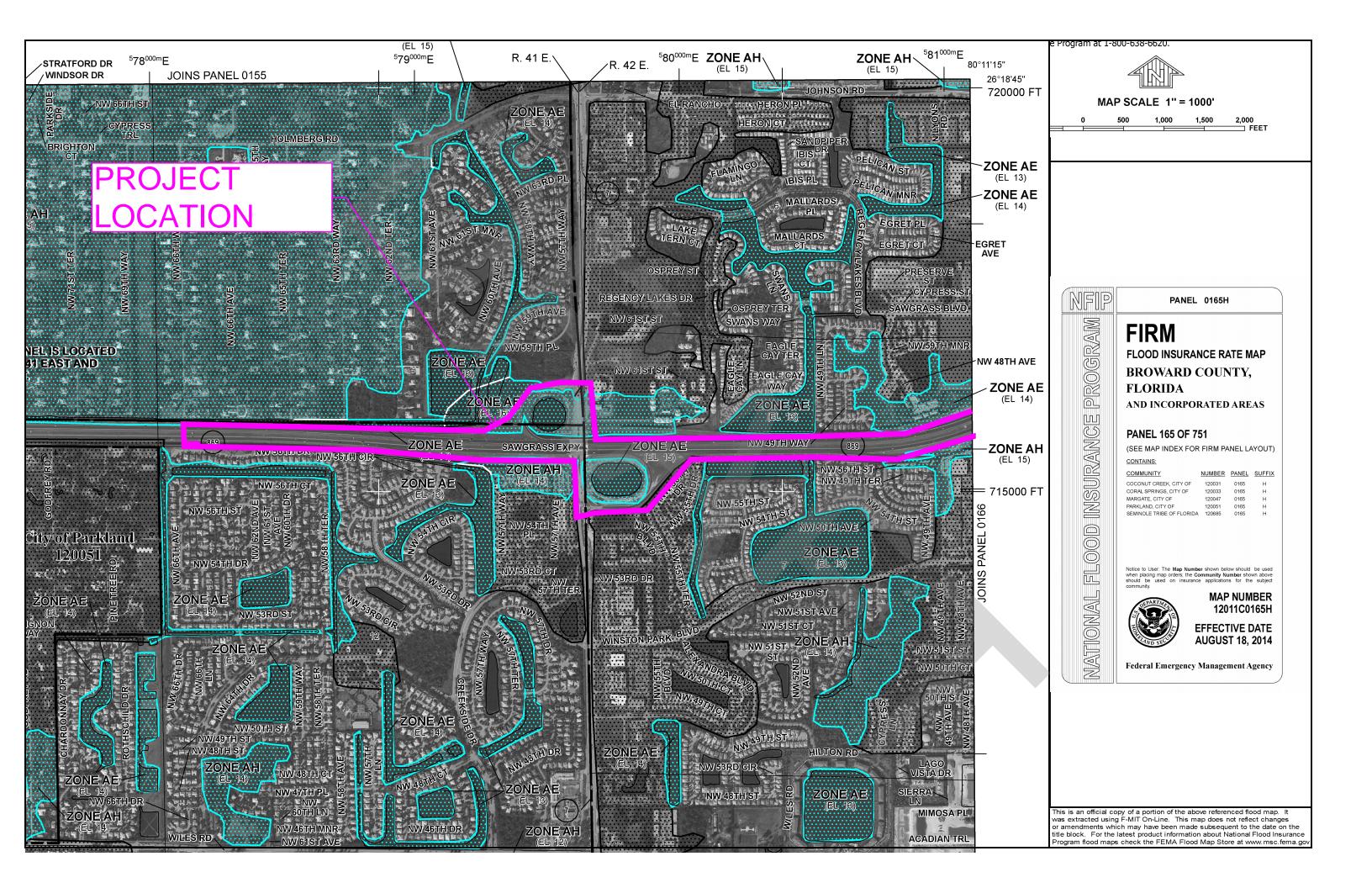




Draft Pond Siting Report

APPENDIX D

FEMA Maps & Floodplain Compensation Calculations



NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway. Data and/or Summary of Sillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Lears should be aware that BFEs shown on the FIRM represent rounded territh-flood should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations (BFEs) shown on this map apply only landward of 0.0° North American Vertical Datum of 1998 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Sillwater Elevations table in the Flood Insurance Study report for the jurisdiction. Elevations shown in the Summary of Sillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Porgam. Floodway withis and other pertirent floodways data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control** structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in preparation of this map was Transverse Mercator State Plane Florida East FIPS 0001. The horizontal datum was NAD83 HAPN, GRS1880. Spherold. Differences in datum, spheroid projection or State Plane zones used in production of FIRMs for adjacent juridictions may result in slipht positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1998. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <u>http://www.mss.noea.gov/</u> or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, Maryland 20910-3282 (301) 713-3242

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the information Services Branch of the National Geodetic Survey at (**301) 713-3242** or visit its website at <u>http://www.ngs.noaa.gov/</u>.

Base map information shown on this FIRM was provided in digital format by Broward County. The original orthophotographic base imagery was provided in color with a one-foot pixel resolution at a scale of 1" = 300' from photography flown in 2008.

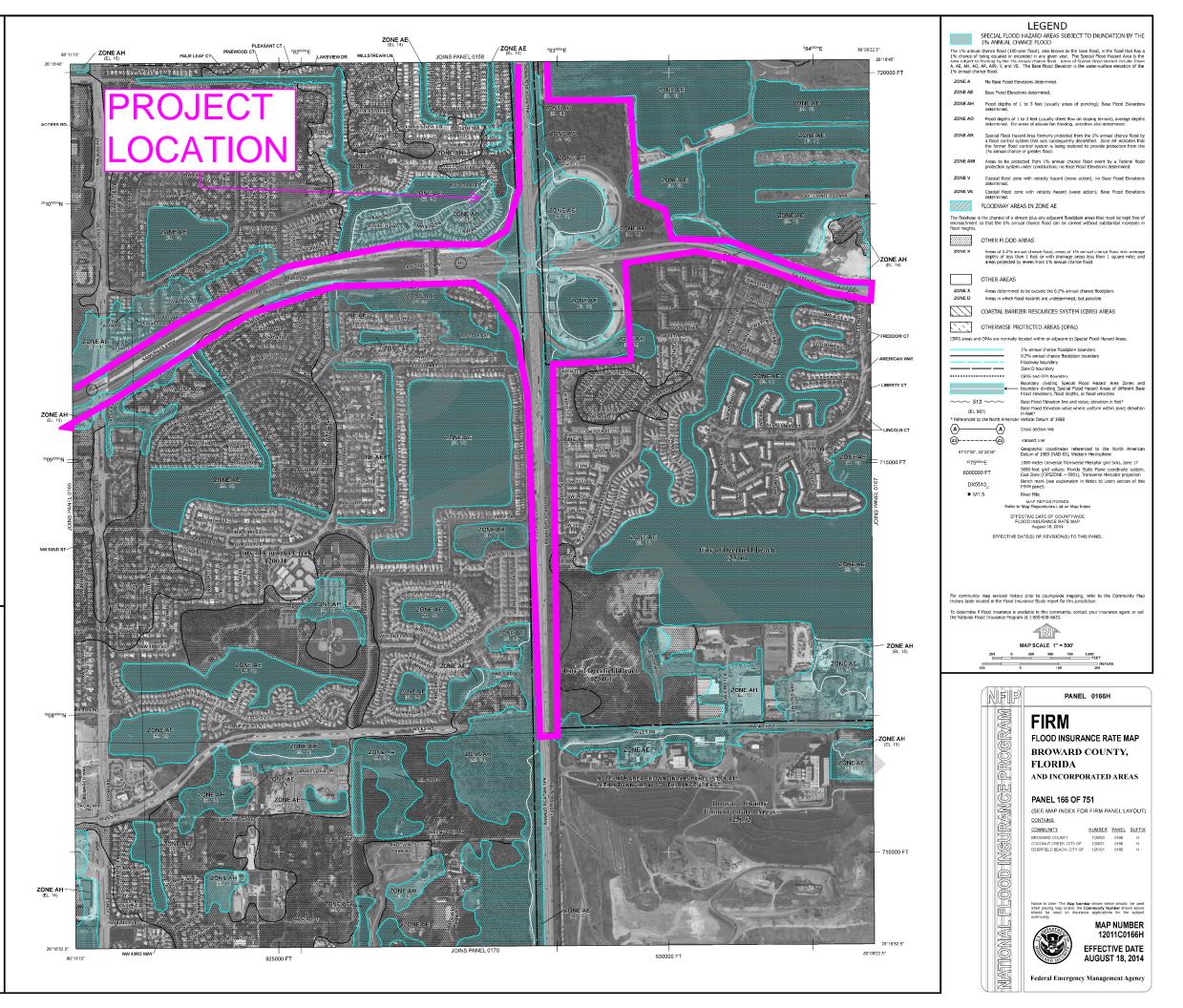
This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to controm to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

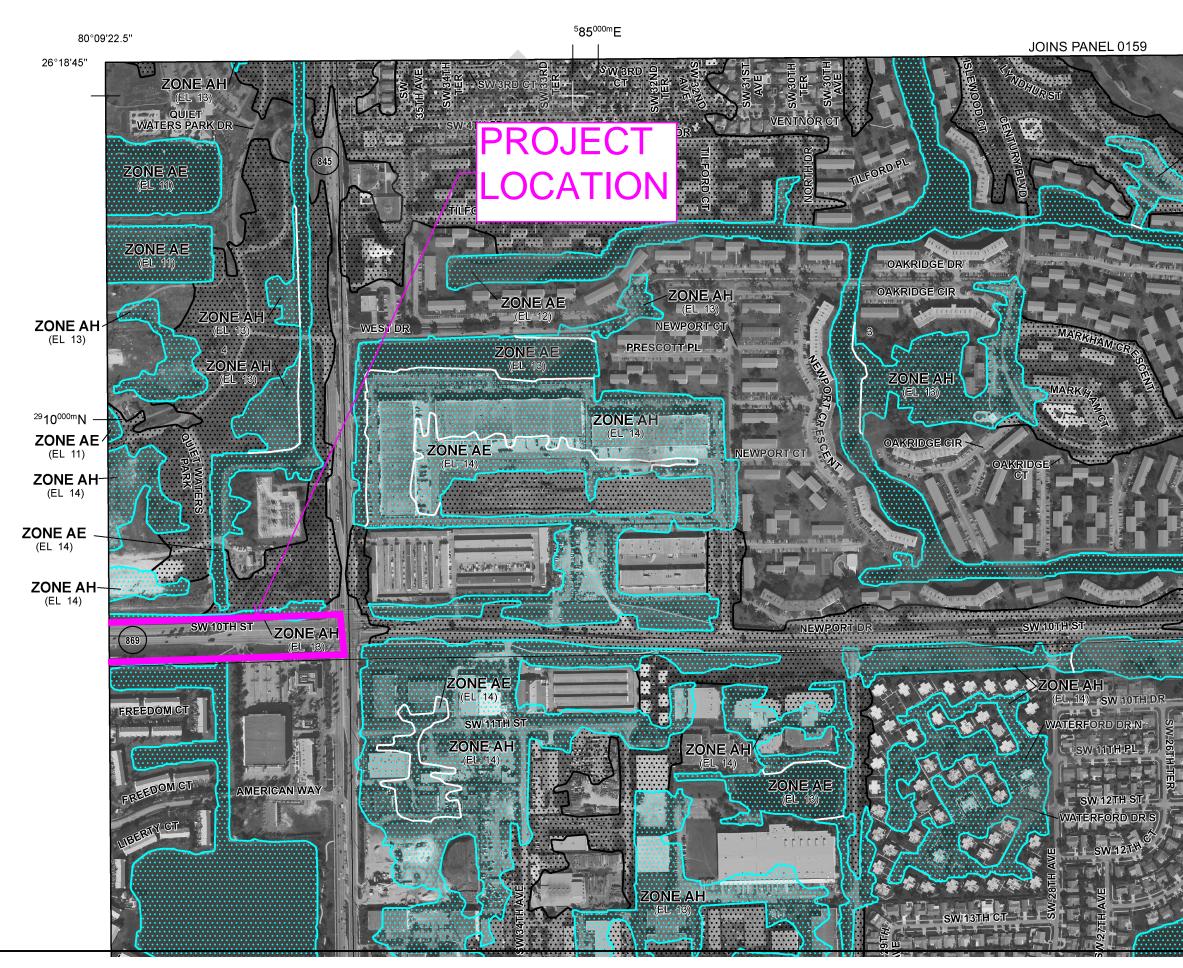
Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map penels community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

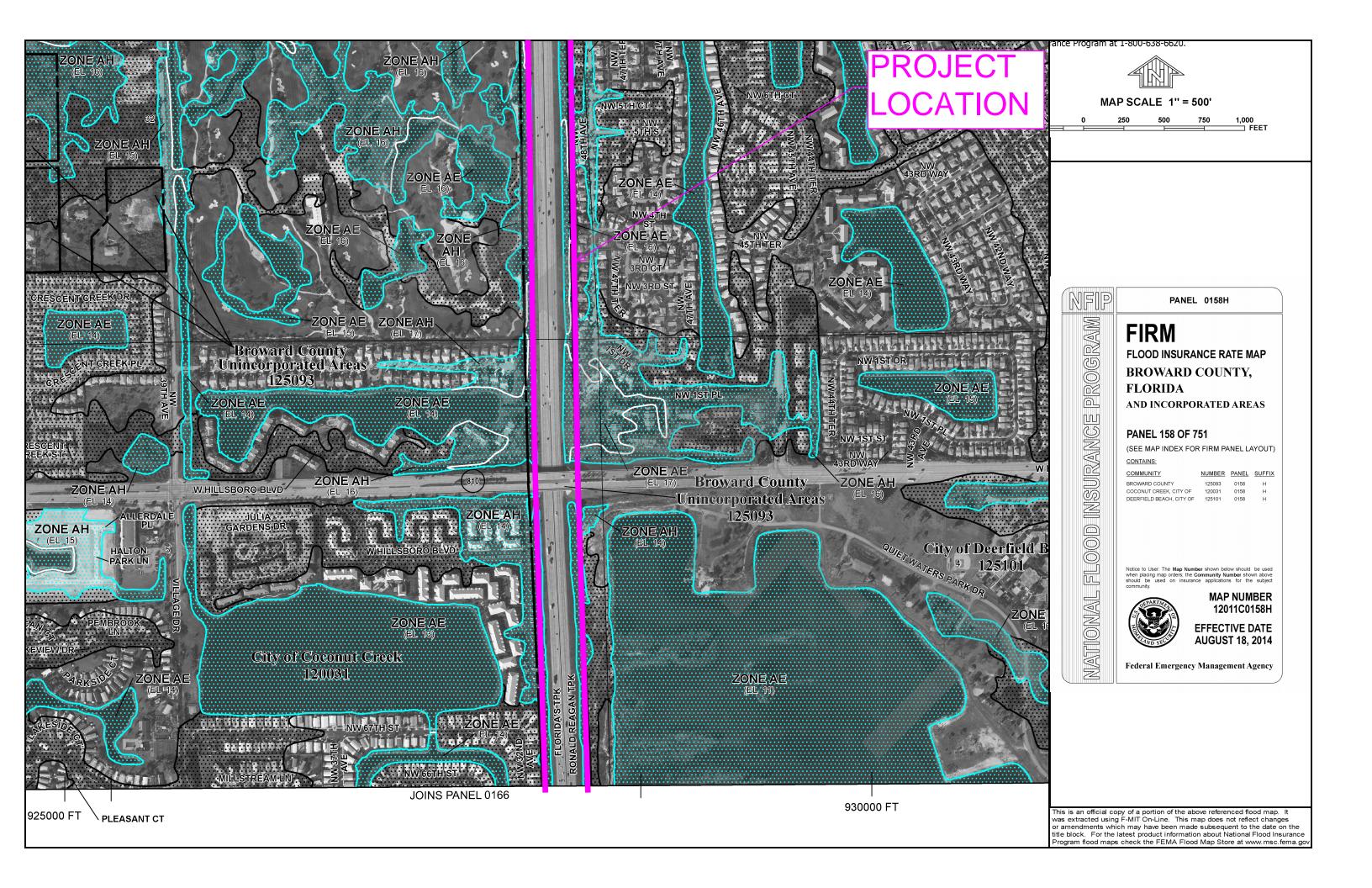
For Information and questions about this map available products associated with this FIRM including historic versions of this FIRM, how to order products, or the National Section 2014 (1997) and the Section 2014 (1997) and t

The "profile base lines" depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic dat, the profile base line, in some cases, may devate significantly from the channel centerline or appear outside the SFHA.





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	FEET
	PANEL 0167H
ATTONAL FLOOD INSURANCE PROGRAM	<section-header><text><text><text><text><text><text><text><text></text></text></text></text></text></text></text></text></section-header>



NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

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Coastal Base Flood Elevations (BFEs) shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD 88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to Section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in preparation of this map was Transverse Mercator State Plane Florida East FIPS 0901. The horizontal datum was NAD83 HARN, GRS1980 spheroid. Differences in datum, spheroid, projection or State Plane zones used in production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988. These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.mgs.noaa.gov/ or contact the National Geodetic Survey at the following address

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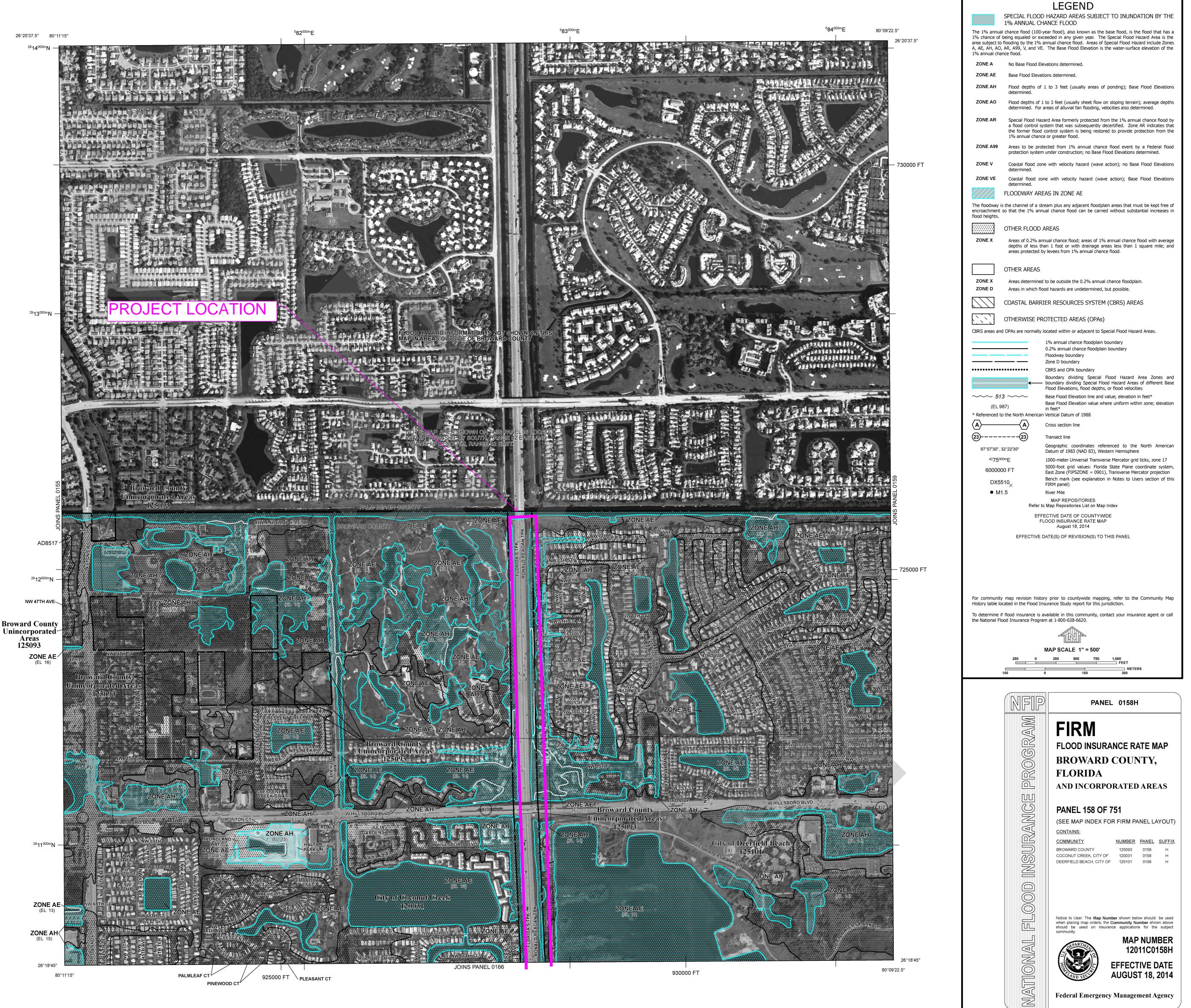
This map reflects more detailed and up-to-date stream channel configurations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

For information and questions about this map, available products associated with this FIRM including historic versions of this FIRM, how to order products, or the National Flood Insurance Program in general, please call the FEMA Map Information eXchange at 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA Map Service Center website at <u>http://msc.fema/gov</u>. Available products may include previously issues Letters of Map Change, a Flood Insurance Study report, and/or digital versions of this map. Many of these products can be ordered or obtained directly from the website. Users may determine the current map date for each FIRM panel by visiting the FEMA Map Service Center website or by calling the FEMA Map Information eXchange.

The "profile base lines" depicted on this map represent the hydraulic modeling baselines that match the flood profiles in the FIS report. As a result of improved topographic data, the profile base line, in some cases, may deviate significantly from the channel centerline or appear outside the SFHA.



Summary of Floodplain Compensation Ca	lculations			
LOCAL WATER MANAGEMENT DISTRICT	PROJECT BASIN	FLOODPLAIN ENCROACHMENT (AC-FT)	Provided Storage In	FLOODPLAIN COMPENSATION (AC-FT)
Pinetree Water Management District	BASIN 1	0.98	POND 1-1	1.26
Cocomar Water Management District	BASIN 2 & 3A	8.50	POND 2-1	5.17
Broward County Water Management District # 2	BASIN 3B, 3B-1, 3B- 2 & 4	38.76	FLOOD MITIGATION 3B-1 FLOOD MITIGATION	18.70
			3B-2	23.26
	PROJECT TOTAL =	<u>48.24</u>		<u>48.39</u>

FLOODPLAIN COMPENSATION CALCULATIONS

BASIN 1

SHW EL = 10 (NAVD) BOTTOM EL = 11 (NAVD)

FEMA 100YR EI = 14

STORAGE IN DRY POND 1-1

Average Pond Area @bottom El (ac)	Average Pond Area @ El 12(ac)	Depth between Avg. Ground El 12 & Bottom El 11 (ft)	Storage provided in pond (ac-ft)
1.17	1.34	1.0	1.26

BASIN 2 & 3A

SHW EL = 9.5 (NAVD)

TREATMENT AND ATTENUATION EL = 11.0 (NAVD)

FEMA 100YR EI = 15

STORAGE IN WET POND 2-1

Average c/s area	Periphery of	Storage provided in POND 2-1	Storage provided in
between El 11 & 15 (sf)	POND 2-1 (ft)		pond (ac-ft)
116.0	1943.00	225388.0	5.17

Note: Treatment and attenuation will be provided between SHW El 9.5 and El 11.0. Therefore the storage is excluded as floodplain compensation

BASIN 3B & 4

SHW EL = 8.4 (NAVD) AVG. EXIST. GR EL = 12.00

FEMA 100YR EI = 14

STORAGE IN FLOOD MITIGATION 3B-1 (DRY)

Average Pond Area @ Bottom El 9.4 (ac)	Average Pond Area @ El 12.0 (ac)		Storage provided in pond (ac-ft)
7.06	7.33	2.6	18.70

SHW EL = 8.4 (NAVD) AVG. EXIST. GR EL = 12.50

FEMA 100YR El = 14

STORAGE IN FLOOD MITIGATION 3B-2 (DRY)

Average Pond Area @ Bottom El 9.4 (ac)	Average Pond Area @ El 12.5 (ac)	Depth between Avg. Ground El 12 & Bottom El 9.4 (ft)	Storage provided in pond (ac-ft)
7.34	7.67	3.1	23.26

BASIN 1

SHW EL = 10 (NAVD)

	OFFSE	T/RT (FEM	A 100YR EI :	= 14)		OF	SET/LT		REQUIRED	PROVIDED
Station	Area of Fill within Flood Plain (sf)	Average (sf)	Length (ft)	Volume of Fill within FloodPlain (ac-ft)	Area of Fill within Flood Plain (sf)	Average	Length	Volume of Fill within FloodPlain (ac-ft)	FLOODPLAIN COMPENSATION (AC-FT)	FLOODPLAIN COMPENSATION (AC-FT)
1033+00	91.1				0					
1037+00	5.9	42.5	1000	0.98	0	0	0	0.00	0.98	
1039+00	30.5				0					
									0.98 ac-ft	1.26

<u>BASIN 2 & 3A</u>

SHW EL = 8.4 (NAVD)

Station	OFFSE	T/RT (FEM	A 100YR EI :	= 14)	OFFSET/LT (FEMA 100YR EI = 16)				REQUIRED	PROVIDED
	Area of Fill within Flood Plain (sf)	Average (sf)	Length (ft)	Volume of Fill within FloodPlain (ac-ft)	Area of Fill within Flood Plain (sf)	Average	Length	Volume of Fill within FloodPlain (ac-ft)	FLOODPLAIN	FLOODPLAIN COMPENSATION (AC-FT)
1044+00	0	0	0	0	283.2	266.6	1000	6.12	6.12	
1046+00	0	0	0	0	250.0			0.12	0.12	
	OFFSE	ET/RT (FEM	A 100YR EI =	: 15)	OFFSET/LT (FEMA 100YR EI = 15)					
1152+00	20.40	20.4	200	0.09	78.9	78.9	200	0.36	0.46	
	OFFSE	ET/RT (FEM	A 100YR EI =	: 14)	OFFSET/LT (FEMA 100YR EI = 15)					
1088+00	52.65				67.3					
1090+00	66.05	63.3	300	0.44	149.2	108.2	600	1.49	1.93	
1091+00	71.30				1652.5					
			-			-			8.50 ac-ft	5.17

BASIN 3B, 3B-1, 3B-2 & 4 SHW EL = 8.4 (NAVD)

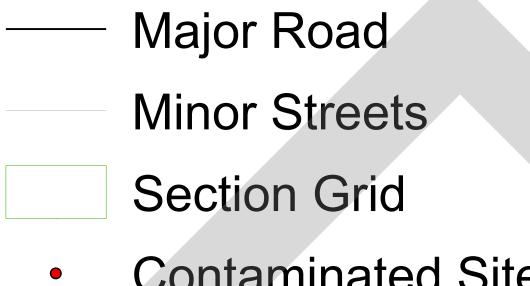
	OFFSE	T/RT (FEM	A 100YR EI :	= 14)	OF	FSET/LT (FE	MA 100YR	El = 14)	REQUIRED	PROVIDED
Station	Area of Fill within Flood Plain (sf)	Average (sf)	Length (ft)	Volume of Fill within FloodPlain (ac-ft)	Area of Fill within Flood Plain (sf)	Average	Length	Volume of Fill within FloodPlain (ac-ft)	FLOODPLAIN COMPENSATION (AC-FT)	FLOODPLAIN COMPENSATION (AC-FT)
5580+00	0	0	0	0	195.35	202.7	1200	5.58	5.58	
5590+00	0	0	U	0	210	202.7	1200	5.56	5.56	
5610+00	381.3	403.3	1200	11.11	0	0.0	0	0.00	11.11	
5620+00	425.3	405.5	1200	11.11	0	0.0	0	0.00	11.11	
5650+00	524.3				13.7					
5660+00	75.2	233.9	3200	17.18	79.0	46.3	3200	3.40	20.59	
5665+00	102.3		-		84.1					
5681+00	0.0	0.0	0	0.00	14.0	19.0	3400	1.48	1.48	
5715+00	0.0	0.0	U	0.00	24.0	19.0	5400	1.48	1.48	
									38.76 ac-ft	18.70

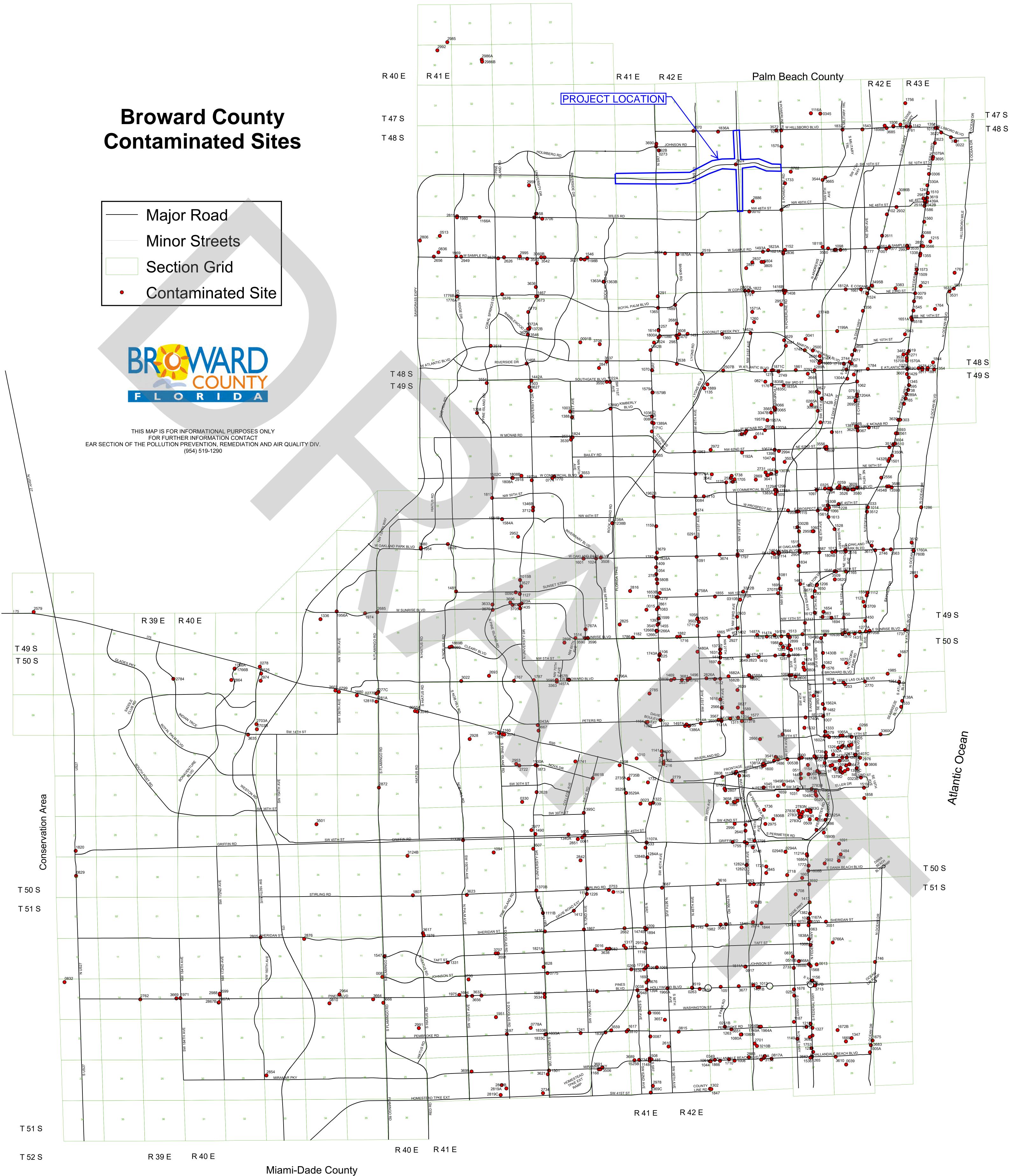
Draft Pond Siting Report



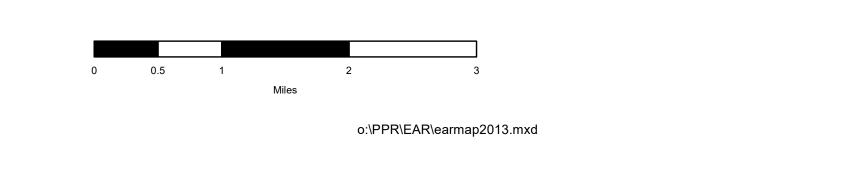
APPENDIX E

Contaminated Sites and Excerpts of Draft CSER











5.2 GOVERNMENT DATABASE AND REGULATORY FILE REVIEW

Information regarding potentially contaminated sites was obtained through the ETDM system, EST contamination layer, and Florida Geographic Data Library Geographic Information System (GIS) layers as well as the FDEP OCULUS, FDEP Map Direct, Broward County and USEPA RCRA contamination databases. These data sources include information on biomedical waste sites, brownfield location boundaries, dry cleaners, gasoline stations, hazardous waste sites, NPL and Superfund sites, nuclear site locations, State Underground Petroleum Environmental Response Act (SUPER Act) Risk Sources, solid waste facilities, storage tanks, RCRA facilities, and sites where environmental contamination has been documented in the soil or groundwater.

5.3 HISTORICAL AERIAL PHOTOGRAPH REVIEW

A review of historical aerial photos was performed using a website from the University of Florida Digital Collections (http://ufdc.ufl.edu/aerials/all/table/2), which contained aerial photographs of the project area from 1949, 1958 and 1984. Aerial photographs from 1995 to today's date were reviewed using the Google Earth historical imagery function. The aerial images were reviewed for potential contamination concerns, including but not limited to mounds, depressions, storage areas or drastic changes in landscaping or geographic features. Prior to 1980s, the region around the project was mostly undeveloped or used as agricultural land and most of the available aerial photography lacks labeled reference points (e.g. major roads) necessary to identify the project corridor. A brief discussion of the review of historic aerial photographs is provided below.

- 1949- The area around the project is predominantly undeveloped or under agricultural use.
- 1958- Additional roads are visible and expanded agricultural areas are evident.
- 1984- Only the far eastern portion of the Broward County is visible. An extensive network of roads and canals near the coast is visible and there is evident urbanization in the region.



- 1995- The project area is predominantly developed with residential land uses similar to the present configuration. The current road network is in place, and multiple drainage ponds are visible.
- 1999- Additional residential development is especially evident between US
 441 and Powerline Road.
- 2002- Residential and commercial development as filled in much of the remaining undeveloped land, particularly just south of N. Hillsborough Blvd and east of US 441 and near the intersection of Florida's Turnpike and NW 48th Street.
- 2005- Little major changes are evident as the majority of the region has been urbanized. Small infill developments are visible.

5.4 FIELD REVIEWS

Field observations to determine the potential for contamination include visual inspections for pipes, storage tanks or other infrastructure, and changes in topography, mounding, or depressions that may indicate subsurface concerns. Field inspections also note any staining, odor, or distressed vegetation that could indicate a spill or release of contaminants and when available include interviews with local residents and business employees. Photographs and notes about each potentially contaminated site are also collected.

Field reviews of the project area were conducted on October 14 and 18, 2019. Data was collected to determine the location of potentially contaminated sites and the current occupancy and operations.

5.5 RISK RATINGS

Based on the compilation of data collection activities described above, each site was assigned a risk rating as outlined in the FDOT *PD&E Manual*. The ratings system expresses the degree of concern for a potential contamination impact to the project via cost and schedule. Each site was assigned a contamination risk rating of No, Low, Medium, or High based on the following criteria:



- No A review of available information on the property and a review of the design plans indicates there is no potential for contamination to impact the project. It is possible that contaminants had been handled on the property. However, all information (assessment reports, monitoring well abandonments, results of recent soil and groundwater sampling, etc.) indicate that contamination impacts are not expected.
- 2. Low A review of available information indicates that former or current activities on the property have an ongoing contamination concern, has a hazardous waste generator identification (ID) number, or handles hazardous materials in some capacity. However, based on all available information and current design plans, it is not likely that there would be any contamination impacts related to this project.
- 3. Medium After a review of all available information, the potential contamination has been identified. This may include known soil and/or groundwater contamination that may not require remediation, is currently being remediated, or that is currently in the monitoring only phase. The complete status of remediation is important to determine what FDOT must do if the property were to be acquired. If there is insufficient reliable information (such as regulatory records or site historical documents) to make a determination as to the potential for contamination, and there is reasonable suspicion that contamination may exist, the property should be rated at least as a "Medium".

A recommendation should be made for each property in this category based on whether it would be within the proposed project, what additional assessment or remedial actions might be required if the property is acquired, and the possible requirements for additional actions if there is a need to avoid the property.

This ranking is the lowest possible rating a currently operating petroleum fueling, or storage facility can receive in an assessment document, based on its distance to the right-of-way, contamination type, need for dewatering in the area, etc.



4. High - After a review of all available information and current conceptual or design plans, there is a reasonable potential for contamination impacts during construction. Once the Design Alternative has been selected, sites rated with high contamination potential require further assessment to confirm and delineate potential contaminants and to determine if remediation or special construction provisions will be needed during construction. The recommendation for this rating should include a listing of the parameters of concern and media to be assessed, and if known, what construction activities will occur within or adjacent to the contaminated media. Properties used historically as gasoline stations, and which have not been evaluated or assessed, would likely receive this rating.



6.0 POTENTIAL PROJECT IMPACTS

6.1 POTENTIALLY CONTAMINATED SITES

After screening for sites within appropriate buffers, a total of 16 sites of potential contamination risk were identified. Those include two High Risk, five Medium Risk, and nine Low Risk sites (Error! Reference source not found. None of these p otentially contaminated sites are proposed for right-of-way acquisition. Information on each site is summarized in Table 6.2 and locations are shown in Figures 6.1 and 6.2Error! Reference source not found.. Individual site descriptions, i ncluding field observations and a summary of available documentation, are provided in the text below. Appendix A contains regulatory documents and information associated with each site and Appendix B contains photographs related to each Medium and High risk site.

Risk Rating	Number of Sites	Sites Proposed for Right-of-Way Acquisition Under Preferred Alterative
Low	9	0
Medium	5	0
High	2	0

Table 6.1— Contamination Risk Rating Summary for Preferred Alternative

	Table 6.2— Sites of Potential Contamination Risk										
Site #	Facility Name	Address/Location	Facility ID (FDEP/RCRA)	Databases	Concern	Distance to Preferred Alternative	Risk Rating				
1	Lou Bachrodt Chevrolet/Mazda	5500 N Hwy 7	9700091	STCM	Storage Tank	Adjacent	Low				
2	Capella Farms Inc	5730 N SR 7	8625849	FDEP STCM	Historic record of Underground Storage Tank	Within FDOT right-of-way	Low				
3	Residence Inn by Marriott	5730 N State Road 7	9817162	STCM	Storage Tank	Adjacent	Low				
4	Club Caribe Associates	5510 NW 61 st Street #101	9400067	FDEP STCM, PCTS	Diesel spill and Storage Tanks	Adjacent	Low				
5	BOCA #7838	6330 N SR 7	9807767	STCM, PCTS	Retail Gas Station	Adjacent	High				
6	Hampton Inn and Suites	5740 N SR 7	9814296	STCM	Diesel Storage Tank	Adjacent	Low				
7	Marathon – Winston Park	5425 Lyons Road	9801357	FDEP STCM	Retail Gas Station	Adjacent	Medium				
8	Coconut Creek – Shell Station	6135 Lyons Road	9805503	FDEP STCM, PCTS	Retail Gas Station	Adjacent	Medium				
9	McFarlane Trucking 03-2I-0279	Deerfield Toll Plaza EB on Sawgrass	9807332	FDEP STCM, PCTS	Diesel Fuel Spill	Within FDOT right-of-way	Medium				
10	Douglas Fertilizer/Liquid AG Systems	Sawgrass Expressway	ERIC 10265	ERIC	Liquid Fertilizer Spill	Within FDOT right-of-way	Low				
11	Chemical Waste Mgt Pompano	2700 NW 48 th St	77155917, ERIC_8689	DEP Cleanup	Waste Transfer Station	Adjacent	Low				
12	Central Sanitary Landfill and Recycling Center	1801 SW 45 Way	77155916, ERIC 8682	DEP Cleanup	Mapped by FDEP Map Direct	Adjacent	Low				
13	Monarch Hill	2700 Wiles Road	8622531 / 55093	FDEP STCM, Solid Waste, Institutional Controls Registry	Landfill, Leachate in groundwater	Adjacent	High				
14	Med-Care Pharmacy	1052 S. Powerline Road	9100756	FDEP STCM	Storage Tanks	200 ft	Low				
15	Shell – First Coast Energy #1836	1011 S Powerline Road	9800891	FDEP STCM	Retail Gas Station	Adjacent	Medium				
16	FDOT Bridges and Overpasses	Throughout Project	None	None	Asbestos, Lead Based Paint, Metal Coatings	Within FDOT right-of-way	Medium				

869

Sawgrass Expressway (SR 869) Widening PD&E Study Contamination Screening Evaluation Report



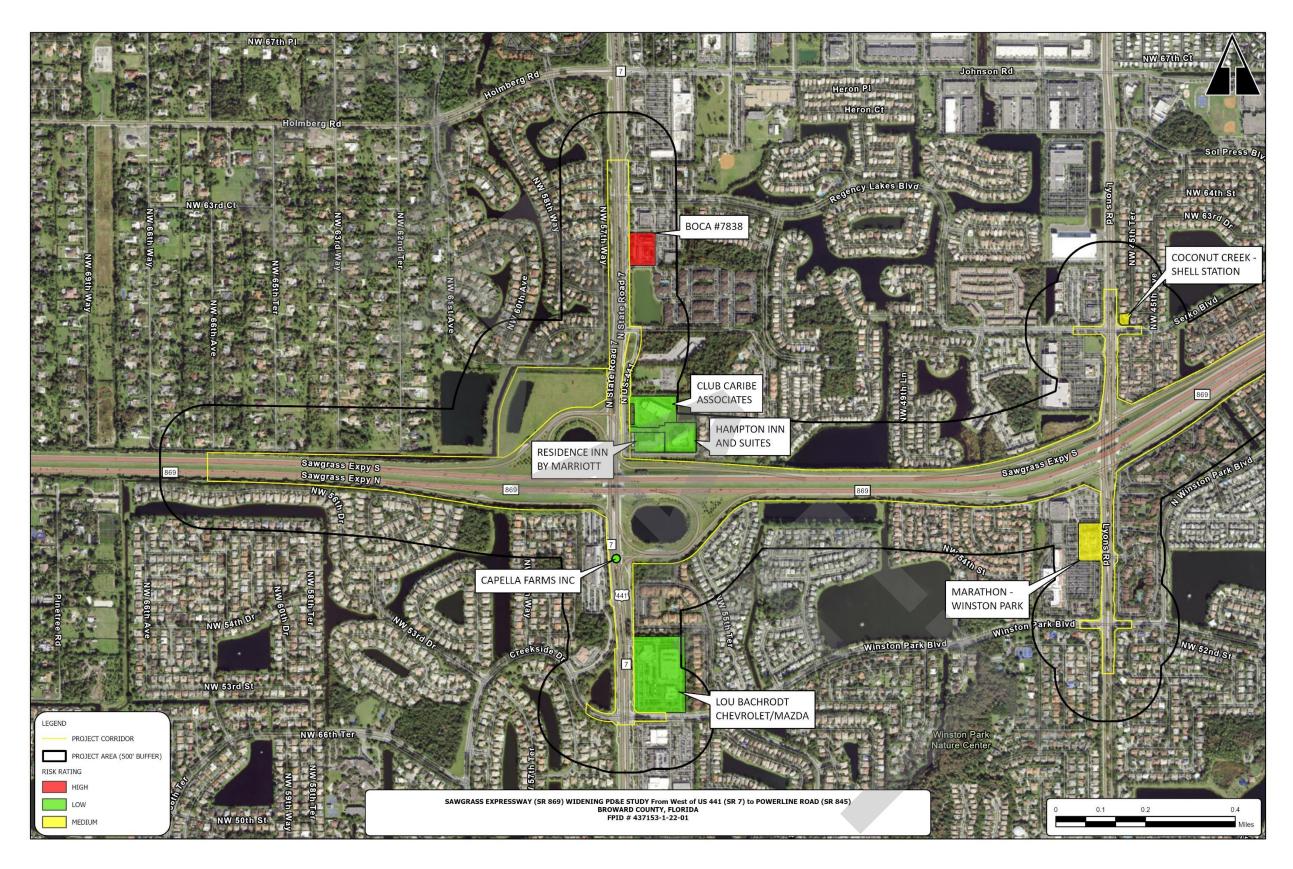


Figure 6.1 — Potentially Contaminated Sites in Western Project Area

Sawgrass Expressway (SR 869) Widening PD&E Study Contamination Screening Evaluation Report



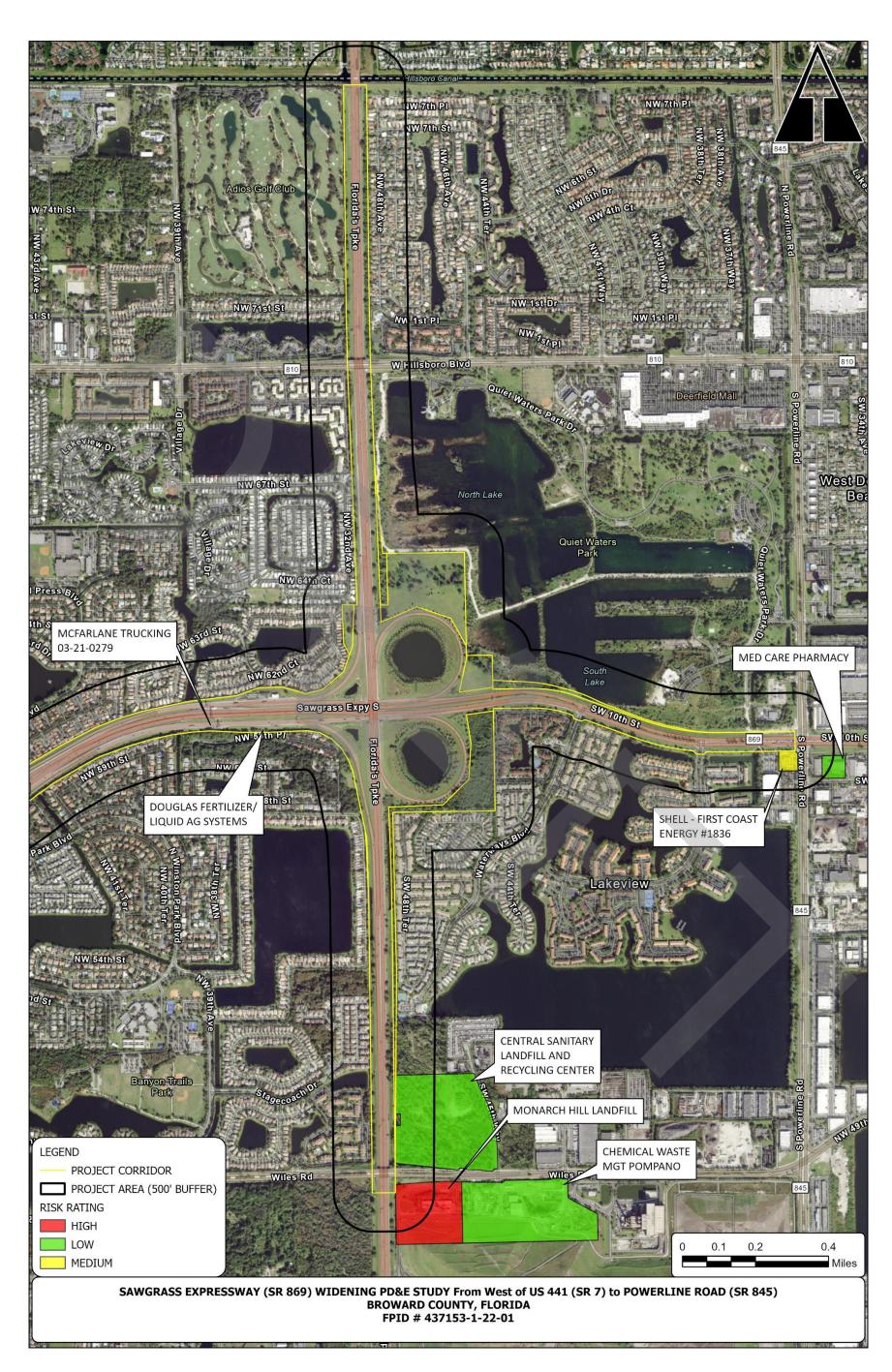


Figure 6.2 — Potentially Contaminated Sites in Eastern Project Area



6.2 INDIVIDUAL SITE SUMMARIES

6.2.1 SITE 1 LOU BACHRODT CHEVROLET/MAZDA

5500 N HWY 7 Facility ID: 9700091

Risk Rating: Low

This site is a car dealership located in the northeast quadrant of the intersection of US 441 and Winston Park Boulevard. It was reported in the FDEP Storage Tank Contamination Monitoring database. Documents on file indicate the presence of one 2,000 gallon diesel storage tank and one 2,200-gallon gasoline storage tank by October 1996. There is no documented history of spills or release of any contaminants. Because this site has fuel storage tanks but no history of contaminated soil or groundwater, it is assigned a risk rating of Low.

6.2.2 SITE 2 CAPELLA FARMS INC

5730 N SR 7 Facility ID: 8625849

Risk Rating: Low

This site is located east of US 441, immediately south of Sawgrass Expressway. This area contains ramps and a stormwater pond for Sawgrass Expressway as well as a complex of condominiums. The only document on file with FDEP is a Stationary Tank Registration/Notification Form dated 11-14-1984. It reports the presence of two 2,000-gallon and one 3,000-gallon underground storage tank used for unleaded fuel. That form includes a map (**Appendix A**) showing the location of the three tanks in the central part of the property, not adjacent to US 441. A handwritten note on that form states the site was closed February 1996. Although it has been redeveloped, there is a record of underground storage tanks on site and no official documentation of removal, so site is assigned a risk rating of Low.

6.2.3 SITE 3 RESIDENCE INN BY MARRIOTT

5730 N State Road 7 Facility ID: 9817162 Risk Rating: Low

This site is a hotel in the northeastern quadrant of the intersection of Sawgrass Expressway and US 441. It was reported in the Storage Tank Contamination Monitoring database. Records indicate the presence of five underground storage tanks in 1984. Because there are storage tanks but no record of soil or groundwater contamination, this site is assigned a risk rating of Low.



6.2.4 SITE 4 CLUB CARIBE ASSOCIATES

5510 NW 61st St #101 Facility ID: 9400067 Risk Rating: Low

This site is a condominium development located in the northeastern quadrant of the intersection of Sawgrass Expressway and US 441. Construction on condominiums appears to have started around 1990. Portions of this site were previously under cultivation. Documents from FDEP indicate that in 1989 multiple storage tanks were removed and no signs of contamination were apparent. A discharge of diesel fuel was reported on 12-7-1992. FDEP indicated that the site was assigned No Further Cleanup status by January 19, 1994. A Storage Tank Registration Form from February 1, 1993, indicates the presence of three 1,000gallon storage tanks at this site. Because of the No Further Action Status, this site is assigned a risk rating of Low.

6.2.5 SITE 5 BOCA #7838

6330 N SR 7 Facility ID: 9807767 Risk Rating: High

This site is a retail gas station located approximately 2,200 feet north of Sawgrass Expressway and immediately east of US 441. The property contains a covered fuel island, a convenience store and car wash. The earliest document on file is a 2005 Storage Tank Facility Registration Form noting the new registration of three underground storage tanks (20,000 gallon, 12,000 gallon, and 10,000 gallon). A Storage Tank System Leak Autopsy Report Form from September 2007 reports that gasoline was discharged into groundwater. A Discharge Report Form was also completed September 2007. There is no record of site closure or cleanup. Because this site is an operating retail fuel facility with a history of discharge and no site closure or testing to show contaminants are below target levels, this site is assigned a risk rating of High.

6.2.6 SITE 6 HAMPTON INN AND SUITES

5740 N State Road 7 Facility ID: 9814296 Risk Ratina: Low

This site is a hotel in the northeastern quadrant of the intersection of Sawgrass Expressway and US 441. It was reported in the Storage Tank Contamination Monitoring database. A Storage Tank Facility Annual Compliance Site Inspection Report notes the presence of one 1,355-gallon diesel aboveground storage tank in December 2014. The tank was installed without an Install Permit and the facility was also in violation for lack of financial responsibility. There is no documented discharge or any contaminants. Because there are storage tanks but no record of soil or groundwater contamination, this site is assigned a risk rating of Low.

6.2.7 SITE 7 MARATHON – WINSTON PARK

5425 Lyons Rd, Coconut Creek

Facility ID: 9801357

Risk Rating: Medium

This site is an operating gas station located adjacent to the project west of Lyons Road, south of Sawgrass Expressway. Historic aerial images show it was constructed between 1995 and February 1999. It is part of a large commercial center that includes a Publix supermarket. A Storage Tank Registration form from 3-11-1999 lists three 10,000-gallon, double-walled, underground storage tanks used for unleaded gasoline. Minor violations were reported in an inspection report dated 7-5-2019 that confirmed the presence of three underground storage tanks. An associated warning notice was issued by Broward County and is the most recent document on file. A non-compliance letter was issued on February 15, 2022, for one violation regarding repairs. Because this site is an operating gas station, it is assigned a risk rating of Medium.

6.2.8 SITE 8 COCONUT CREEK - SHELL STATION

6135 Lyons Road Facility ID: 9805503

Risk Rating: Medium

This site is an operating gas station located approximately 1,000 feet north of the Sawgrass Expressway and Lyons Road Interchange. This retail fuel-station has three registered Underground Storage Tanks (USTs) which were installed in early 2003; Two 15,000-gallon gasoline and one 12,000-gallon diesel tank. Water apparently leaked into the primary tank, as noted on an Incident Notification Form (INF) dated 3-5-2014, no free product was released according to this form. A separate INF was filed later that year on 9-24-2014 when the diesel spill bucket failed a hydrostatic test. Again, no free product release was reported. A Discharge Reporting Form (DRF) dated 11-12-14 was later filed after free product appeared to be leaking from the spill bucket into the surrounding pea gravel. Impacted pea gravel was excavated and placed into a drum for disposal and the three spill buckets were removed and disposed of later that month. Analytical



results from the soil samples verified that impacted gravel was removed and that no detectable levels of hydrocarbons remained in the area. Final sample results were provided to the Broward County Pollution Prevention Division and on 7-15-2015, the County accepted the report and specified that no additional assessment activities were required at that time. An inspection report from November 2015 documents that no further issues were identified. An attached Memorandum dated March 12, 2020, noted contamination from a discharge on November 6, 2014, was addressed via source removal. The status has been changed from Cleanup to No Cleanup Required (NREQ). Because this facility is an operating gas station and no additional assessment activities were required following a discharge, it is assigned a risk rating of Medium.

6.2.9 SITE 9 MCFARLANE TRUCKING 03-2L-0279

Deerfield Toll Plaza Eastbound on Sawgrass

Facility ID: 9807332

Risk Rating: Medium

This is the location of a diesel spill within FDOT right-of-way on eastbound Sawgrass Expressway, just north of NW 40th Avenue. Documents from FDEP indicate that a diesel spill occurred on 9-26-2003 and the site was referred to Broward County. The spill occurred immediately east the Deerfield Toll Plaza on eastbound lanes. This site is listed in the FDEP Storage Tank Contamination Monitoring Database as well as the Pollution Contamination Monitoring Discharges Database. Maps (**Appendix A**) show the site of the spill adjacent to Sawgrass Expressway travel lanes. Because of the history of a diesel spill and co-location with the project, this site is assigned a risk rating of Medium.

6.2.10 SITE 10 DOUGLAS FERTILIZER/LIQUID AG SYSTEMS

Located within Sawgrass Expressway right-of-way

Facility ID: ERIC_10265

Risk Rating Low

This site is located within FDOT right-of-way on northbound Sawgrass Expressway, approximately 400 feet west of the ramp to Florida's Turnpike southbound. In 2005 a liquid fertilizer truck experienced a severed line and the driver pulled off the road. Approximately 1200 gallons of nitrogen-based liquid fertilizer was released. Source removal was conducted followed by groundwater sampling. Soil and groundwater concentrations were found to be below applicable target levels, and a Site Rehabilitation Completion Order was issued December 27, 2010.



Because a Site Rehabilitation Completion Order was issued, this site is assigned a risk rating of Low.

6.2.11 SITE 11 CHEMICAL WASTE MANAGEMENT POMPANO

2700 NW 48th Street

FDEP Cleanup 77595814, ERIC_8689

Risk Rating: Low

This site is located in the southeast quadrant of the intersection of Florida's Turnpike and Wiles Road. This sits is on the property of a major landfill that covers approximately 500 acres. The earliest document on-file is from 1980 and includes a permit to construct a solid waste transfer station on approximately one acre. That site would receive hazardous waste and transfer it to a location in Emelle, Alabama. Documents indicate a total storage capacity of 160 containers with a provision for storing an additional 20 drums in an emergency. There is no documentation or indication of any historic release of contaminants at this site, and no right-of-way would be acquired from this site. For these reasons, but because the site handles and stores waste, a risk rating of Low is assigned to this site.

6.2.12 SITE 12 CENTRAL SANITARY LANDFILL AND RECYCLING CENTER

1801 SW 45 Way

DEP Cleanup Sites 77155916, ERIC 8682

Risk Rating: Low

This site is mapped by FDEP Map Direct in the northeast quadrant of the intersection of Florida's Turnpike and Wiles Road. The address is reported in Map Direct as 3140 NW 48th Street, but this is apparently erroneous and does not occur in the vicinity of the project. The Property Appraiser reports the address of the mapped location as 1801 SW 45 Way, Deerfield Beach, FL. The property is owned by RMT-SDE-3-LLC and was last sold in 2021.

The parcel is currently forested and crisscrossed with vehicle tracks. An outdoor storage business and a steel fabrication business are located northeast of the property, at the north end of SW 45th Way. The Waterways residential neighborhood borders this property to the north. Historic imagery from 1995 shows the central portion of the property is cleared, but most of it has revegetated by 2002 and vehicle tracks are visible on this and adjoining properties. Dirt piles are visible in several years after 2011 but no pits are apparent.



No documents are visible through map direct and there are no indications of contamination. This site may be incorrectly mapped to this location, and a landfill occurs directly across Wiles Road from this site. Improvements are proposed to the overpass at the intersection of Florida's Turnpike and Wiles Road, but right-of-way would be acquired from this site. For these reasons, and because there is no documentation of contamination at this site, it is assigned a risk rating of low.

6.2.13 SITE 13 MONARCH HILL

2700 Wiles Road Facility ID: 8622531 Risk Rating: High

This site is an operating landfill located in the southeastern quadrant of the intersection of Wiles Road and Florida's Turnpike. The facility covers approximately 500 acres, does not accept hazardous waste and contains two above ground storage tanks. There is an associated waste transfer station and waste-to-energy facility to the east. The landfill is protected by a double composite liner system that include a geosynthetic clay line and a polyethylene membrane to isolate waste and wastewater. Groundwater is monitored through at least 70 wells on the site. On 7-2-2019 site personnel noticed an accidental release of leachate. The location of the release is approximately 2,072 feet from the east property boundary and 987 feet from the north property boundary. Some leachate entered a storm water retention area 340 feet to the east. The affected ground was estimated to be approximately 6,000 square feet and maximum volume of leachate released was estimated to be 3,500 gallons. Future soil sampling is anticipated for this site. A Return to Compliance letter was issued on 3/9/2021 regarding the Department's Storage Tank rules and regulations. Because this site is an active landfill that contains waste and because of the recent release of leachate and lack of an official completion or closure order, this site is assigned a risk rating of High.

6.2.14 SITE 14 MED-CARE PHARMACY INC.

1052 S. Powerline Road Facility ID: 9100756 Risk Rating: Low

This site is located adjacent to the project, in the southeast quadrant of the intersection of S.W. 10th Street and Powerline Road. FDEP documents include a Storage Tank Notification form from 1989 reporting the presence of one 550-gallon underground storage tank and a storage tank registration form from 2003



reporting a 1,000-gallong storage tank. This area was developed prior to 1995. There is no record of release of contaminants and the site has been developed since at least 1995, but because there are storage tanks present, this site is assigned a risk rating of Low.

6.2.15 SITE 15 SHELL-FIRST COAST ENERGY #1836

1011 S Powerline Road

Facility ID: 9800891

Risk Rating: Medium

This site is an operating gas station located in the southwestern quadrant of the intersection of S.W. 10th Street and S. Powerline Road, adjacent to the project. This retail fuel station contains five USTs for gasoline and diesel which were installed in October of 1998; four 10,000-gallon tanks and one 8,000-gallon tank. Groundwater flow in this area appears to generally be towards the north. A DRF dated 3/10/15 was completed when gasoline was observed leaking into surrounding soil from fill buckets during a closure assessment conducted while replacing fill buckets. Leaks were speculated to have originated from punctures to two of the USTs. Gravel was excavated down to approximately 3-feet below land surface in the area around each USTs fill and vapor bucket and were submitted for analysis. Results revealed that groundwater samples were above the Groundwater Cleanup Target Levels (GCTLs) for dissolved-phase hydrocarbon concentrations, but below Natural Attenuation Default Concentrations (NADCs), at two of the twelve Monitoring Wells located on-site. The remaining samples collected demonstrated dissolved phase hydrocarbons concentrations below GCTLs, and thus a Natural Attenuation Monitoring (NAM) Plan was requested by the facility on November 12, 2015. The NAM Plan was approved on February 25, 2016, by the Broward County Pollution Prevention Division (Division) on behalf of the Florida Department of Environmental Protection. In September 2017 a Site Rehabilitation and Completion Report and No Further Action Proposal were accepted by Broward County on behalf of FDEP. This site resolved all violations concerning ongoing cleanup efforts and has been advised to resume state-funded cleanup activities. Because this site is an operating gas station, it is assigned a risk rating of Medium.



6.2.16 SITE 16 FDOT BRIDGES AND OVERPASSES

Throughout the Project Facility ID: None Risk Rating: Medium

The project contains multiple bridges and overpasses that could potentially contain asbestos, lead-based paint, or metal-based coatings. A series of 2017 survey reports provide the results of analysis of several bridges in the project area. Those bridges include the Wiles Road bridge over Florida's Turnpike, the Sawgrass Expressway bridges over US 441, and the Sawgrass Expressway bridges over Lyons Road. For each of these bridges, the 2017 reports noted that no asbestos was detected, but lead and chromium were present. Because lead and chromium were detected, a risk rating of Medium is assigned to all bridges and overpasses. Additional surveys are recommended prior to construction to determine the presence and extent asbestos, lead-based paint, and metal-based coatings.



7.0 CONCLUSION AND RECOMMENDATIONS

A total of 16 sites of potential contamination risk were identified, including two High Risk, five Medium Risk, and nine Low Risk sites. None of these potentially contaminated sites are proposed for right-of-way acquisition. Level II Contamination Assessment investigations are recommended where proposed dewatering or subsurface work (e.g., pole foundations, drainage features, soil excavation, etc.) would occur at or adjacent to any sites rated High or Medium Risk. If dewatering is necessary during construction, a SFWMD Dewatering Permit will be required. The contractor will be held responsible for ensuring compliance with any necessary dewatering permit(s). A dewatering plan will be necessary to avoid potential exacerbation of any contamination plumes. All permits will be obtained in accordance with Federal, state, and local laws and regulations, and in coordination with the District Contamination Impact Coordinator.

Draft Pond Siting Report



APPENDIX F

Wellfield Protection Zone

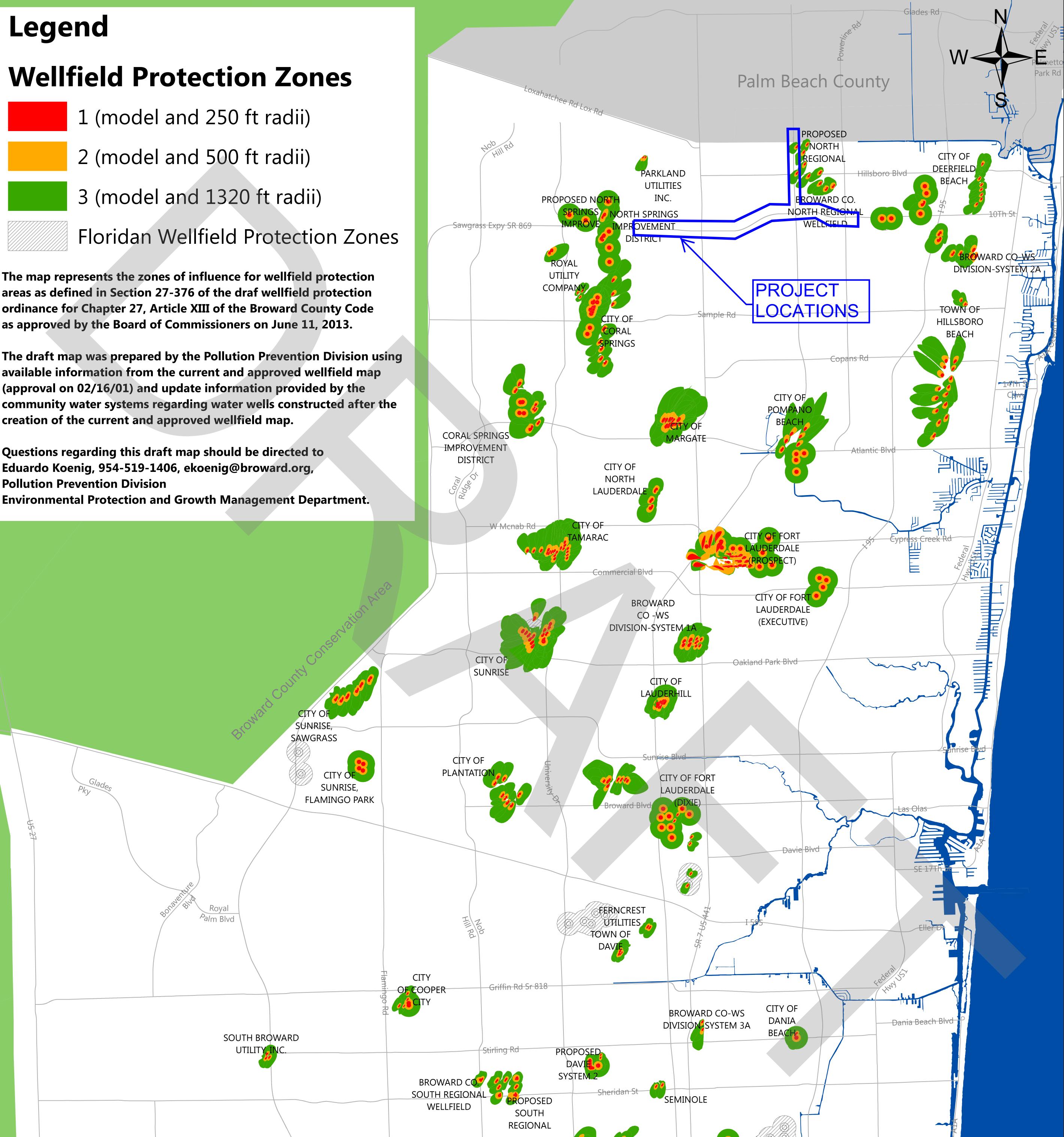


Broward County Commissioners Chapter 27- Article XIII Rule of 6/11/2013

Legend

Wellfield Protection Zones

The map represents the zones of influence for wellfield protection areas as defined in Section 27-376 of the draf wellfield protection ordinance for Chapter 27, Article XIII of the Broward County Code as approved by the Board of Commissioners on June 11, 2013.

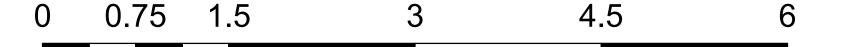


The draft map was prepared by the Pollution Prevention Division using available information from the current and approved wellfield map (approval on 02/16/01) and update information provided by the community water systems regarding water wells constructed after the creation of the current and approved wellfield map.

Questions regarding this draft map should be directed to Eduardo Koenig, 954-519-1406, ekoenig@broward.org, **Pollution Prevention Division**

Zone 1 means the wellfield protection area shown on the zones of influence map approved on February 16, 2001 for water wells included on that map, and within 250 feet from an existing water well for wells not included on that map. Zone 2 means the wellfield protection area shown on the zones of influence map approved on February 16, 2001 for water wells included on that map, and situated between the outer boundary of Zone 1 and 500 feet from an existing water well for wells not included on that map. Zone 3 means the wellfield protection area shown on the zones of influence map approved on February 16, 2001 for water wells included on that map, and situated between the outer boundary of Zone 2 and 1320 feet from an existing water well for wells not included on that map.







Update By N. Herne - POLLUTION PREVENTION DIVISION: 1/15/2016



Draft Pond Siting Report

APPENDIX G

Calculations

					TABLE 10.1	WATER QUA	ALITY AND O		ALCULATION SUM	MMARY				
				Basin	Area Calcu	ulation			Water Quality Calculation Summary	Water Quantity Calculation Summary	Required Storage For Stormwater Management	Provided Storage	Surplus Volume	
	Project Basin	Stat	tion	Side	Length	Total Area	Impervi	ous Area	Water Quality Treatment Req'd	Net Runoff Increase (Post-Pre)	Water Quality+Water Quantity	(4)	(5)	Remarks
									(1)	(2)	(3)			
No.	Description	From	То	LT/RT	(ft.)	(Ac.)	Exist.	Prop.	(Ac.ft.)	(Ac.ft.)	(Ac.ft.)	(Ac.ft.)	(Ac.ff.)	
1	From West of US-441 to US- 441	962+30	1041+00	LT/RT	7870	110.64	45.33	86.76	13.45	6.29	19.74	24.25	4.51	Dry Detention and Wet Detention
2	From US-441 to East of Lyons Road	1041+00	1110+00	LT/RT	6900	85.88	40.29	64.08	12.04	3.54	15.58	15.65	0.07	Dry Detention and Wet Detention
3A	From East of Lyons Road to FTE	1135+00	1153+00	LT/RT	1800	22.37	12.30	14.92	2.66	0.42	3.08	5.58	2.50	Dry Detention and Wet Detention
3B	Sawgrass Expressway from FTE to Waterways Blvd.	1110+00	1180+00	LT/RT	7000	158.37	68.10	98.91	14.46	4.67	19.13	48.11	28.98	Wet Detention. Required storage for Basin 4 is added in Required Storage
3B-1	FTE from north of the Sawgrass/FTE interchange to Hilsboro Canal	5652+38	5716+70	LT/RT	6432	44.27	18.66	30.89	6.44	1.82	8.25	2.40	-5.85	Dry Detention along right side
3B-2	Turnpike from Wiles Road to Sawgrass/FTE interchange	5572+50	5622+10	LT/RT	4960	36.01	0.00	24.39	5.08	3.71	8.79	4.90	-3.89	Dry Detention along right side
4	From Waterways Blvd. to Powerline Rd.	1180+00	1209+00	LT/RT	2900	17.80	0.00	20.47	3.16	18.55	21.71	0.00	-21.71	Wet Detention. Provided storage is Basin 3B
										Total 3A, 3B, 3B-1 & 4	60.96	60.99	0.03	

Notes:

(1) Water Quality Treatment required = Wet detention volume calculated for the first inch of runoff from the developed project, or the total runoff of 2.5 inches times the total impervious area, whichever is greater. See separate spreadsheet for detail calculation.

(2) Water Quantity required = Net Runoff generated from Design Storm (25YR-72HR) = Post Development Runoff - Pre Development Runoff. See separate spreadsheets for detail calculation.

(3) Required Storage for Stormwater management (Water Quality + Water Quantity) = (1) + (2)

(4) From Table 2 Wet and Dry Detention System Storage Calculation

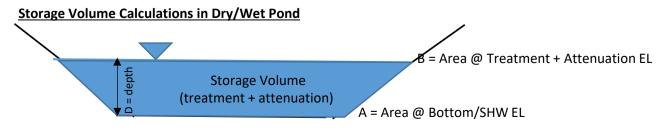
(5) Surplus Volume (5) = (4) - (3). Landscaping needs can be fulfilled at the area where surplus volume is available.

Project No. 437153-1-22-01

BASIN		WATER MAN	AGEMENT F	ACILITIE	ES		NTROL TTOM EL		MENT AND UATION EL	STORAGE	REMARKS
No.	DECOUDTION	From Cho	To Cho	Side	TREATMENT	Elev.	Area	Elev	Area	Volume	Minimum Berm
	DESCRIPTION	From Sta.	To Sta.	Side	METHOD	(ft.)	(Ac.)	(ft.)	(Ac.)	(Ac-ft)	EL
	Swale 1-1	962+50	1012+00	LT	Dry Detention	11	0.57	12.5	0.66	0.92	14.50
	Swale 1-2	962+50	1009+00	RT	Dry Detention	11	1.71	12.5	1.86	2.68	13.50
	Swale 1-3	1023+50	1030+50	LT	Dry Detention	11	0.39	12.5	0.47	0.64	13.50
	Swale 1-4	1028+00	1040+00	RT	Dry Detention	11	0.43	12.5	0.51	0.70	13.50
1	Dry Pond 1-1	1030+00	1035+00	LT	Dry Detention	11	1.19	12.5	1.32	1.88	13.50
Т								ΣDry	Detention =	6.83	
	Wet Pond 1-1	1030+00	1039+00	LT	Wet Detention	10	7.73	11.5	8.05	11.84	14.50
	Wet Pond 1-2	1035+00	1040+00	LT	Wet Detention	10	3.61	11.5	3.83	5.58	14.50
									Detention =	17.42	
		-						_		SIN 1 = 24.25	_
	Dry Pond 2-1	1042+00	1043+00	RT	Dry Detention	11	0.21	12.5	0.27	0.36	13.50
	Swale 2-1	1042+00	1046+00	RT	Dry Detention	11	0.23	12.5	0.32	0.41	13.20
	Swale 2-2	1042+50	1058+50	LT	Dry Detention	11	0.65	12.5	0.75	1.05	13.20
	Swale 2-3	1047+00	1052+50	RT	Dry Detention	11	0.20	12.5	0.25	0.34	13.20
	Dry Pond 2-2	1049+00	1053+00	RT	Dry Detention	11	0.99	12.5	1.11	1.57	13.45
	Swale 2-4	1076+00	1091+00	RT	Dry Detention	11	0.41	12.5	0.49	0.67	13.20
2	Swale 2-5	1077+00	1083+00	LT	Dry Detention	11	0.09	12.5	0.13	0.16	13.45
	Swale 2-6	1082+50	1092+50	RT	Dry Detention	11	0.32	12.5	0.39	0.53	13.45
	Dry Pond 2-3	1090+50	1092+50	RT	Dry Detention	11	0.21	12.5	0.27	0.36	13.45
	Dry Pond 2-4	1095+00	1097+00	LT	Dry Detention	11	0.17	12.5	0.22	0.29	13.45
	Dry Pond 2-5	1095+00	1098+00	LT	Dry Detention	11	0.13	12.5	0.17	0.23	13.45
	Mat David 2.4	1042.50	1040.50		Mat Datastics		6.20		Detention =	5.98	42.05
	Wet Pond 2-1	1042+50	1048+50	RT	Wet Detention	9.5	6.30	11.0	6.59	9.67	12.95
				_		TOTAL			Detention =	<i>9.67</i> 5IN 2 = 15.65	
	Wet Pond 3A-1	1147+00	1151+00	LT	Wet Detention	8.4	1.21	9.9	1.27	1.86	12.95
	Wet Pond 3A-2	1149+00	1151+00	LT	Wet Detention	8.4	0.86	9.9	0.91	1.33	12.95
	Wet Fond SA-2	1149+00	1131+00		Wet Detention	0.4	0.80		Detention =	3.19	12.95
3A	Dry Pond 3A-1	1136+00	1143+00	LT	Dry Detention	11	0.64	12.5	0.69	1.00	13.45
5/1	Dry Pond 3A-2	1145+00	1150+00	LT	Dry Detention	11	0.90	12.5	0.96	1.39	13.45
	Dry rond SA 2	1145100	1150.00		Dry Detention	1 1	0.50		Detention =	2.39	13.45
						TOTAL	AVAILABI			IN 3A = 5.58	AC-FT
	Wet Pond 3B-1	1147+50	1151+00	RT	Wet Detention	8.4	0.85	9.9	0.90	1.32	11.9
	Wet Pond 3B-2	1155+00	1162+50	LT	Wet Detention	8.4	6.08	9.9	6.22	9.23	11.9
	Wet Pond 3B-3	1155+00	1159+00	RT	Wet Detention	8.4	12.90	9.9	13.11	19.51	11.9
	Wet Pond 3B-4	1156+00	1163+00	LT	Wet Detention	8.4	2.09	9.9	2.17	3.20	11.9
	Wet Pond 3B-5	1155+00	1163+00	RT	Wet Detention	8.4	7.59	9.9	7.75	11.50	11.9
3B								ΣWet	Detention =	44.75	
	Dry Pond 3B-1	1090+50	1092+50	RT	Dry Detention	9.4	0.61	10.9	0.70	0.98	11.90
	Dry Pond 3B-2	1090+50	1092+50	RT	Dry Detention	9.4	0.49	10.9	0.57	0.80	11.90
	Dry Pond 3B-3	1090+50	1092+50	RT	Dry Detention	9.4	0.99	10.9	1.11	1.57	11.90
	-					I		ΣDry	Detention =	3.36	
					•	TOTAL A	VAILABL	E STORA	GE AT BAS	N 3B = 48.1	1 AC-FT
3B-1	Swale 3B-1	5651+00	5678+00	RT	Dry Detention	9.4	1.53	10.9	1.68	2.40	11.90
20-T						OTAL A	VAILABL	E STORA	GE AT BASI	N 3B-1 = 2.4	0 AC-FT

Notes:

- 1. Swale and Pond locations are shown in the associated Drainage Map.
- 2. Swale and pond sections are estimated from existing plans and conceptual roadway design. See associated exhibit
- 3. Treatment volume provided by new roadside swales or enlarging interchange ponds.
- 4. All Elevations are in feet NAVD-88 datum. Datum Conversion factor, NAVD EL = NGVD EL 1.529'



Storage Volume = Average of (A & B)X D

Drainage Area: Basin 1 POND No. OUTFALL Pine Tree WCD WATER QUALITY CRITERIA FROM SFWMD

	AREAS			
DATA:	PRE-DEV.	POST-DEV.		
FROM STA. ft	962+30	962+30		
to sta. ft	1041+00	1041+00		
LENGHT ft	7870	7870		
ROW WIDTH ft	612.4	612.4		
PAVED WIDTH ft	173.85	357.22		
TOTAL AREA				
INSIDE ROW AC	110.64	110.64		
OUTSIDE ROW AC	0	0		
TOTAL AREA AC	110.64	110.64		
	110.04	110.04		
IMP. AREA				
PAVED AREAS AC	31.41	64.54		
WET OUT AREA AC	13.92	22.22		
OTHER IMP. AREA AC	0.00	0.00		
TOTAL (Ai) Ac	45.33	86.76		
PER. AREA	65.31	23.88		
Ap Ac	65.31	23.88		
	SFW	/MD		
WET DETENTION	PRE-DEV.	POST-DEV		
1" on the Basin Ac-ft		9.22		
2.5" on Pav. Area Ac-ft		13.45		
Greater of Above Ac-ft		13.45		

SFW	MD							
PRE-DEV.								
9.22								
	13.45							
	13.45							

PEAK ATTENUATION: SCS METHOD

-			POST-DEV.			
SOIL TYPE D		E-DEV.				
	CN	AREA (Ac)	CN	AREA (Ac)		
IMP. AREA						
Paved Areas	98	31.41	98	64.54		
Lakes and wet areas	100	13.92	100	22.22		
Other	98	0.00	98	0.00		
SUB-TOTAL (Ai)	98.6	45.33	98.5	86.76		
PER. AREA						
Gravel Roads	91	0.00	91	0.00		
Dirt Roads	89	0.00	89	0.00		
Cultivated Land	91	0.00	91	0.00		
Pasture or range	80	0.00	80	0.00		
Meadow, good cond.	78	0.00	78	0.00		
Wood or forest land	83	0.00	83	0.00		
Lawns/sod, fair cond.	84	65.31	84	23.88		
Other	0	0.00	0	0.00		
SUB-TOTAL (Ap)	84.0	65.31	84.0	23.88		
TOTAL AREA (At= Ai+Ap)						
CNw=Sum(A*CN)/At	90.0	110.64	95.4	110.64		
_						
DESIGN RAINFALL (25yr-72hr) (P)	in	13		13		
WATERSHED STORAGE: S=(1000/CNw)-10	in	1.11		0.48		
DIRECT RUNOFF: R=(P-0.2S) ² /(P+0.8S)	in	11.75		12.44		
TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	108.37		114.66		
NET RUNOFF = POST DEV. RUNC	OFF - PRE	DEV. RUNOFF	Ac-ft	6.29		

Project No. 437153-1-22-01

Date: 11/28/2023

Designed By: IS

AREAS

Drainage Area: Basin 2 POND No. **OUTFALL Cocomar WCD** WATER QUALITY CRITERIA FROM SFWMD

DATA:	PRE-DEV.	POST-DEV.
FROM STA. ft	1041+00	1041+00
TO STA. ft	1110+00	1110+00
LENGHT ft	6900	6900
ROW WIDTH ft	542.15	542.15
PAVED WIDTH ft	223.48	364.77
TOTAL AREA		
INSIDE ROW AC	85.88	85.88
OUTSIDE ROW AC	0	0
TOTAL AREA AC	85.88	85.88
IMP. AREA		
PAVED AREAS AC	35.40	57.78
WET OUT AREA AC	4.89	6.30
OTHER IMP. AREA AC	0.00	0.00
TOTAL (Ai) Ac	40.29	64.08
PER. AREA		
Ap Ac	45.59	21.80
	-	MD
WET DETENTION	PRE-DEV.	POST-DEV
1" on the Basin Ac-ft		7.16
2.5" on Pav. Area Ac-ft		12.04
Greater of Above Ac-ft		12.04

Project No. 437153-1-22-01

3.54

Designed By: IS Date: 11/28/2023

PEAK ATTENUATION: SCS METHOD

SOIL TYPE D	PR	E-DEV.	POS	T-DEV.
	CN	AREA (Ac)	CN	AREA (Ac)
IMP. AREA				
Paved Areas	98	35.40	98	57.78
Lakes and wet areas	100	4.89	100	6.30
Other	98	0.00	98	0.00
SUB-TOTAL (Ai)	98.2	40.29	98.2	64.08
PER. AREA				
Gravel Roads	91	0.00	91	0.00
Dirt Roads	89	0.00	89	0.00
Cultivated Land	91	0.00	91	0.00
Pasture or range	80	0.00	80	0.00
Meadow, good cond.	78	0.00	78	0.00
Wood or forest land	83	0.00	83	0.00
Lawns/sod, fair cond.	84	45.59	84	21.80
	0	0.00	0	0.00
SUB-TOTAL (Ap)	84.0	45.59	84.0	21.80
TOTAL AREA (At= Ai+Ap)				
CNw=Sum(A*CN)/At	90.7	85.88	94.6	85.88
_				
DESIGN RAINFALL (25yr-72hr) (P)	in	13		13
	:. <u>.</u>	1.02		0.57
WATERSHED STORAGE: S=(1000/CNw)-10 DIRECT RUNOFF: R=(P-0.2S) ² /(P+0.8S)	in :	1.03		0.57
	in	11.84		12.34
TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	84.76		88.30

> NET RUNOFF = POST DEV. RUNOFF - PRE DEV. RUNOFF Ac-ft

Drainage Area: Basin 3A POND No. OUTFALL Cocomar WCD WATER QUALITY CRITERIA FROM SFWMD

Designed By: IS Date: 11/28/2023 PEAK ATTENUATION: SCS METHOD

	AR	EAS	SOIL TYPE D	PR	E-DEV.	POS	ST-DEV.
DATA:	PRE-DEV.	POST-DEV.		CN	AREA (Ac)	CN	AREA (Ac)
FROM STA. ft	1135+00	1135+00	IMP. AREA				
TO STA. ft	1153+00	1153+00	Paved Areas	98	11.68	98	12.79
LENGHT ft	1800	1800	Lakes and wet areas	100	0.62	100	2.13
ROW WIDTH ft	541.4	541.4	Other	98	0.00	98	0.00
PAVED WIDTH ft	282.65	309.52	SUB-TOTAL (Ai)	98.1	12.30	98.3	14.92
TOTAL AREA			PER. AREA				
INSIDE ROW AC	22.37	22.37	Gravel Roads	91	0.00	91	0.00
OUTSIDE ROW AC	0	0	Dirt Roads	89	0.00	89	0.00
TOTAL AREA AC	22.37	22.37	Cultivated Land	91	0.00	91	0.00
			Pasture or range	80	0.00	80	0.00
IMP. AREA			Meadow, good cond.	78	0.00	78	0.00
PAVED AREAS AC	11.68	12.79	Wood or forest land	83	0.00	83	0.00
WET OUT AREA AC	0.62	2.13	Lawns/sod, fair cond.	84	10.07	84	7.45
OTHER IMP. AREA AC	0.00	0.00	Other	0	0.00	0	0.00
TOTAL (Ai) AC	12.30	14.92	SUB-TOTAL (Ap)	84.0	10.07	84.0	7.45
PER. AREA			TOTAL AREA (At= Ai+Ap)				
Ap Ac	10.07	7.45	CNw=Sum(A*CN)/At	91.8	22.37	93.5	22.37
	SFW	/MD	DESIGN RAINFALL (25yr-72hr) (P)	in	13		13
WET DETENTION	PRE-DEV.	POST-DEV					
1" on the Basin Ac-ft		1.86	WATERSHED STORAGE: S=(1000/CNw)-10	in	0.90		0.69
2.5" on Pav. Area Ac-ft		2.66	DIRECT RUNOFF: R=(P-0.2S) ² /(P+0.8S)	in	11.98		12.20
Greater of Above Ac-ft		2.66	TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	22.34		22.75
			NET RUNOFF = POST DEV. RUNO	FF - PRE I	DEV. RUNOFF	Ac-ft	0.42

Drainage Area: Basin 3B POND No. **OUTFALL BCWCD No.2** WATER QUALITY CRITERIA FROM SFWMD

	AREAS		SOIL TYPE D	PR	E-DEV.	POS	T-DEV.
DATA:	PRE-DEV.	POST-DEV.		CN	AREA (Ac)	CN	AREA (Ac)
FROM STA. ft	1110+00	1110+00	IMP. AREA				
TO STA. ft	1180+00	1180+00	Paved Areas	98	42.66	98	69.41
LENGHT ft	7000	7000	Lakes and wet areas	100	25.44	100	29.50
ROW WIDTH ft	985.51	985.51	Other	98	0.00	98	0.00
PAVED WIDTH ft	265.47	431.93	SUB-TOTAL (Ai)	98.7	68.10	98.6	98.91
TOTAL AREA			PER. AREA				
INSIDE ROW AC	158.37	158.37	Gravel Roads	91	0.00	91	0.00
OUTSIDE ROW AC	0	0	Dirt Roads	89	0.00	89	0.00
TOTAL AREA AC	158.37	158.37	Cultivated Land	91	0.00	91	0.00
			Pasture or range	80	0.00	80	0.00
IMP. AREA			Meadow, good cond.	78	0.00	78	0.00
PAVED AREAS AC	42.66	69.41	Wood or forest land	83	0.00	83	0.00
WET OUT AREA AC	25.44	29.50	Lawns/sod, fair cond.	84	90.27	84	59.46
OTHER IMP. AREA AC	0.00	0.00	Other	0	0.00	0	0.00
TOTAL (Ai) Ac	68.10	98.91	SUB-TOTAL (Ap)	84.0	90.27	84.0	59.46
PER. AREA			TOTAL AREA (At= Ai+Ap)				
Ap Ac	90.27	59.46	CNw=Sum(A*CN)/At	90.3	158.37	93.1	158.37
		IMD	DESIGN RAINFALL (25yr-72hr) (P)	in	13		13
WET DETENTION	PRE-DEV.	POST-DEV					
1" on the Basin Ac-ft		13.20	WATERSHED STORAGE: S=(1000/CNw)-10	in	1.07		0.74
2.5" on Pav. Area Ac-ft		14.46	DIRECT RUNOFF: R=(P-0.2S) ² /(P+0.8S)	in	11.80		12.15
Greater of Above Ac-ft		14.46	TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	155.72		160.39
			NET RUNOFF = POST DEV. RUNO	FF - PRE I	DEV. RUNOFF	Ac-ft	4.67

Project No. 437153-1-22-01

Designed By: IS Date: 11/28/2023

PEAK ATTENUATION: SCS METHOD

Drainage Area: Basin 3B-1 POND No. OUTFALL BCWCD No.2 WATER QUALITY CRITERIA FROM SFWMD

	AREAS		SOIL TYPE D	PR	E-DEV.	POS	T-DEV.
DATA:	PRE-DEV.	POST-DEV.		CN	AREA (Ac)	CN	AREA (Ac)
FROM STA. ft	5652+38	5652+38	IMP. AREA				
to sta. ft	5716+70	5716+70	Paved Areas	98	18.66	98	30.89
	(120	(120		100	0.00	100	0.00
LENGHT ft ROW WIDTH ft	6432	6432 299.81	Lakes and wet areas	100	0.00 0.00	100	0.00
	299.81			98		98	0.00
PAVED WIDTH ft	126.37	209.2	SUB-TOTAL (Ai)	98.0	18.66	98.0	30.89
TOTAL AREA			PER. AREA				
INSIDE ROW AC	44.27	44.27	Gravel Roads	91	0.00	91	0.00
OUTSIDE ROW AC	0	0	Dirt Roads	89	0.00	89	0.00
TOTAL AREA AC	44.27	44.27	Cultivated Land	91	0.00	91	0.00
			Pasture or range	80	0.00	80	0.00
IMP. AREA			Meadow, good cond.	78	0.00	78	0.00
PAVED AREAS AC	18.66	30.89	Wood or forest land	83	0.00	83	0.00
WET OUT AREA AC	0.00	0.00	Lawns/sod, fair cond.	84	25.61	84	13.38
OTHER IMP. AREA AC	0.00	0.00	Other	0	0.00	0	0.00
TOTAL (Ai) Ac	18.66	30.89	SUB-TOTAL (Ap)	84.0	25.61	84.0	13.38
							
PER. AREA	05 (1	13.38		89.9	44.27	93.8	44.27
Ар Ас	25.61	13.38	CNw=Sum(A*CN)/At	89.9	44.27	93.8	44.27
	SFW	/MD	DESIGN RAINFALL (25yr-72hr) (P)	in	13		13
WET DETENTION	PRE-DEV.	POST-DEV					
1" on the Basin Ac-ft		3.69	WATERSHED STORAGE: S=(1000/CNw)-10	in	1.12		0.66
2.5" on Pav. Area Ac-ft		6.44	DIRECT RUNOFF: R=(P-0.2S) ² /(P+0.8S)	in	11.74		12.24
Greater of Above Ac-ft		6.44	TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	43.32		45.14
			NET RUNOFF = POST DEV. RUNOF	FF - PRE I	DEV. RUNOFF	Ac-ft	1.82
							B

Project No. 437153-1-22-01

Designed By: IS Date: 11/28/2023

PEAK ATTENUATION: SCS METHOD

Drainage Area: Basin 3B-2 POND No. OUTFALL BCWCD No.2 WATER QUALITY CRITERIA FROM SFWMD

	AR	EAS	SOIL TYPE D	PR	E-DEV.	POS	T-DEV.
DATA:	PRE-DEV.	POST-DEV.		CN	AREA (Ac)	CN	AREA (Ac)
FROM STA. ft	5572+50	5572+50	IMP. AREA				
TO STA. ft	5622+10	5622+10	Paved Areas	98	0.00	98	24.39
LENGHT ft	4960	4960	Lakes and wet areas	100	0.00	100	0.00
ROW WIDTH ft	316.25	316.25	Other	98	0.00	98	0.00
PAVED WIDTH ft	0	214.2	SUB-TOTAL (Ai)	98.0	0.00	98.0	24.39
TOTAL AREA			PER. AREA				
INSIDE ROW AC	36.01	36.01	Gravel Roads	91	0.00	91	0.00
OUTSIDE ROW AC	0	0	Dirt Roads	89	0.00	89	0.00
TOTAL AREA AC	36.01	36.01	Cultivated Land	91	0.00	91	0.00
			Pasture or range	80	0.00	80	0.00
IMP. AREA			Meadow, good cond.	78	0.00	78	0.00
PAVED AREAS AC	0.00	24.39	Wood or forest land	83	0.00	83	0.00
WET OUT AREA AC	0.00	0.00	Lawns/sod, fair cond.	84	36.01	84	11.62
OTHER IMP. AREA AC	0.00	0.00	Other	0	0.00	0	0.00
TOTAL (Ai) Ac	0.00	24.39	SUB-TOTAL (Ap)	84.0	36.01	84.0	11.62
PER. AREA			TOTAL AREA (At= Ai+Ap)				
Ap Ac	36.01	11.62	CNw=Sum(A*CN)/At	84.0	36.01	93.5	36.01
	SFW	/MD	DESIGN RAINFALL (25yr-72hr) (P)	in	13		13
WET DETENTION	PRE-DEV.	POST-DEV					
1" on the Basin Ac-ft		3.00	WATERSHED STORAGE: S=(1000/CNw)-10	in	1.90		0.70
2.5" on Pav. Area Ac-ft		5.08	DIRECT RUNOFF: R=(P-0.2S) ² /(P+0.8S)	in	10.96		12.20
Greater of Above Ac-ft		5.08	TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	32.90		36.61
			NET RUNOFF = POST DEV. RUNO	FF _ PRF		Ac-ft	3.71
							0.71

Project No. 437153-1-22-01

Designed By: IS Date: 11/28/2023

PEAK ATTENUATION: SCS METHOD

Drainage Area: Basin 4 (No Pond) POND No. OUTFALL BCWCD No.2 WATER QUALITY CRITERIA FROM SFWMD

Designed By: AP Date: 11/28/2023 PEAK ATTENUATION: SCS METHOD SOIL TYPE D PRE-DEV. POST-DEV.

AREAS	SOIL TYPE D	PR	E-DEV.	POST-DEV.		
PRE-DEV. POST-DEV.		CN	AREA (Ac)	CN	AREA (A	
ft 1180+00 1180+00	IMP. AREA					
ft 1209+00 1209+00	Paved Areas	98.0	7.56	98	15.17	
ft 2900 2900	Lakes and wet areas	100	0.00	100	0.00	
ft 254.3 267.37	Other	98	0.00	98	0.00	
ft 113.55 245.43	SUB-TOTAL (Ai)	98.0	7.56	98.0	15.17	
Ac 16.93 17.80	PER. AREA Gravel Roads	91	0.00	91	0.00	
Ac 0 0.00	Dirt Roads	89	0.00	89	0.00	
Ac 16.93 17.80	Cultivated Land	91	0.00	91	0.00	
10.75 17.00	Pasture or range	80	0.00	80	0.00	
	Meadow, good cond.	78	0.00	78	0.00	
Ac 7.56 15.17	Wood or forest land	83	0.00	83	0.00	
Ac 0.00 0.00	Lawns/sod, fair cond.	84	0.00 9.37	83 84	2.63	
Ac 0.00 0.00	Other	04	0.00	04 0	0.00	
	SUB-TOTAL (Ap)	84.0	9.37	84.0	2.63	
Ac 7.56 15.17	SUB-IOTAL (AD)	04.0	7.37	04.0	2.03	
	TOTAL AREA (At= Ai+Ap)					
Ac 9.37 2.63	CNw=Sum(A*CN)/At	90.3	16.93	95.9	17.80	
SFWMD	DESIGN RAINFALL (25yr-72hr) (P)	in	13		13	
PRE-DEV. POST-DEV			15		10	
Ac-ft 1.48	WATERSHED STORAGE: S=(1000/CNw)-10	in	1.08		0.42	
Ac-ft 3.16	DIRECT RUNOFF: R=(P-0.2S)2/(P+0.8S)	in	11.79		12.50	
Ac-ft 3.16	TOTAL RUNOFF: (Rt=At*R/12)	Ac-ft	0.00		12.50	
0.10			0.00		10.00	

Project No. 437153-1-22-01

AS-BUILT PLANS AND SWALE STORAGE CALCULATION BACK UP

COMPONENTS OF CONTRACT PLANS SET

ROADWAY PLANS SIGNING AND PAVEMENT MARKING PLANS LIGHTING PLANS STRUCTURE PLANS & SOUNDWALL PLANS LANDSCAPE PLANS

A DETAILED INDEX APPEARS ON THE KEY SHEET OF EACH COMPONENT

INDEX OF ROADWAY PLANS

SHEET NO. SHEET DESCRIPTION KEY SHEET 2 - 15 DRAINAGE MAPS 16 - 18 TYPICAL SECTIONS 19 - 23 SUMMARY OF DRAINAGE STRUCTURES CONSTRUCTION DETAILS 24 - 27 28 - 29 DRAINAGE DETAILS 30 GENERAL NOTES 31 - 66 ROADWAY PLANS 67 - 102 ROADWAY PROFILES 103 - 128 DRAINAGE STRUCTURE 129 - 295G CROSS SECTIONS 296 ROADWAY SOILS SURV 297 - 298 SWPPP PNC-1 - PNC-9 PROJECT NETWORK CO MOT-01 - MOT-59 TRAFFIC CONTROL PL

GOVERNING STANDARDS AND SPECIFICATIONS: FLORIDA DEPARTMENT OF TRANSPORTATION, DESIGN STANDARDS DATED JANUARY 2004, AND STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION DATED 2004, AS AMENDED BY CONTRACT DOCUMENTS.

APPLICABLE DESIGN STANDARDS MODIFICATIONS: 7-1-05 For Design Standards Modifications click on "Design Standards" at the following web site: http://www.dot.state.fl.us/rddesign/

REVISIONS:

- ROADWAY SHEETS MOT-1, MOT-20, MOT-21, MOT-23, MOT-28, MOT-29, MOT-32, MOT-35, MOT-36, MOT-38, MOT-39, MOT-42, MOT-43, MOT-46, MOT-47, MOT-49, MOT-50, MOT-54, & MOT-55 (REVISED 08-02-05)
- ROADWAY SHEETS 16, 25, 29, 30, 32, 33, 34, 37, 38, 39, 40, 41, 42, 43, 45, 46, 47, 49, 51, 52, 56, 85, 86, 106, MOT-1, MOT-2, MOT-5, MOT-13, MOT-33, MOT-34, MOT-37, MOT-40, MOT-41, MOT-44, MOT-45, MOT-48, MOT-51, MOT-52 & MOT-53 (REVISED 08-11-05)
- ROADWAY SHEETS 11, 13, 23, 25, 30, 54, 59, 67-101, 123, 123A, 127, 129-295, 297, 298 MOT-1, MOT-5, MOT-11, MOT-13, MOT-15, MOT-16, MOT-17, MOT-18, MOT-34, MOT-37, MOT-41, MOT-45, MOT-48, MOT-52 (REVISED OB
- ROADWAY SHEETS 30, 41-44, 172-183, 278, 279, MOT 44, & MOT-45 (REVISED 12-
- ROADWAY SHEETS 32-35, 133-144, 261, 262, 270-275 (REVISED 03-10-06)
- ROADWAY SHEETS 36-39, 148-164 (REVISED 01-24-06)
- ROADWAY SHEETS 45-49, 189-203, MOT-12, MOT-52 (REVISED 02-06-06)
- ROADWAY SHEETS 1-7, 16-25, 32-66, 68-100, 104-107, 109-124, 127, 128, 130-287, 294-2956, MOT-15, MOT-34, MOT-37, MOT-41, MOT-45, MOT-48, MOT-52, MOT-58, MOT-59 (REVISED 08-09-1
- ROADWAY SHEETS 19-21, 23, 37, 38, 105-107 (REVISED 06-28-07)
- LIGHTING SHEETS U, L-2A thru L-9, L-12, L-17, L-19 thru L-22, L-31, L-34 thru L-36, L-40 thru L-42, L-49 thru L-53 (08-09-07)
- SIGNING & PAVEMENT MARKING SHEETS 5-1, 5-4 Thru 5-7, 5-12, 5-20, 5-26, 5-29, 5-30, 5-31, 5-33, 5-37 Thru 5-42, 5-44 Thru 5-46, 5-52, 5-53, 5-55, 5-57, 5-58 & 5-59 (08-18-06) SOUNDWALL SHEETS 1.1, 8.00, 8.01, 8.02, 12.06, 12.07, 12.13, 18.08, 19.01, 1.7 (07-20-06)

DEPARTMENT OF TRANSPORTATION

STATE OF FLORIDA



CONTRACT PLANS

LOCATION OF PROJECT

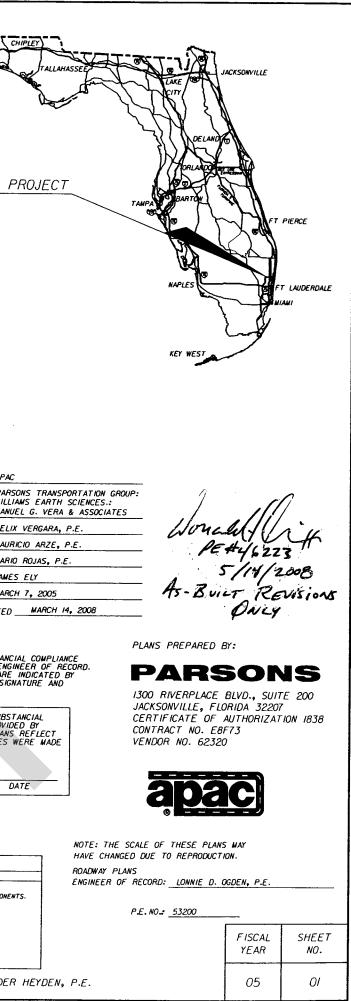
END PROJECT

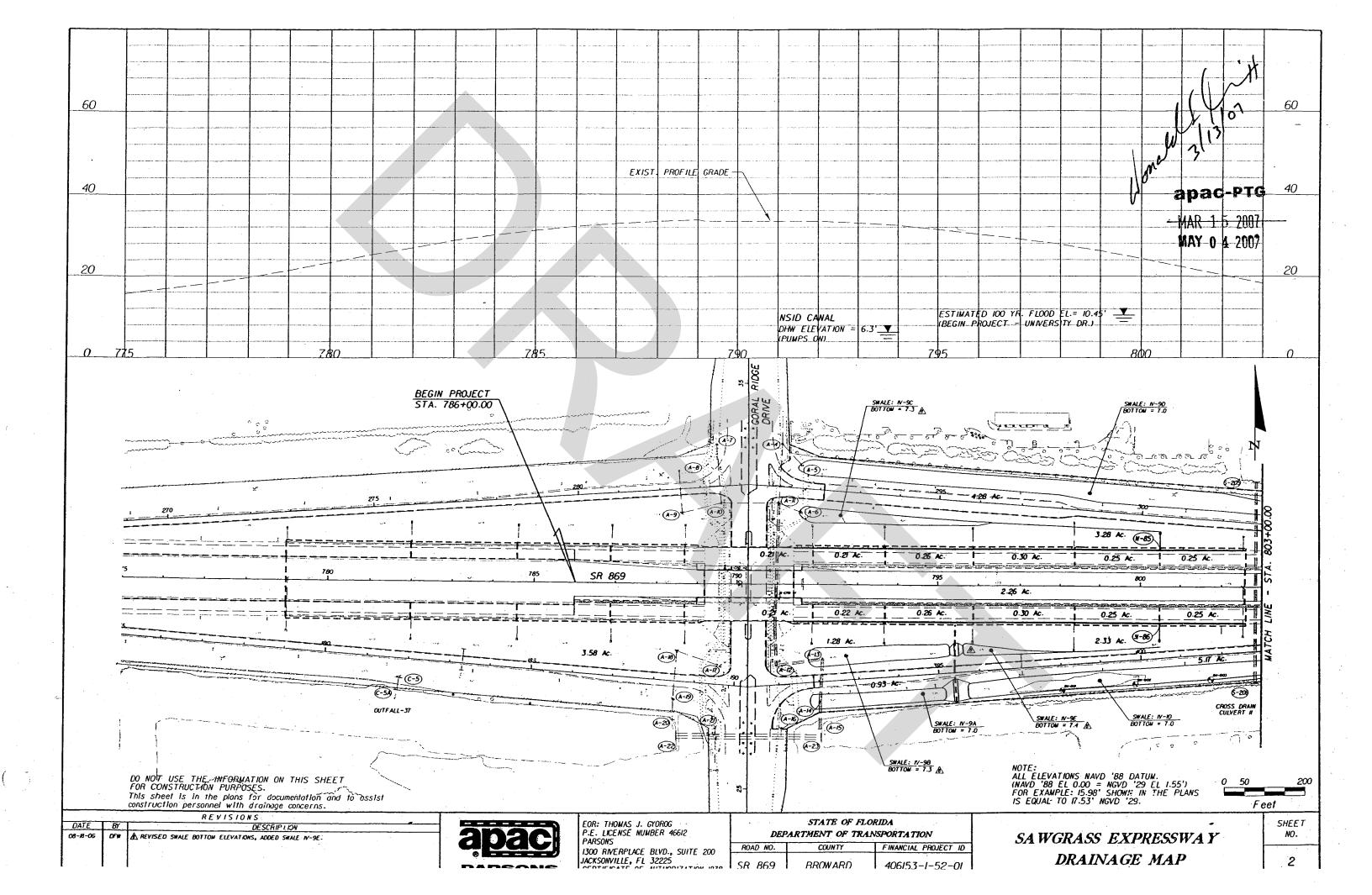
PENSAM

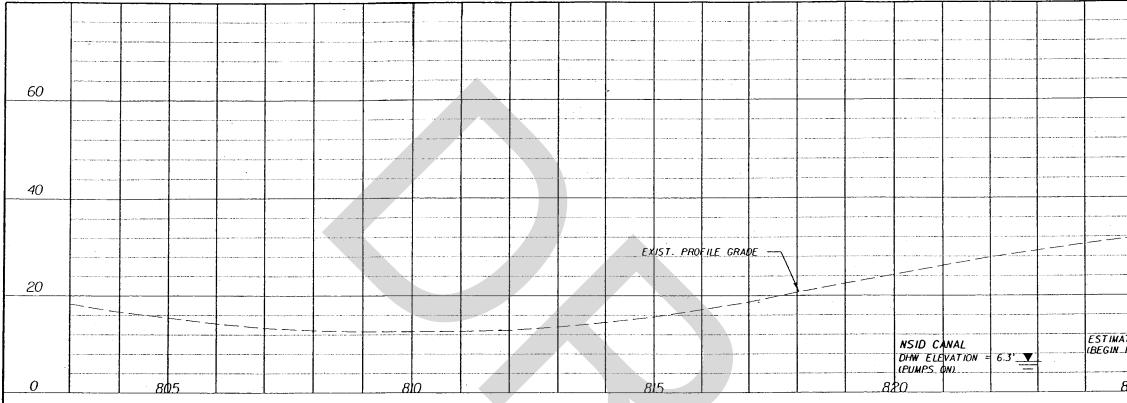
FINANCIAL PROJECT ID 406153-1-52-01 BROWARD COUNTY (86472) STATE ROAD NO. 869

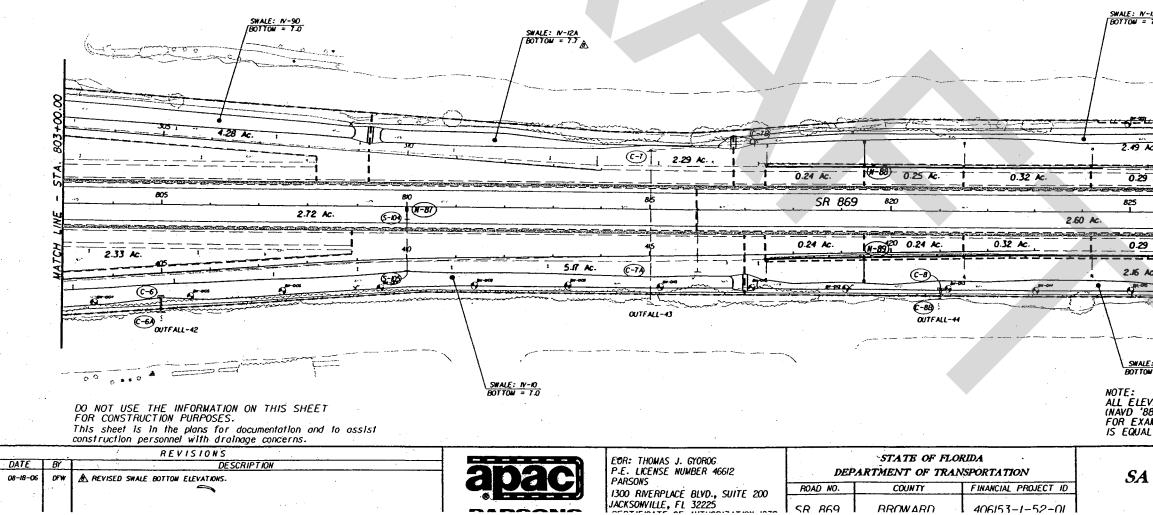
-41-E

		44			
	. Villannuntulli.		Isboro Canal	/ € STA. 1152+00 M.P. 20.712	
RES	22	35 All Mine			
RV EY				<u>T-47-S</u> T-48-S	
	PARKLAND			T~48-S	
CONTROL SHEETS PLANS O I 2	Holmberg Pop. 3, 558	Aur 441 }	Entry Aug		
		(Tail)			
Miles	The Erit to franciscom morning		Evit No 1 1	NAME OF CONTRACTOR	APAC
	Kura Line Line			NAME OF ALL CONSULTANTS INVOLVED	PARSONS WILLIAMS L MANUEL G.
	i CORAL	by by	()	RESIDENT ENGINEER	FELIX VER
	SPR 1 NGS		Sample Exit	PROJECT ADMINISTRATOR	MAURICIO
				CONSTRUCTION PROJECT MANAGER	MARIO ROJ
		плиноно		TURNPIKE CHIEF EXECUTIVE OFFICER	JAMES ELY
<u>BEGIN PROJECT</u> ♀ STA. 786+00	B B B B Royal		Brin for Course	DATE WORK STARTED	MARCH 7,
M.P. 13.781			COCONUT	DATE WORK FINAL ACCEPTANCE OR COMP	PLETEDM
			Pep. 27, 485		
		MARGATE	Coronal Creek	THIS PROJECT WAS CONSTRUCTED IN SU	BSTANCIAL CO
28,		Pep. 42, 985	10 "	THIS PROJECT WAS CONSTRUCTED IN SU WITH THESE PLANS AS PROVIDED BY TH IF CHANGES WERE MADE, THOSE CHANCE BLACK INK REVISION AND BEAR THE SEJ DATE OF THE RESPONSIBLE ENGINEER.	HE ENGINEER ES ARE INDIC
, T-48:	-S Skiller			DATE OF THE RESPONSIBLE ENGINEER.	AL, SIGNATUR
40 .	-5	voress <u>; Cr. </u> Conal	E Fill US	THIS PROJECT WAS CONSTRUCTED I	N SUBSTANCIA
7-37.			Pompana 5- 378	THIS PROJECT WAS CONSTRUCTED IN COMPLIANCE WITH THESE PLANS AS THE ENGINEER OF RECORD. THESE "AS-BUILTS" CONDITIONS AND NO CH.	PROVIDED BY
-37,			Service Plaza	"AS-BUILTS" CONDITIONS AND NO CH. TO THE PLAN SHEETS.	ANGES WERE
234,				RESIDENT ENGINEER	DATE
17, MOT-18, REVISED 08-31-05)					
MOT-45 (REVISED 12-06-05)					
SED 03-10-061	LENGTH	OF PROJEC	CT		
D 02-06-06)		LINEAR FEET	MILES	- KEY SHEET REVISIO	
24,	ROADWAY	35325.00	6.690	08-18-06 DFW ADDED SOUNDWALL	
, REVISED 08-09-07)	BRIDGES	1275.00	0.242		COLF ON LATO
18-07)	NET LENGTH OF PROJECT	36600.00	6.932		
L-22, L-31, L-34 thru L-36,	EXCEPTIONS	0.00	0.00		
5-12, 5-20, 5-26, 5-29, 5-30, 5-31, 5-55, 5-57, 5-58 & 5-59 (08-18-06)	GROSS LENGTH OF PROJECT	36600.00	6.932		
18.08, 19.01, 1.7 (07~20-06)	FDOT PROJECT MANAGER: WIL	LIAM SLOUP, P.E.		GEC PROJECT MANAGER: MIKE VA	N DER HE





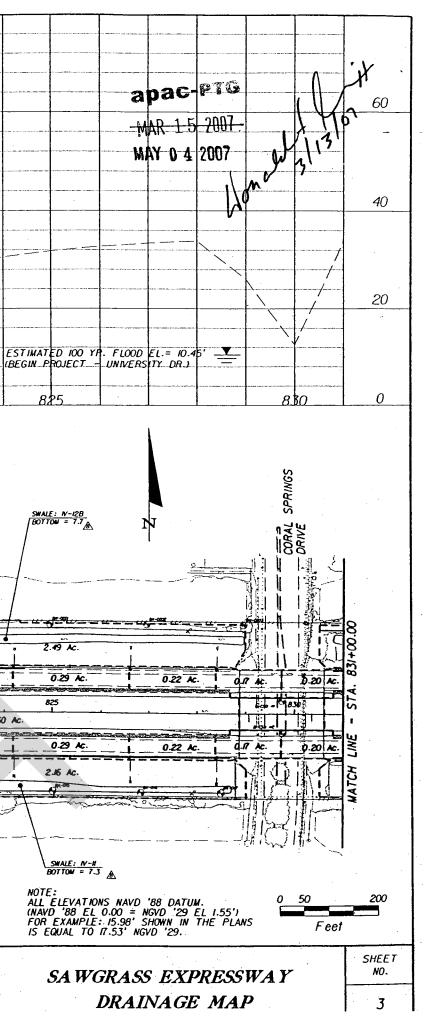


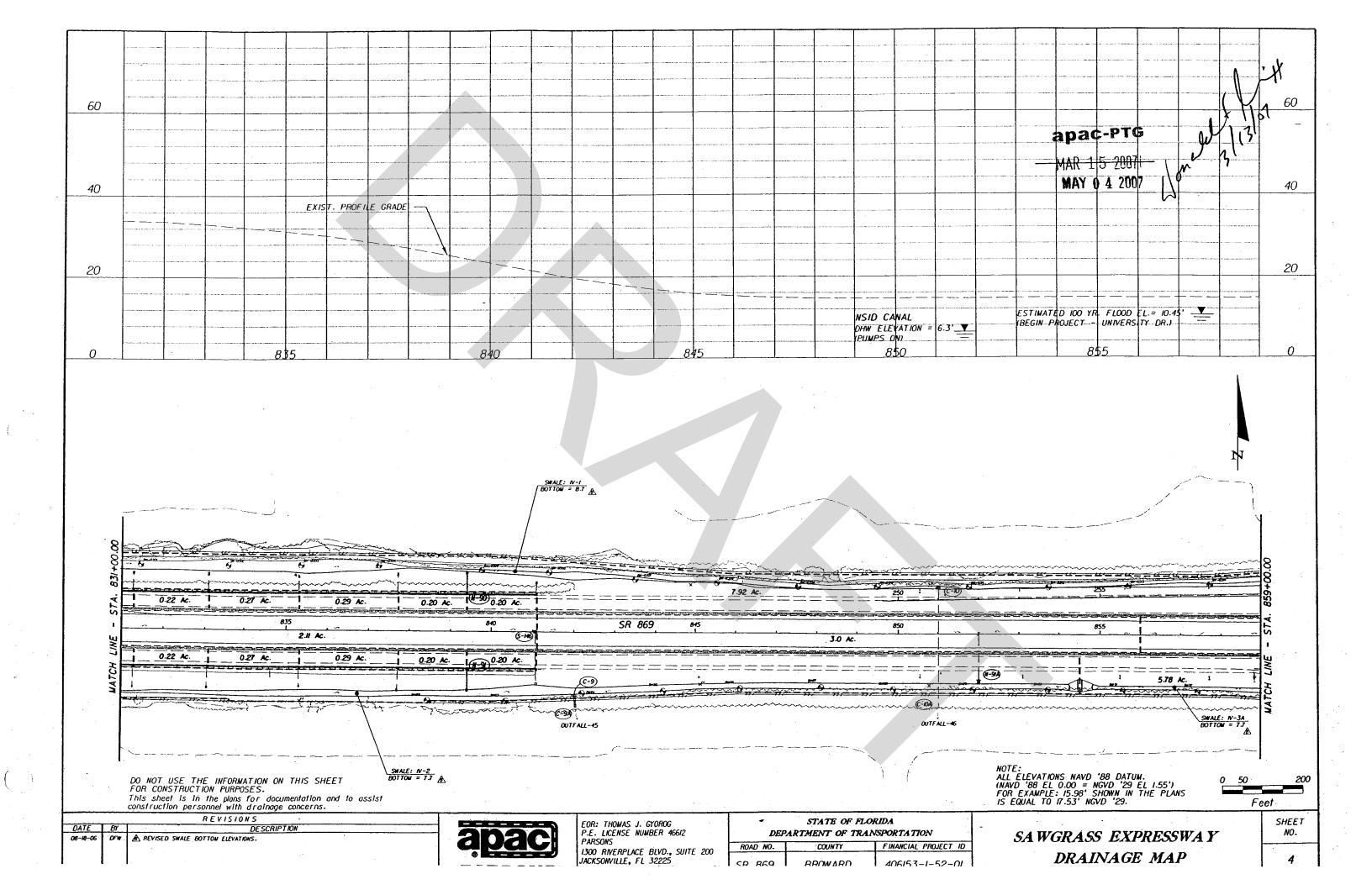


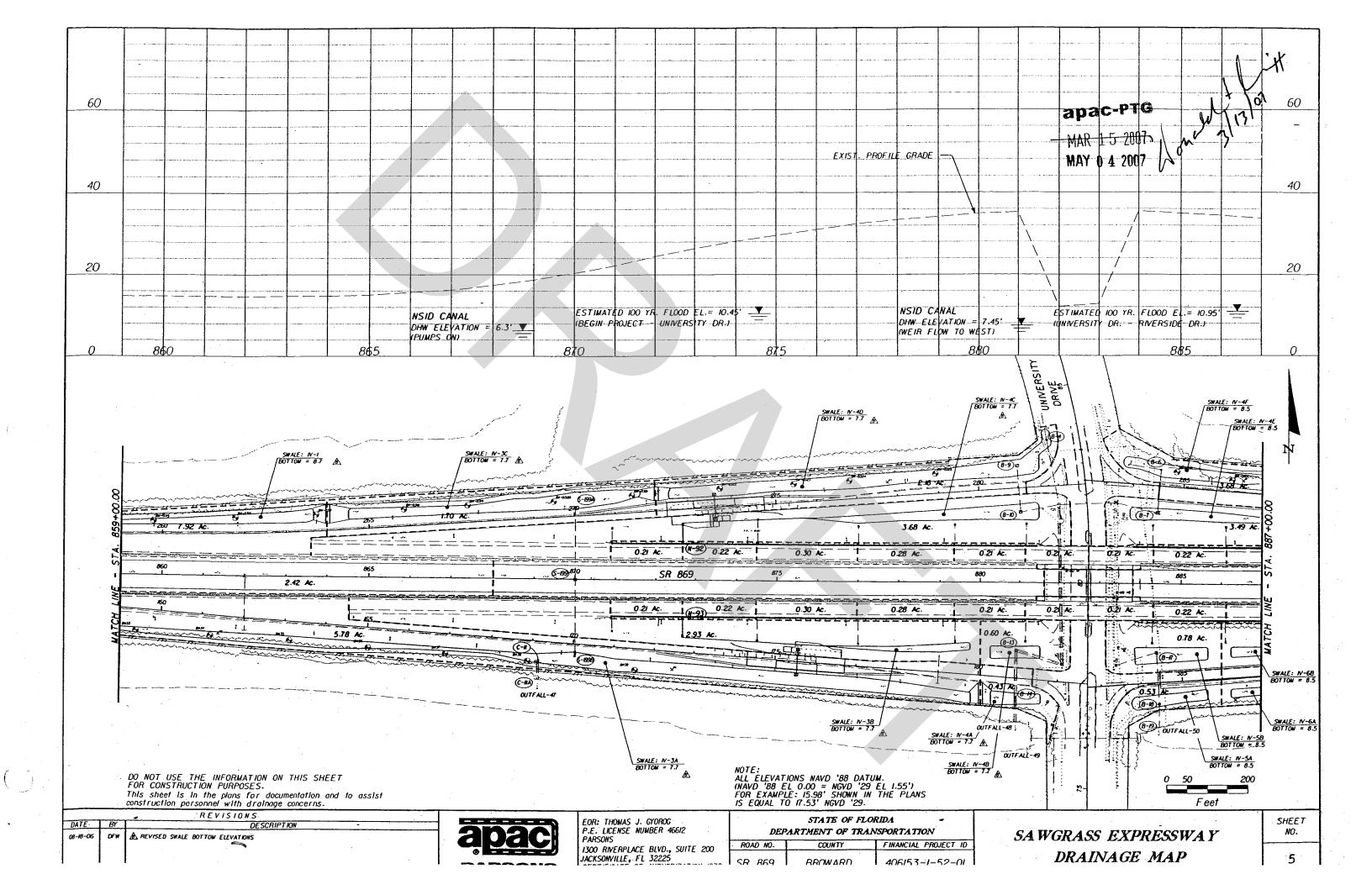
SR 869 RRAWARD 406153-1-52-01

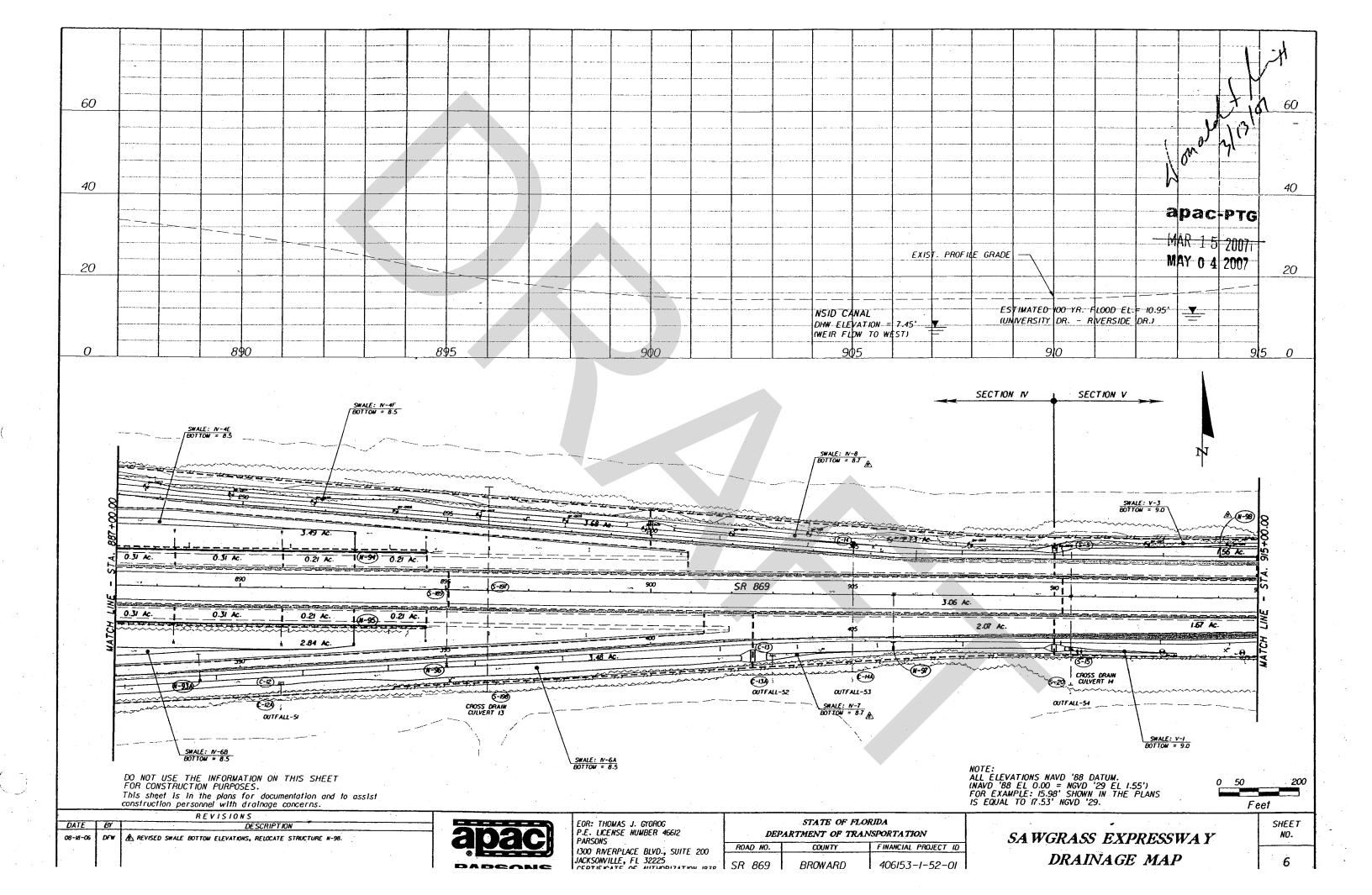
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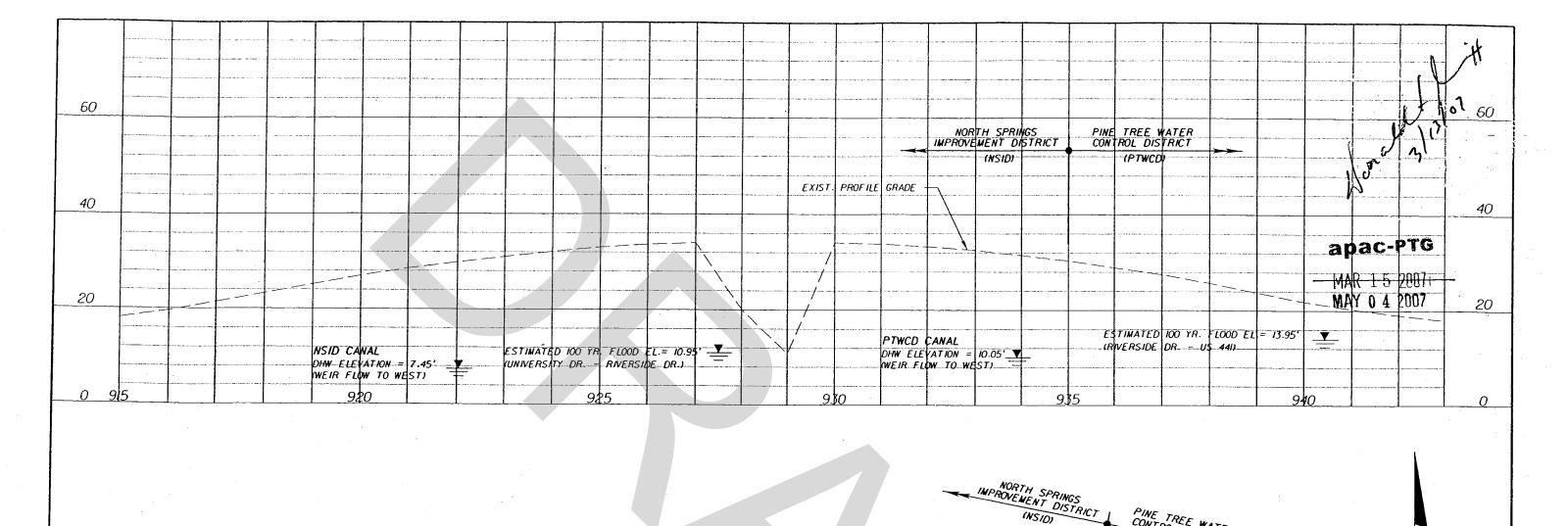
08-18-06







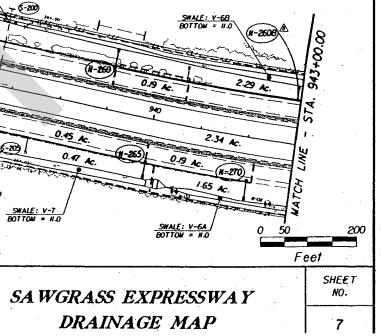


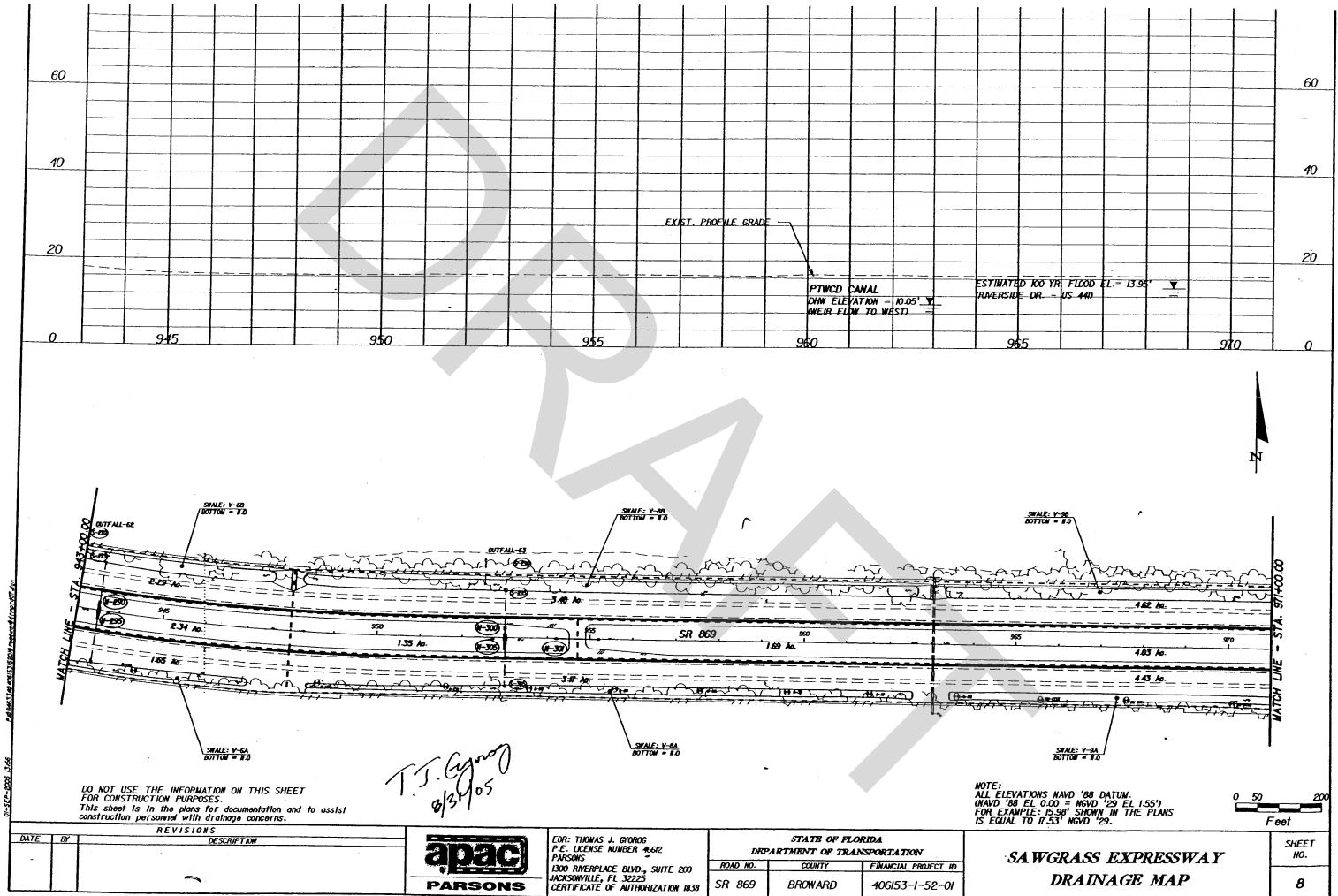


(NSID) States and SWALE: V-3 /BOTTOW = 9.0 SWALE: V-48 BOTTON = 9.0 (- 100 Å The states of -JD 7.56 AC (S-#6 Q-150 and the server 1.19 Ac. 0.20 AC. I 0.15 AC. 0.27 AC. 0.22 Ac. 1 0.38 Ac. 0.16 Ac. 0.17 Ac. 0.80 AC. 920 SR 869 (5-55). <u>6-00</u> ... 925 1.21 K. 0.85 AC 0.72 Ac. 20 し窗 (s-150) The second (1-99) 0.31 Ac. 0.43 Ac. <u>(1-23</u> (1-53) 0.42 Ac. N=55 0.20 AC. 0.19 Ac. 0.22 A 1.30 Ac. 0.33 Ac. 1.67 AC. 0.88 Ac **1-20** 0.48 Ac. -0.87-AC. SWALE: V-2 . **.** . 6.63 -20) OUTFAU-558-49 SWALE: V-I BOTTON = 9.0 OUTFALL-56 ALI-30 -10 -10 -10 -10 -10 -10 C-TOOUTFALL-58 JI D Cop Antonia 6-183 OUTFALL-60 OUTFALL-59 NOTE: ALL ELEVATIONS NAVD '88 DATUM. (NAVD '88 EL 0.00 = NGVD '29 EL 1.55') FOR EXAMPLE: 15.98' SHOWN IN THE PLANS IS EQUAL TO 17.53' NGVD '29. SWALE: V-5 BOTTOW = 9.0 DO NOT USE THE INFORMATION ON THIS SHEET FOR CONSTRUCTION PURPOSES. This sheet is in the plans for documentation and to assist construction personnel with drainage concerns. REVISIONS DATE BY EOR: THOMAS J. GYOROG STATE OF FLORIDA DESCRIPTION P.E. LICENSE NUMBER 46612 08-18-06 DFW A RELOCATE STRUCTURES N-98 AND N-100 ISEE SHEET 6, N-98 NEW LOCATIONI, ADDED STRUCTURE N-2608. DEPARTMENT OF TRANSPORTATION PARSONS ROAD NO. COUNTY FINANCIAL PROJECT ID 1300 RNERPLACE BLVD., SUITE 200. JACKSONVILLE, FL 32225 SR 869 BROWARD 406153-1-52-01

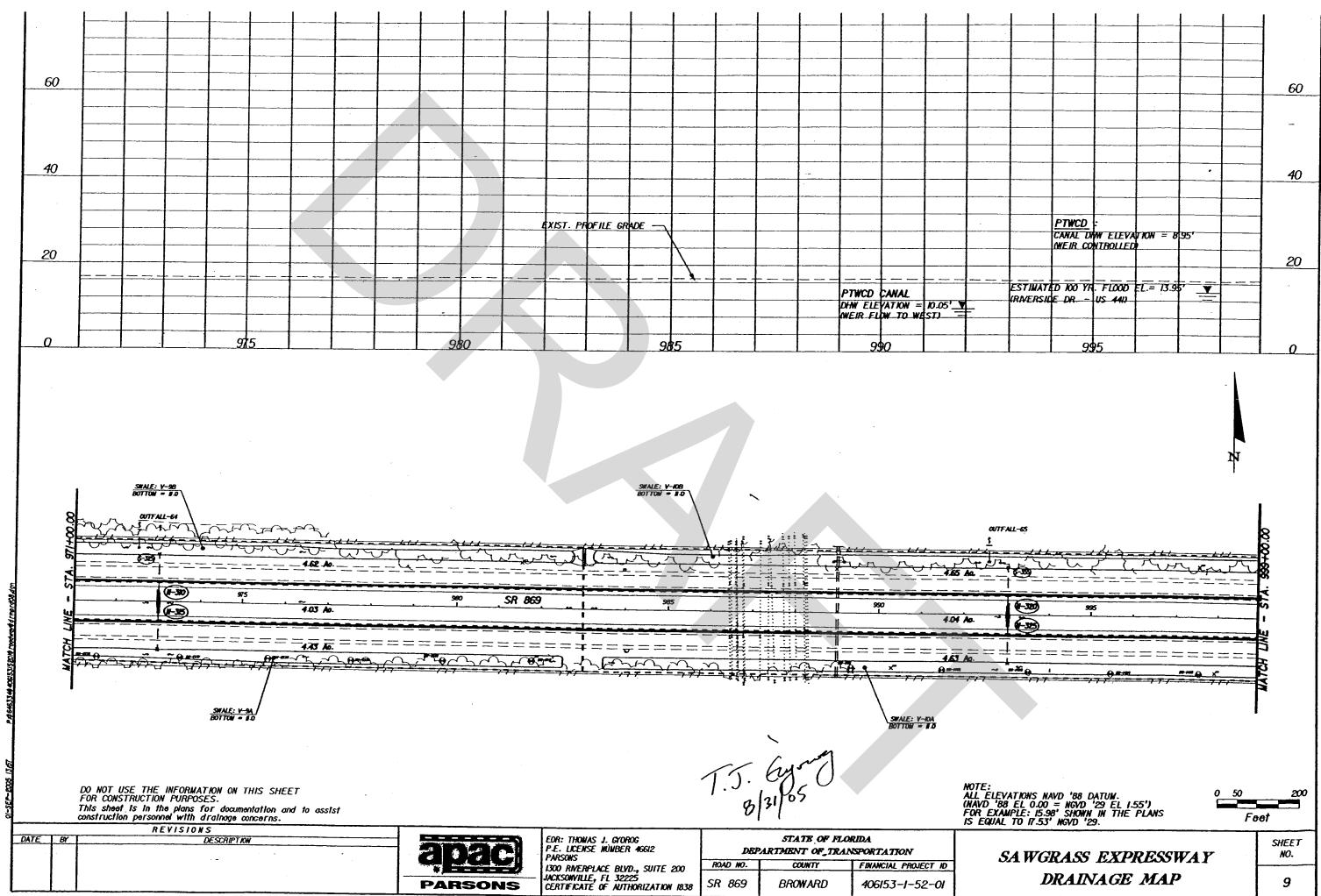
PINE TREE WATER CONTROL DISTRICT (PTWCD)

- EXIST. PIPE (TO REMAIN)



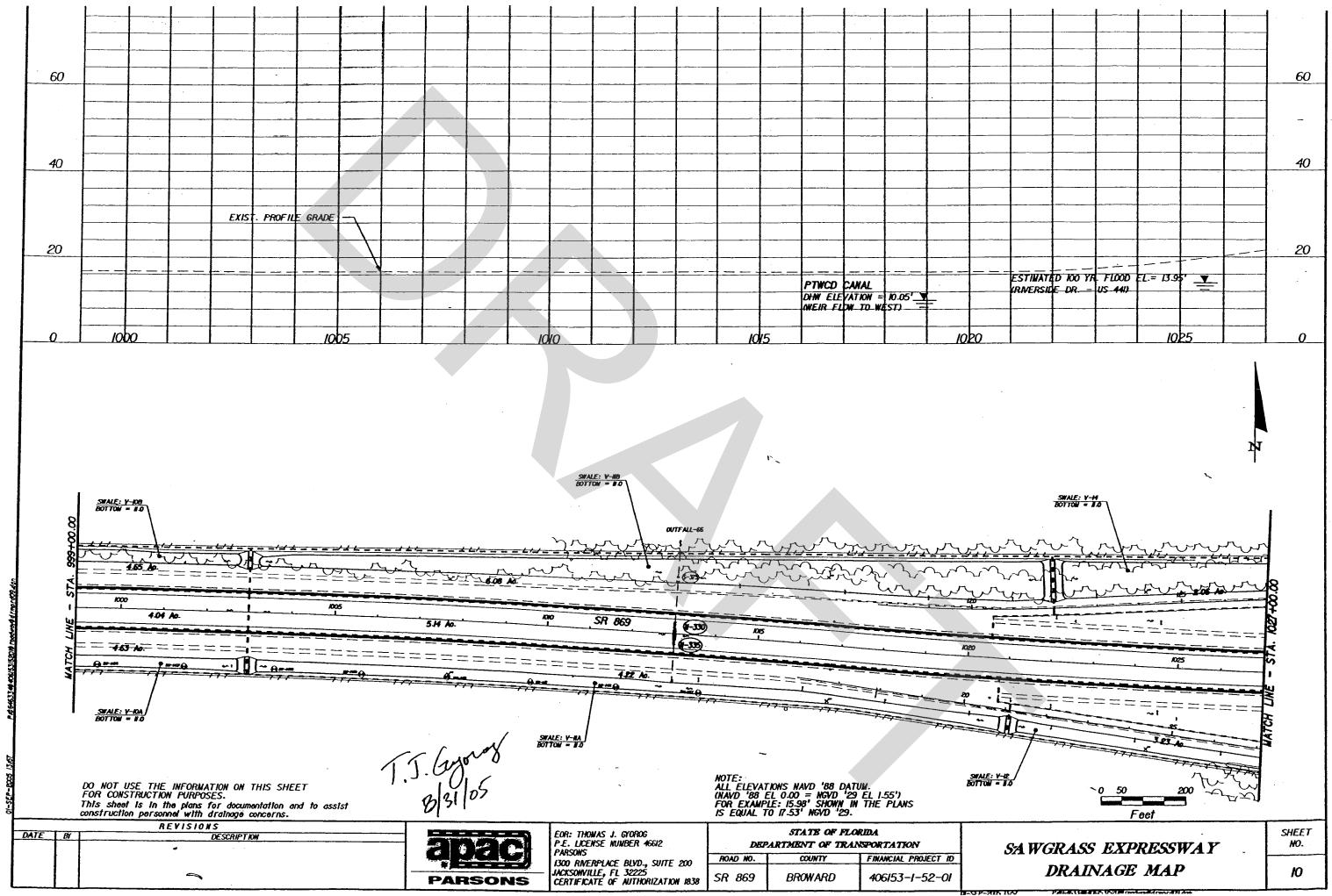


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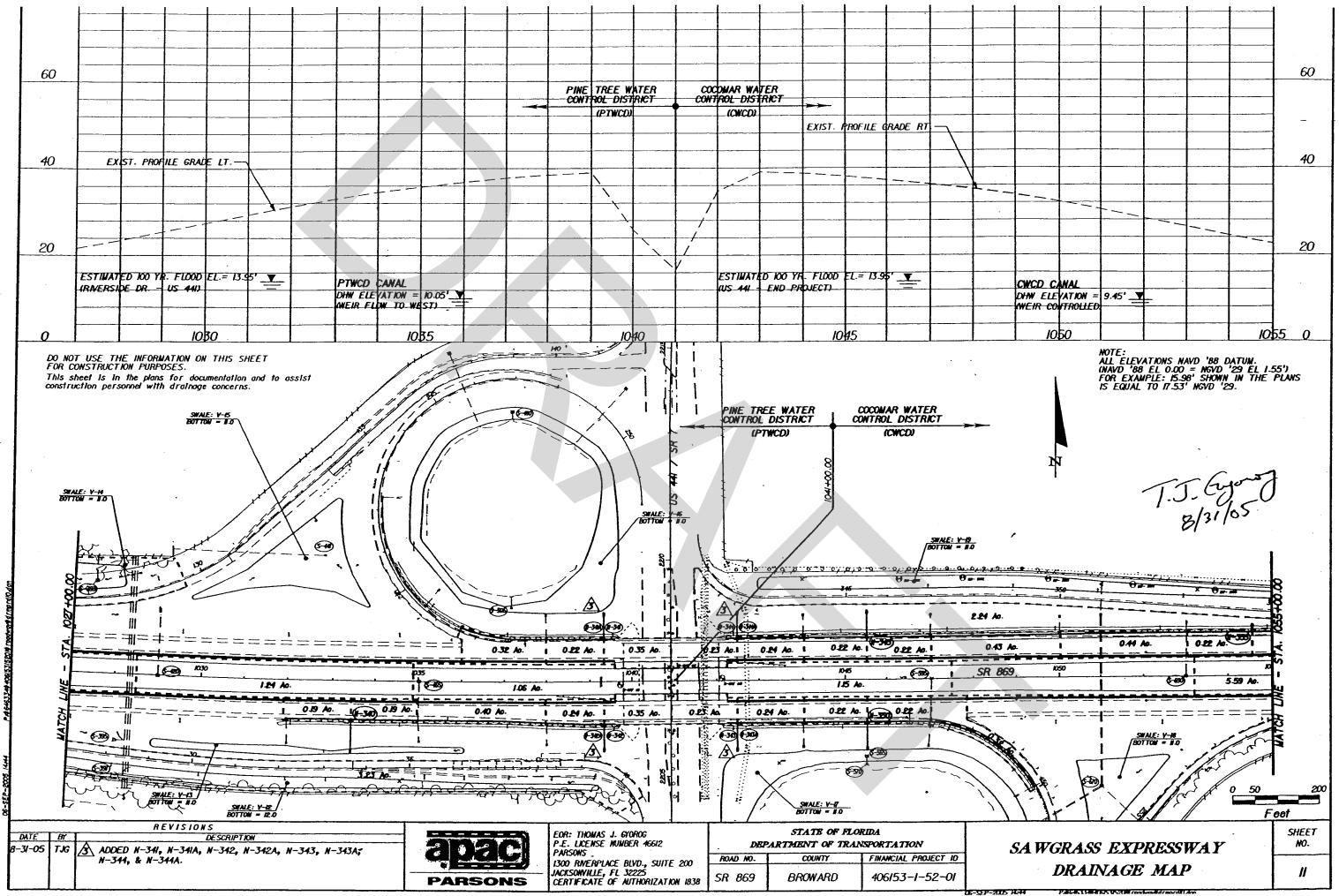
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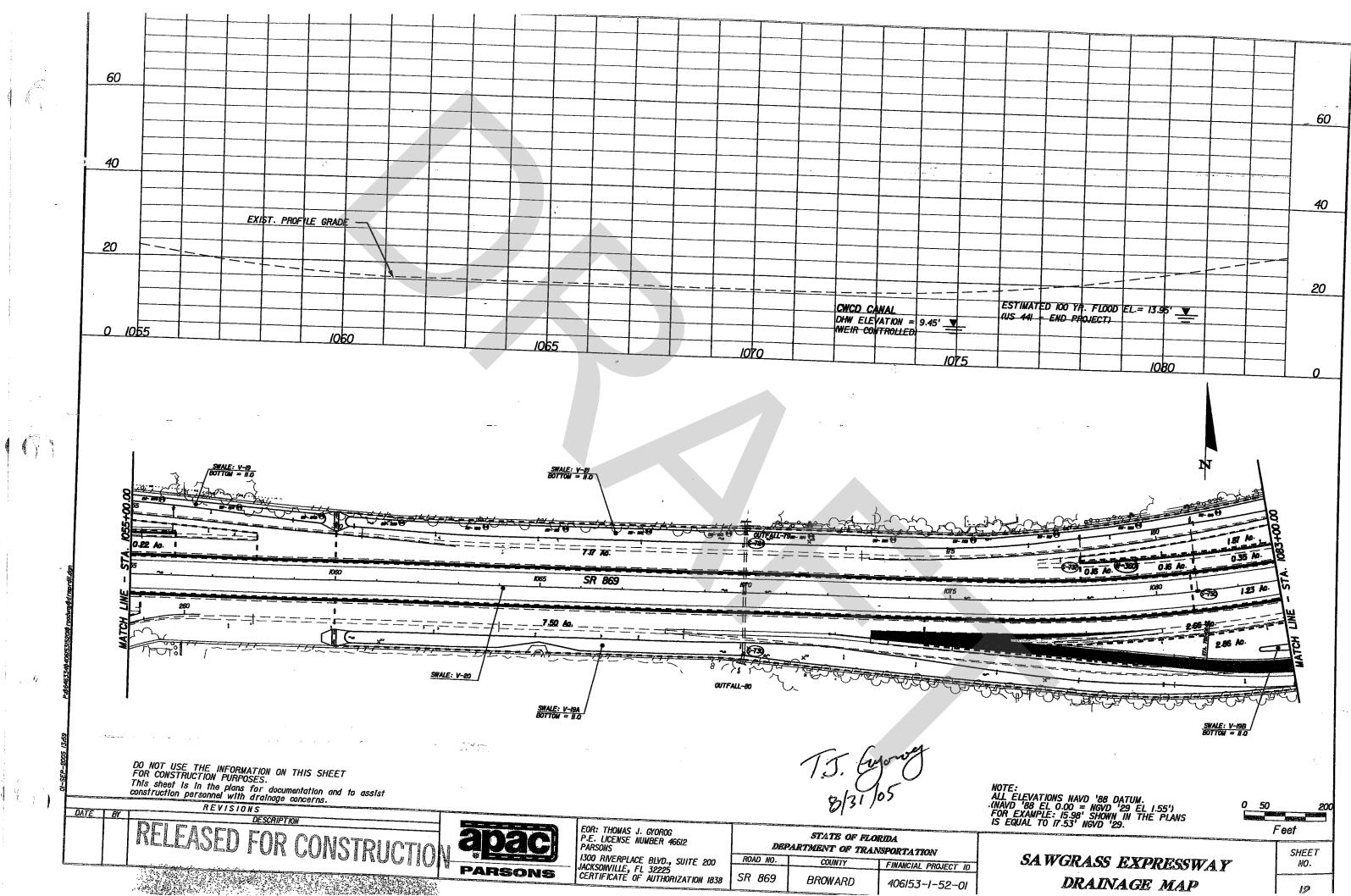
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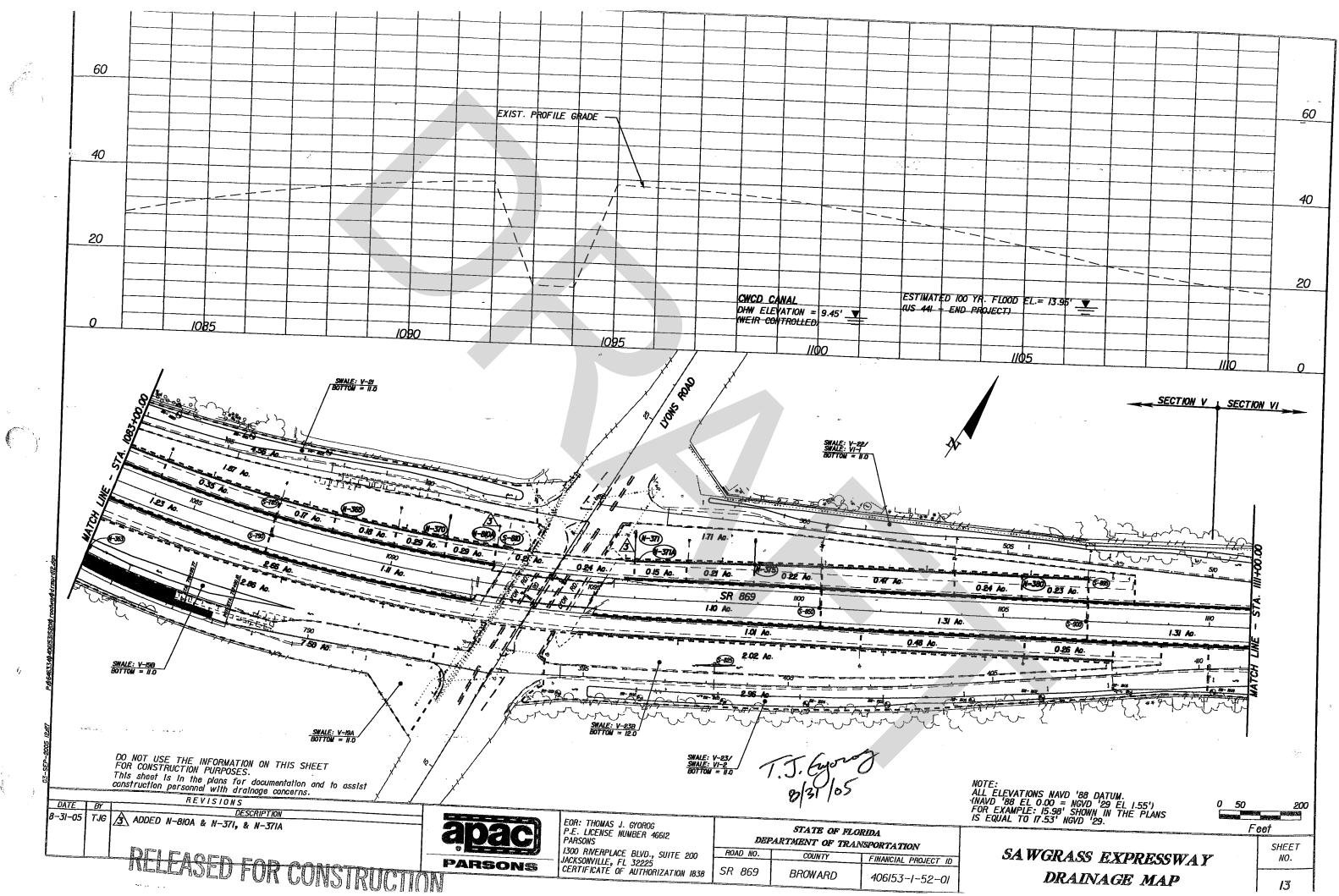
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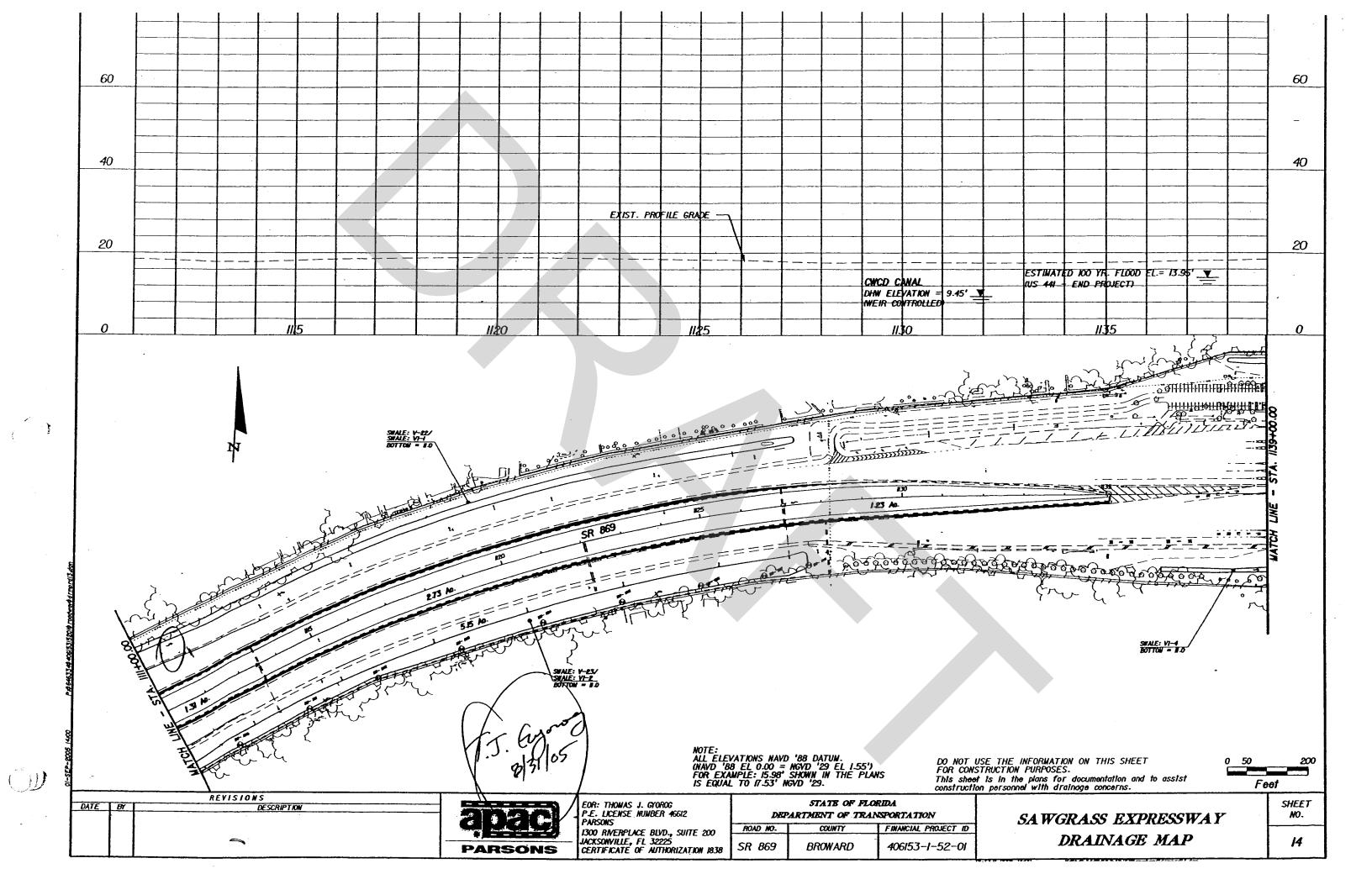
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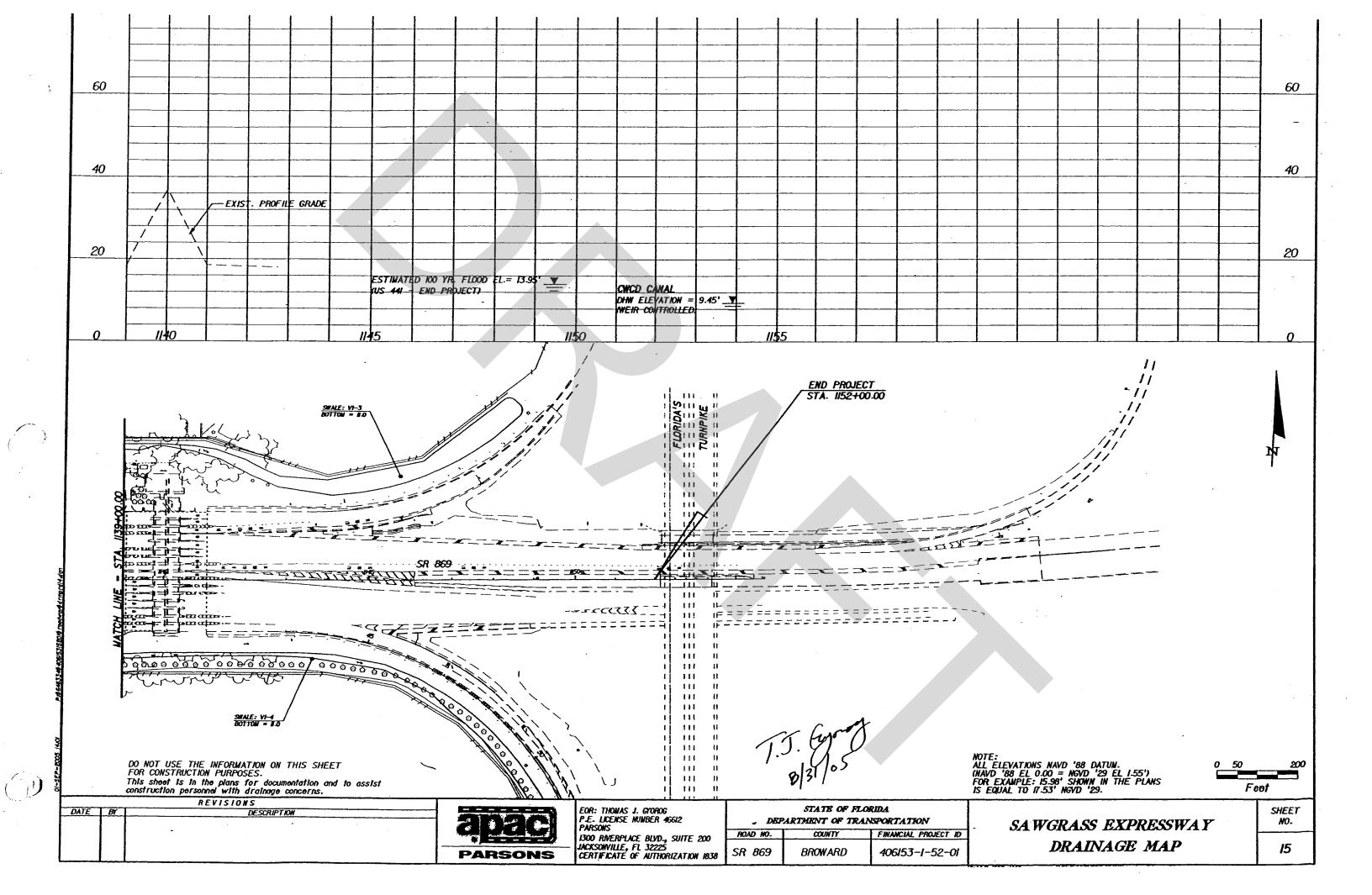
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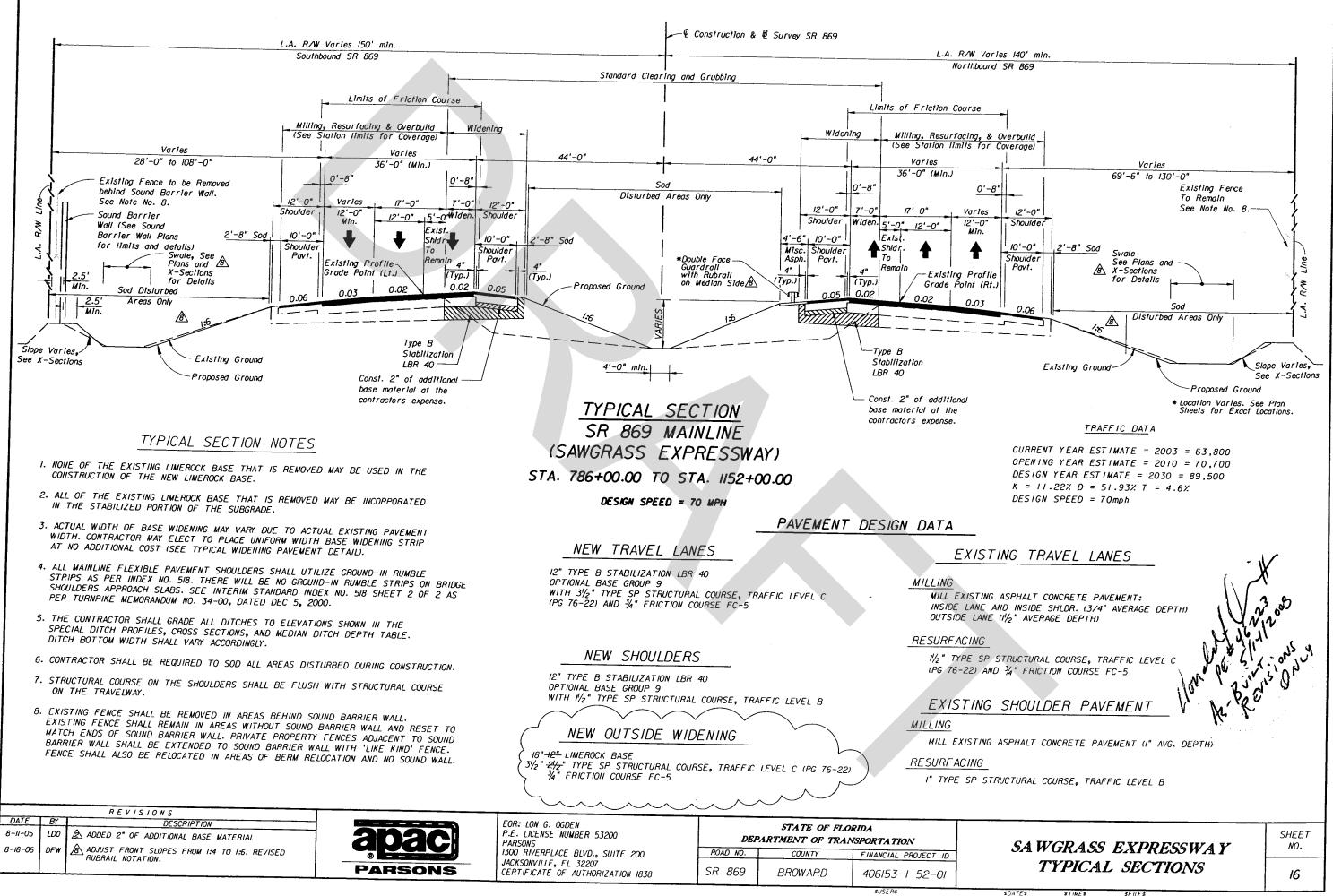


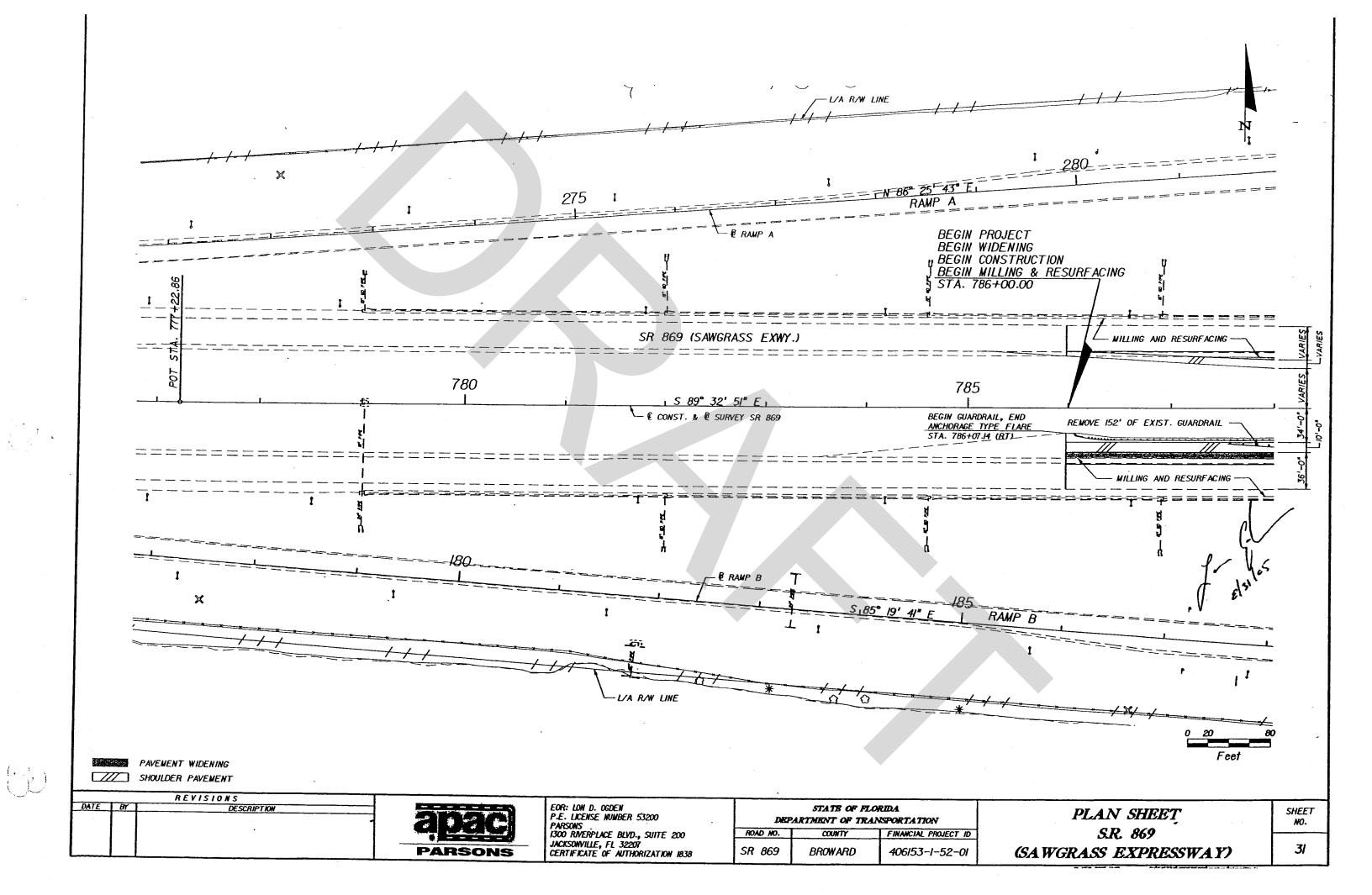
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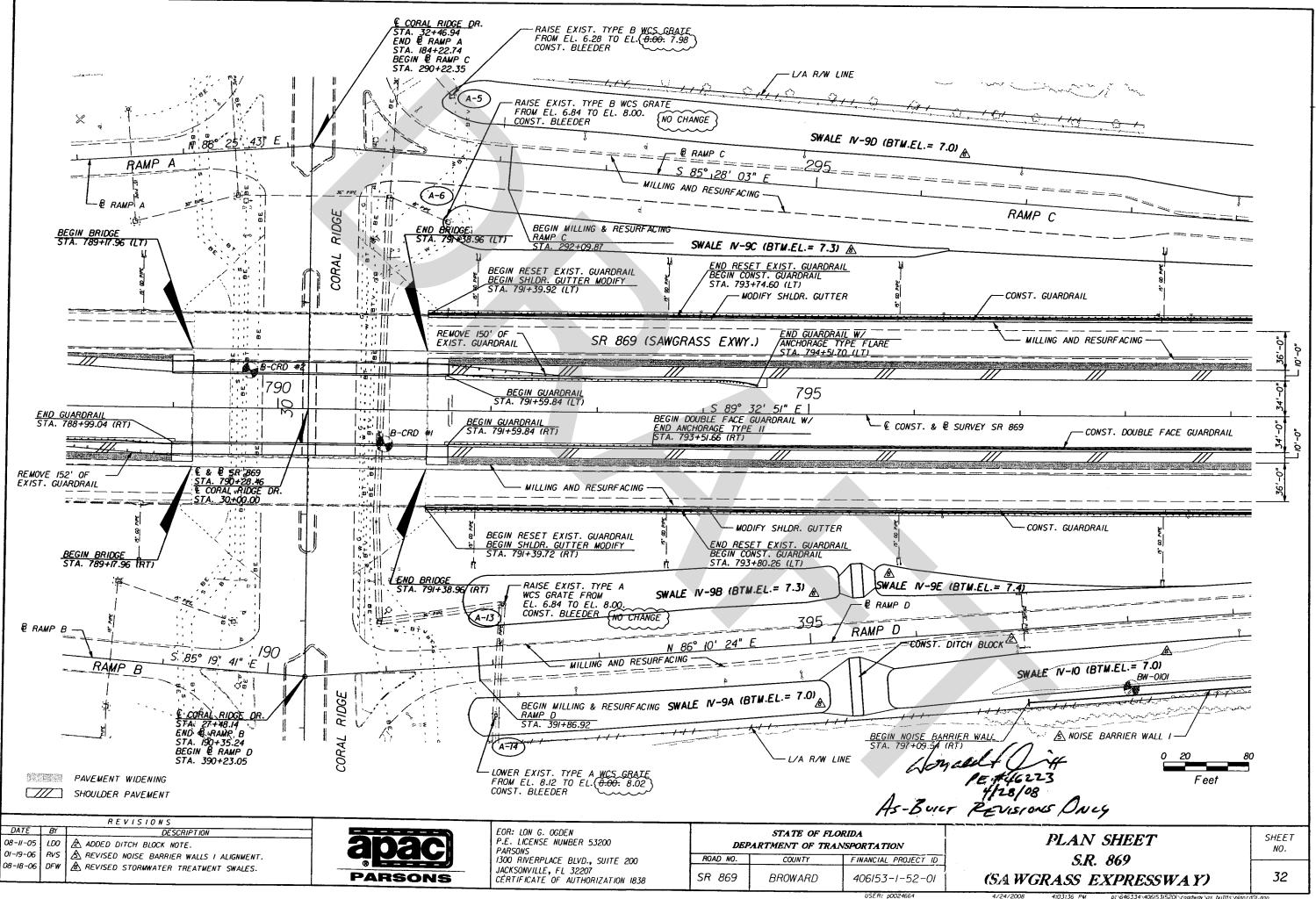


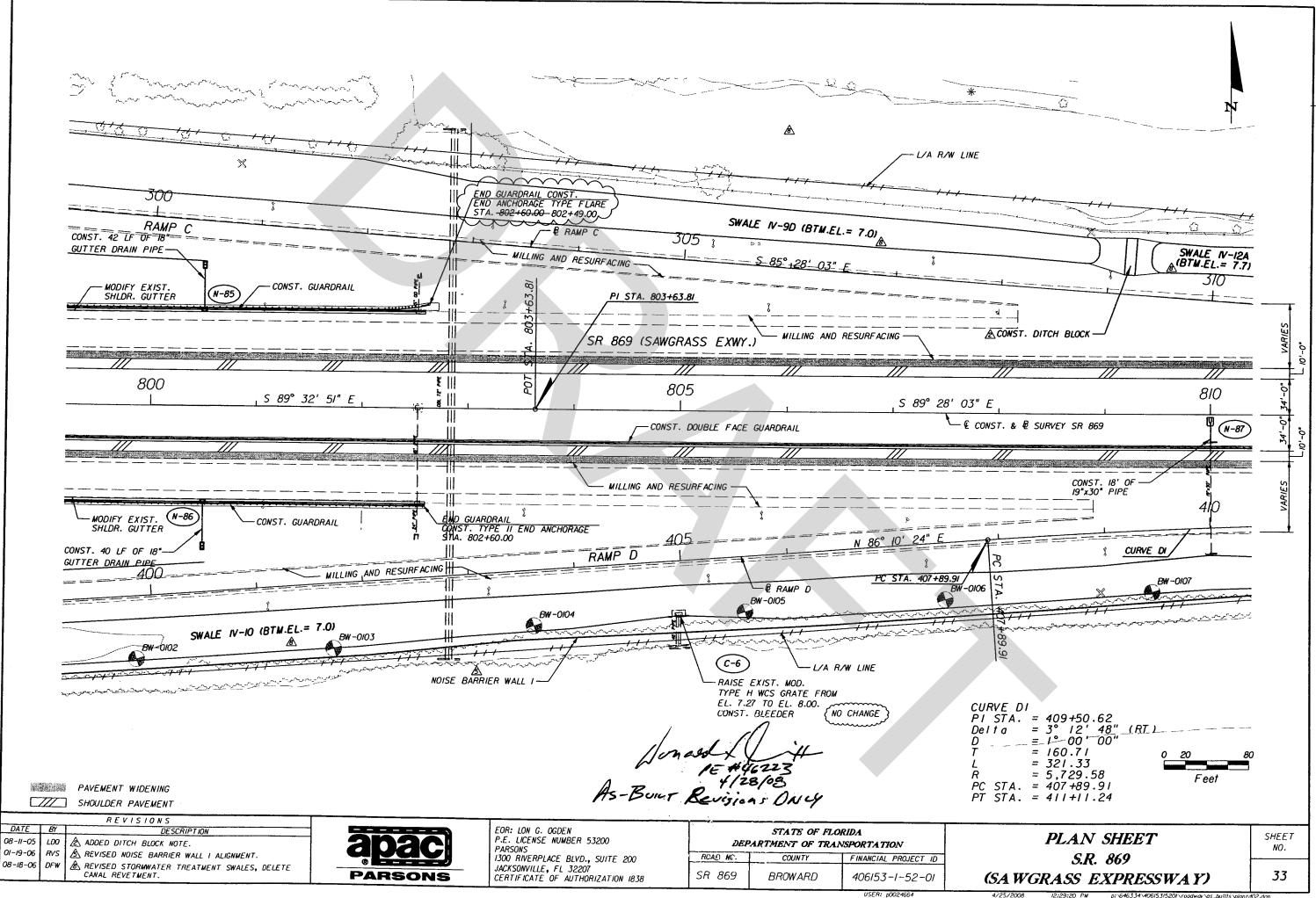


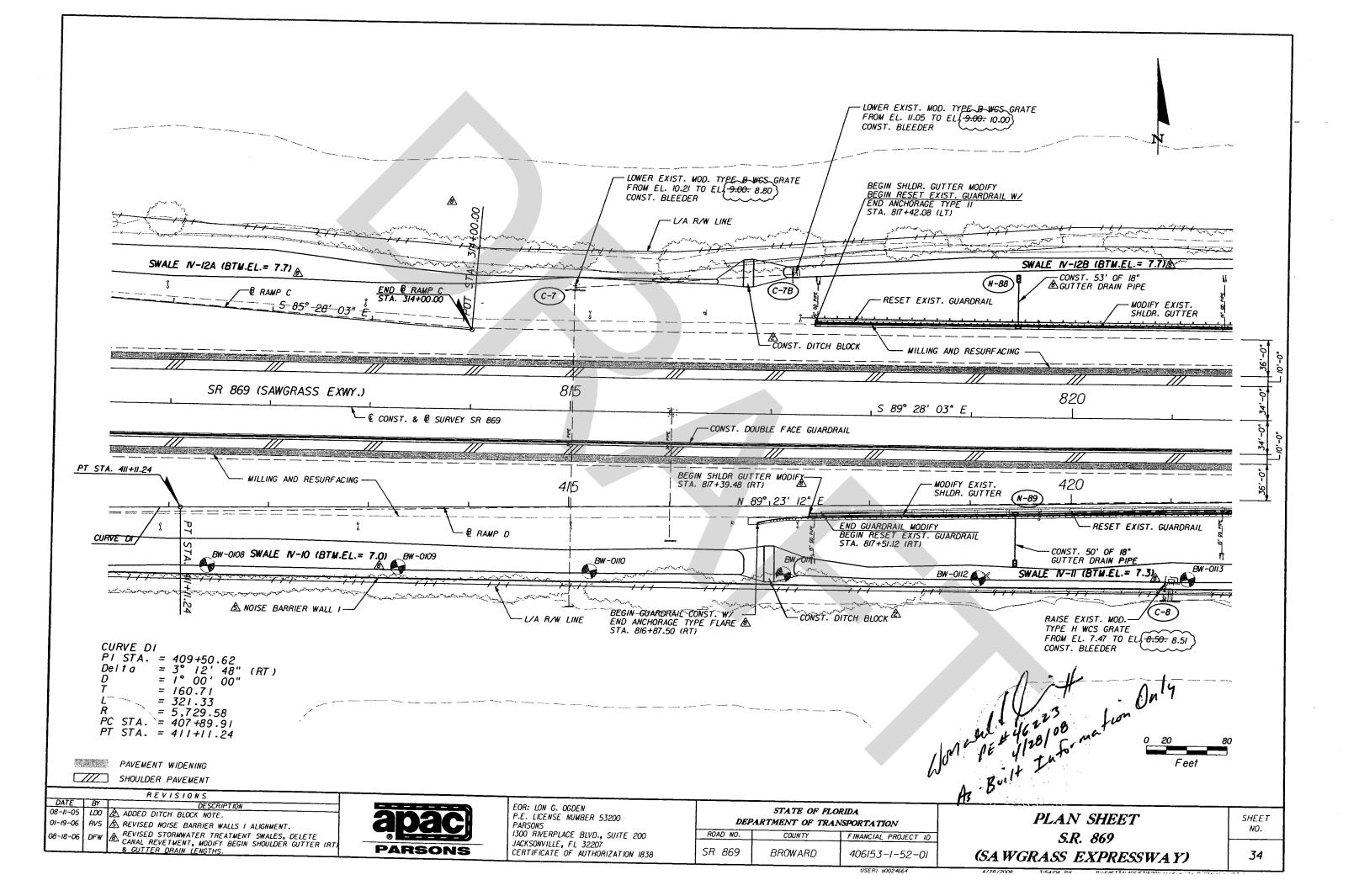


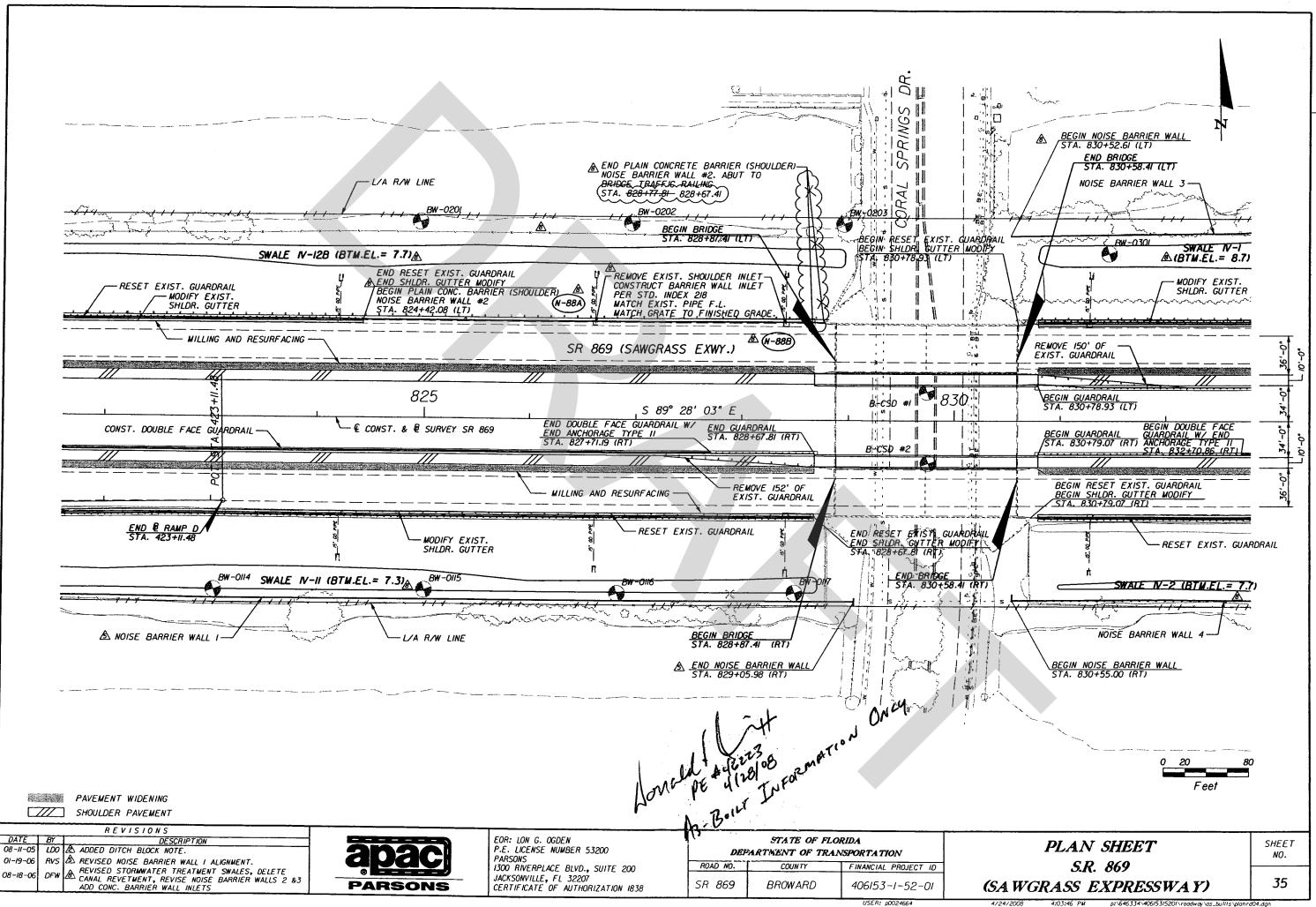


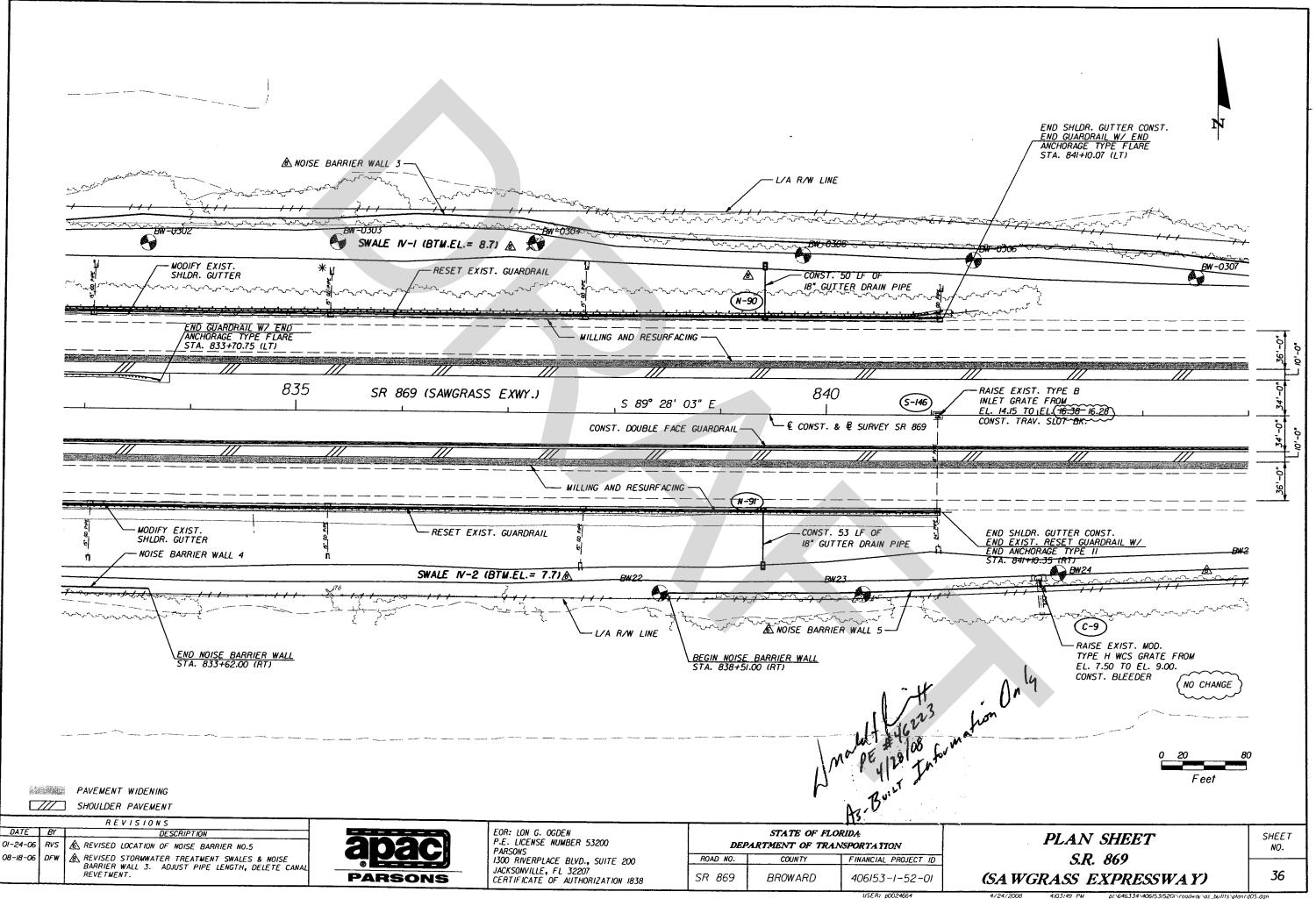


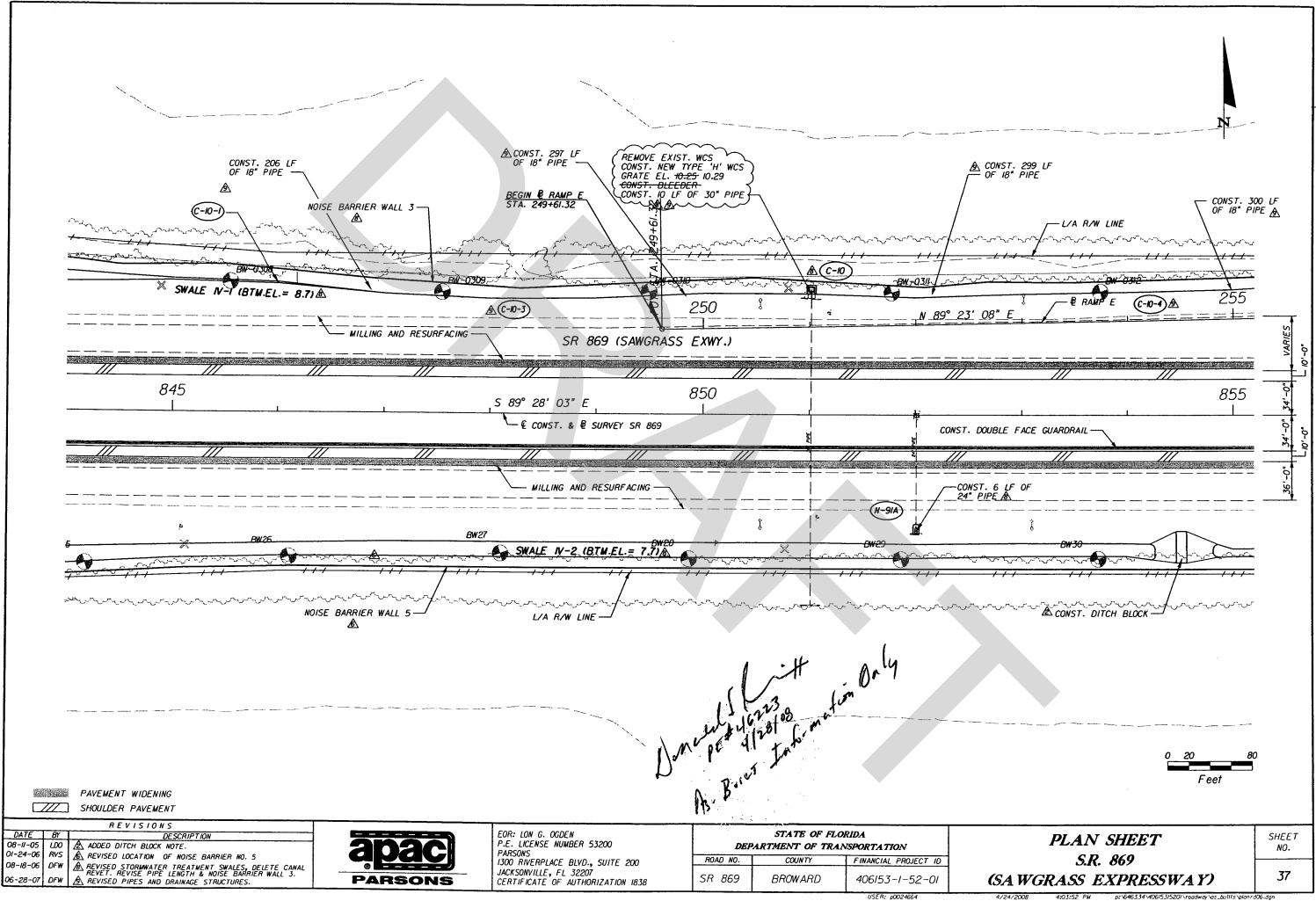


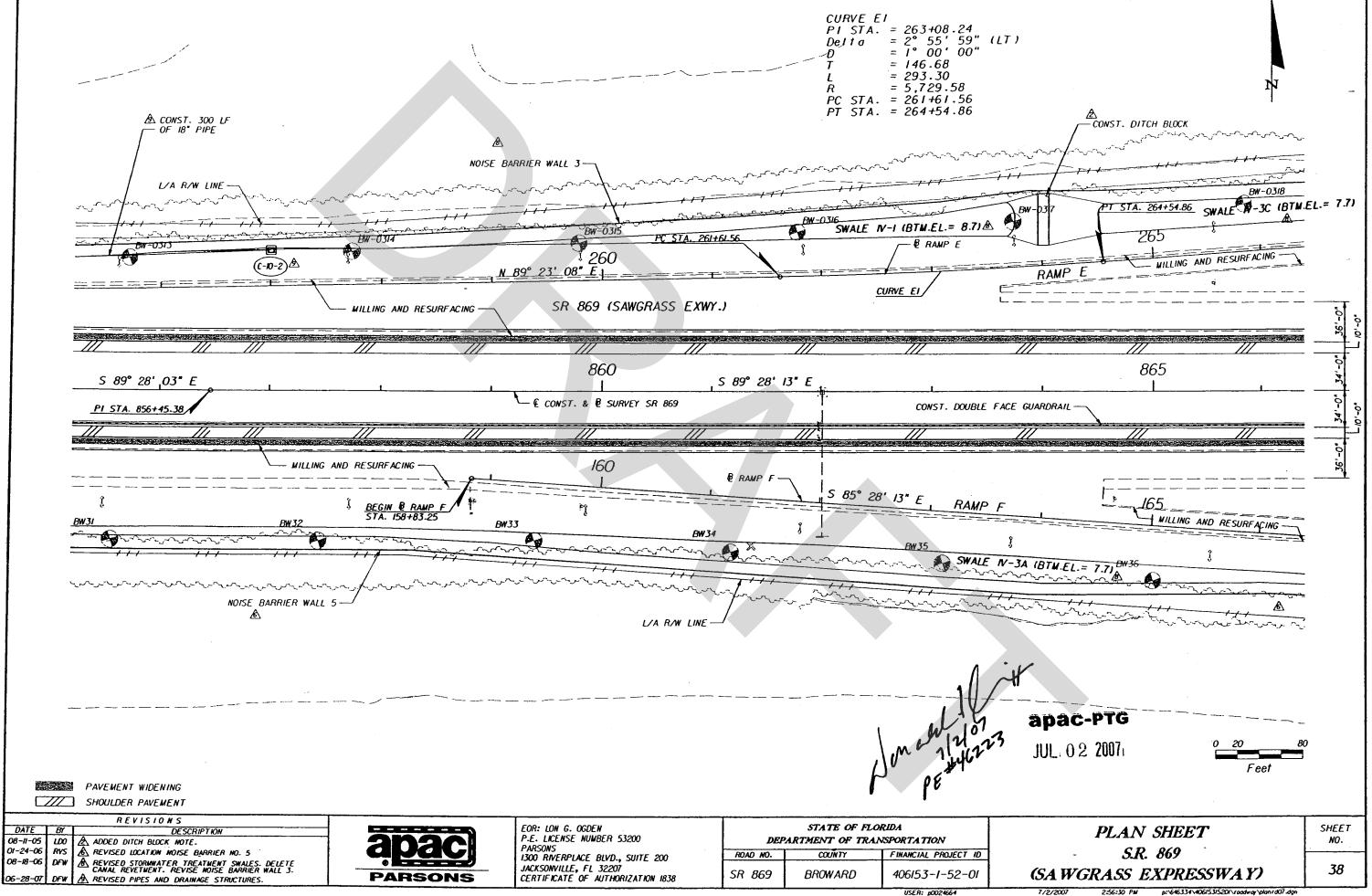








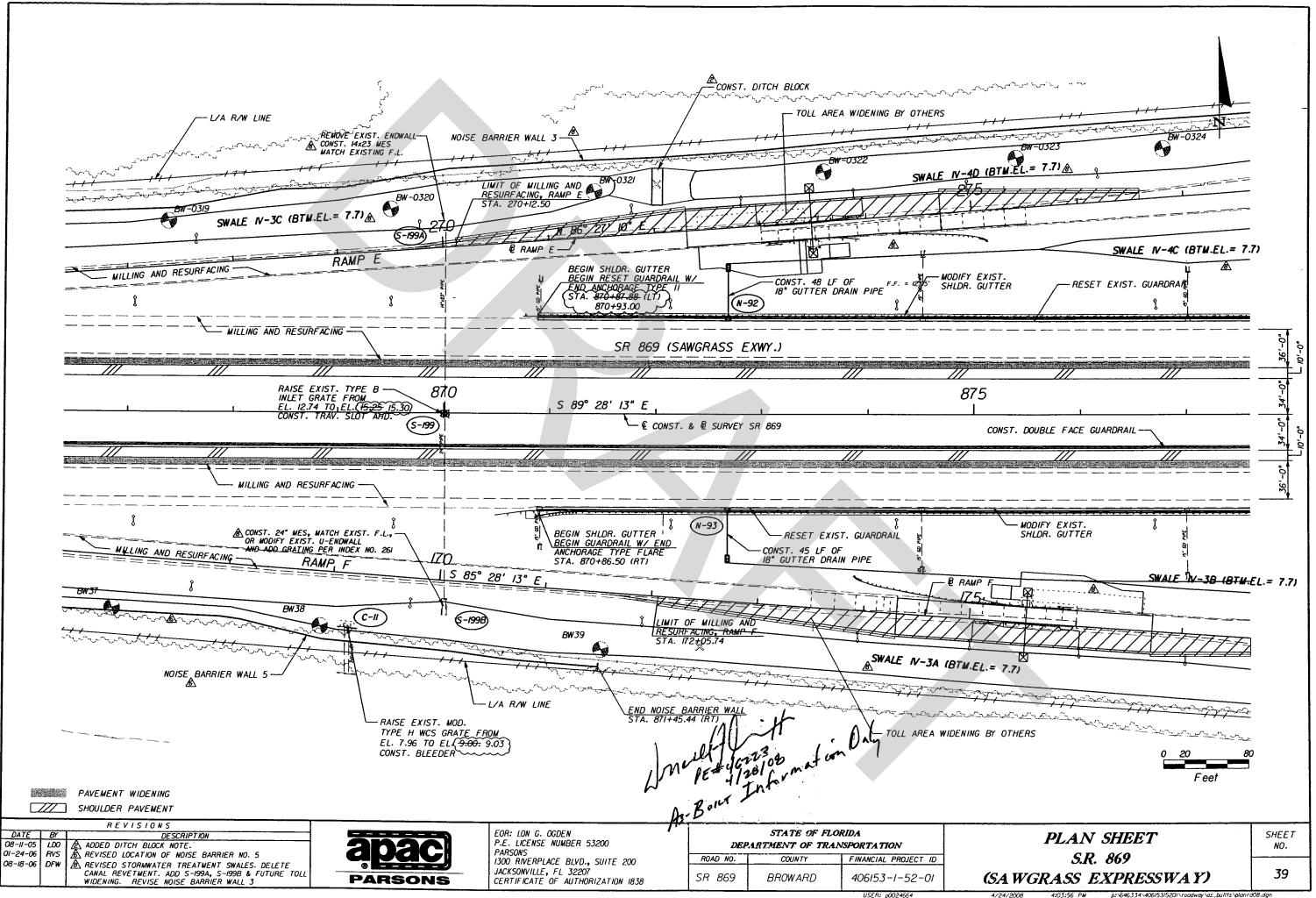


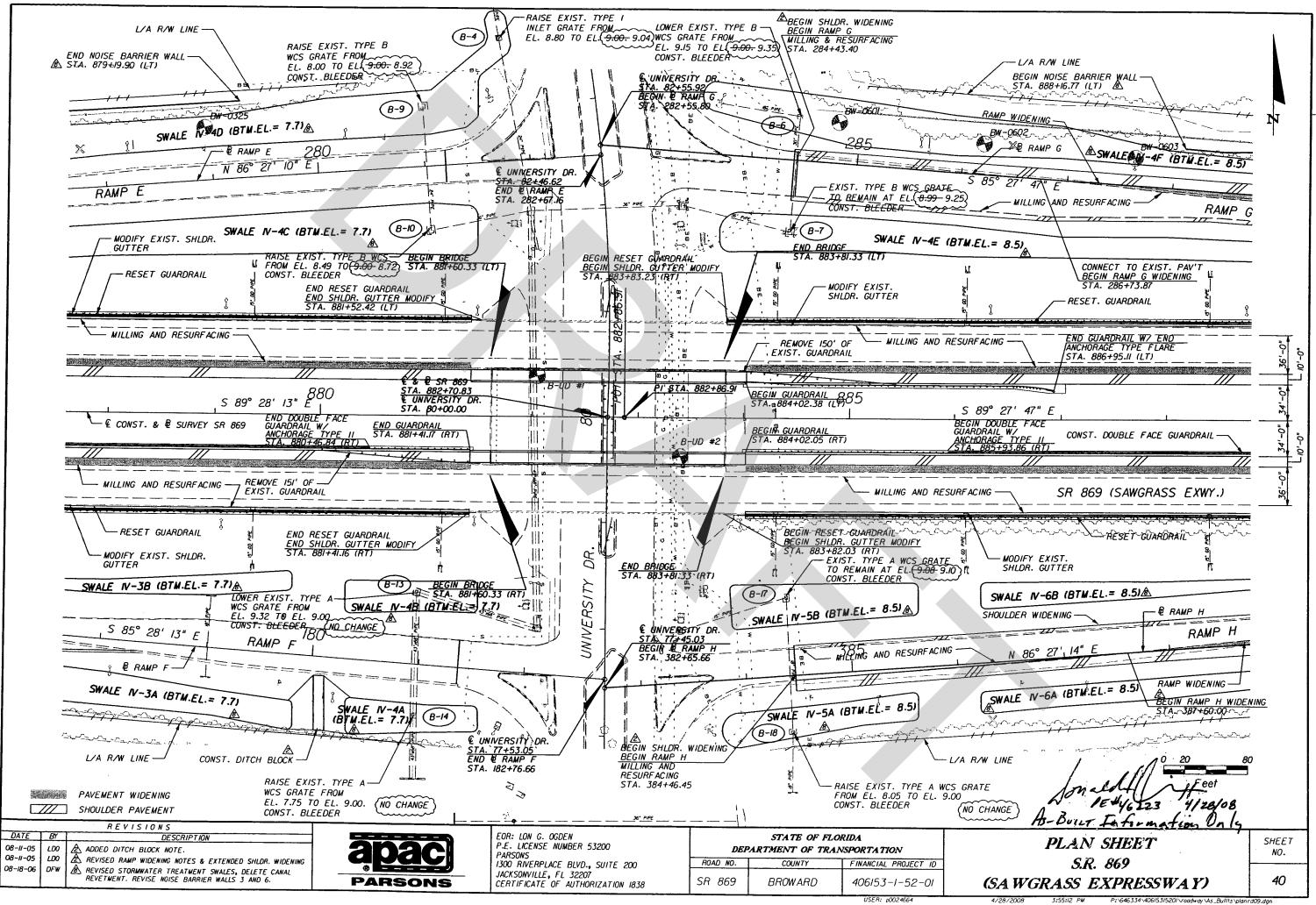


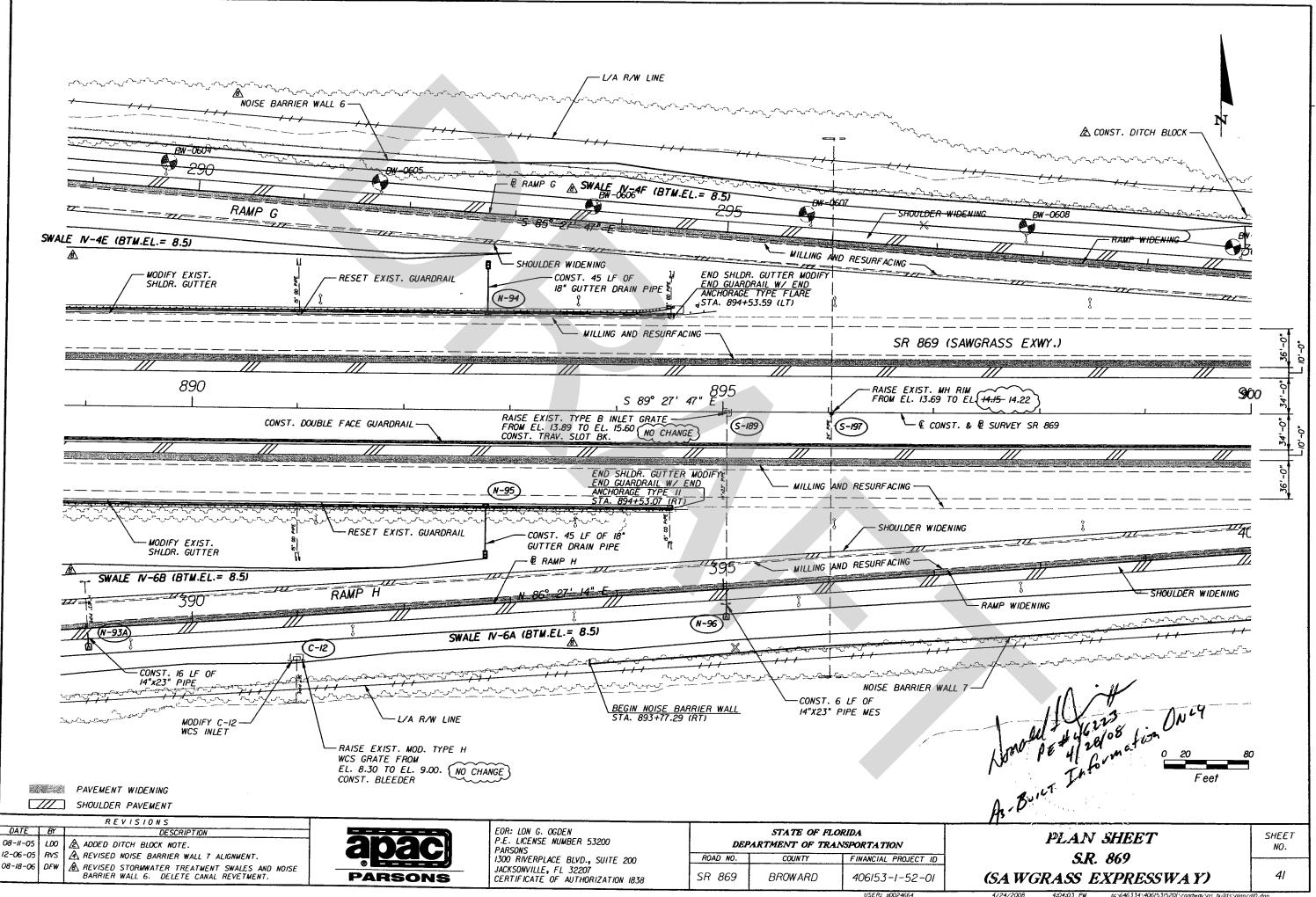
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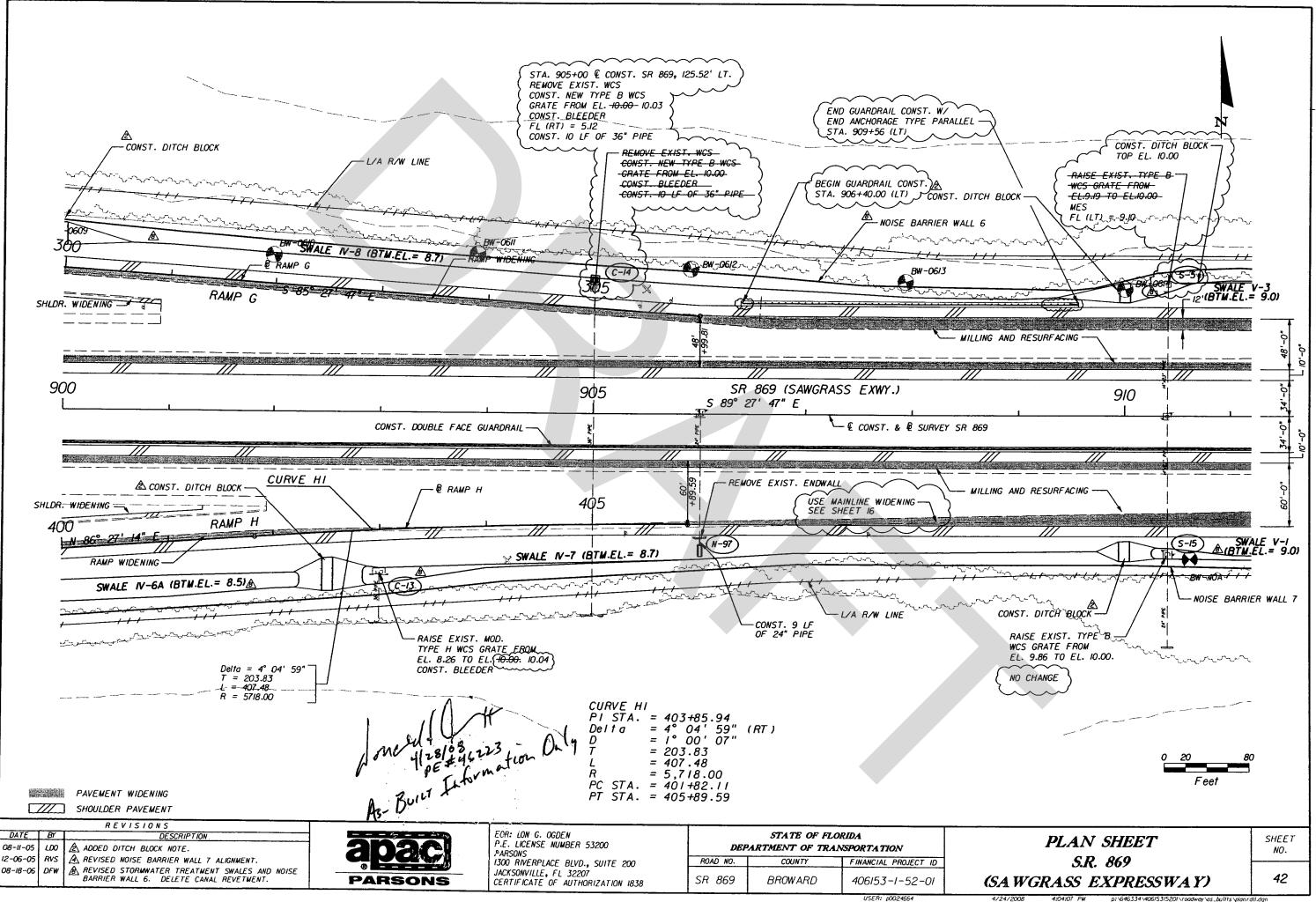
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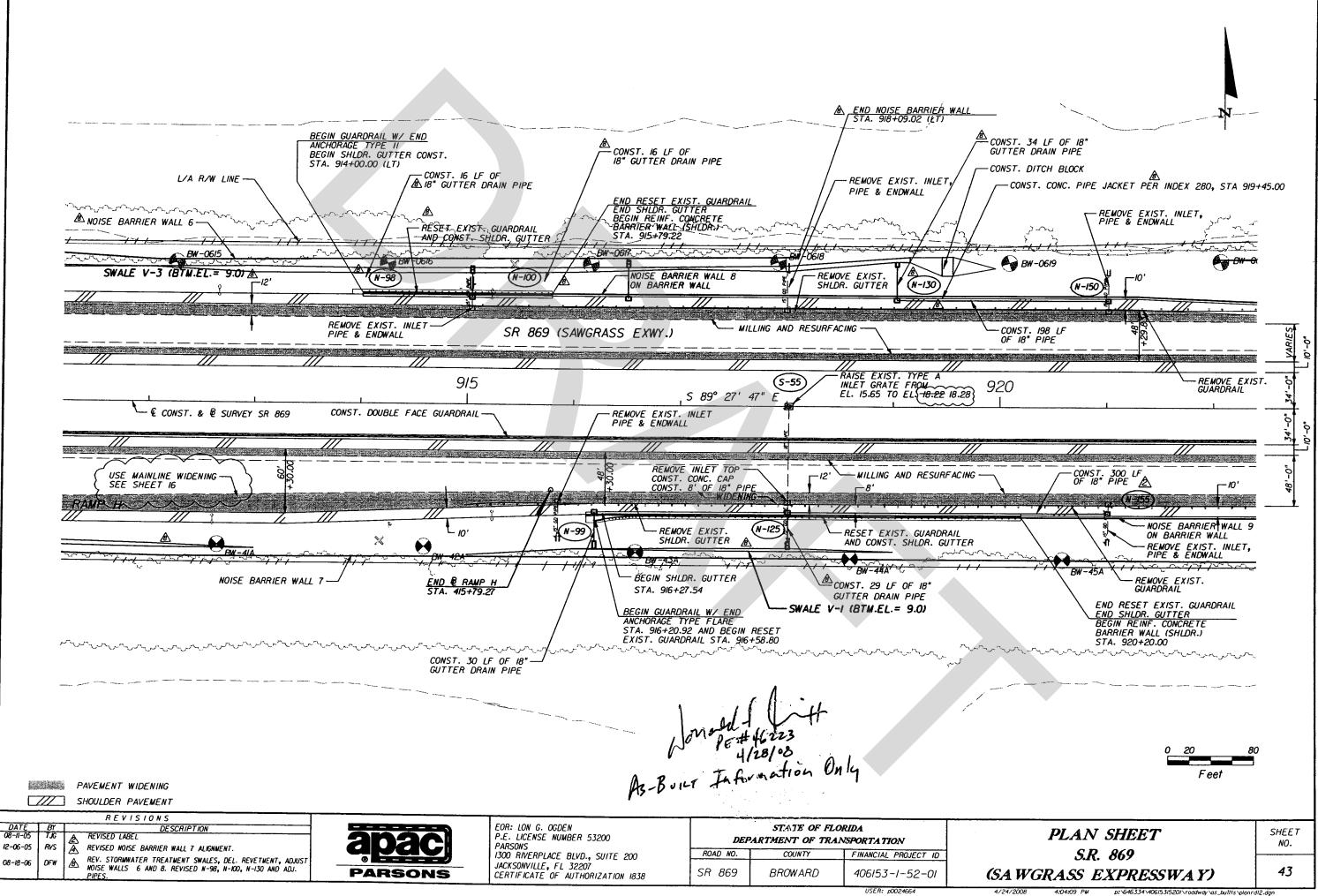
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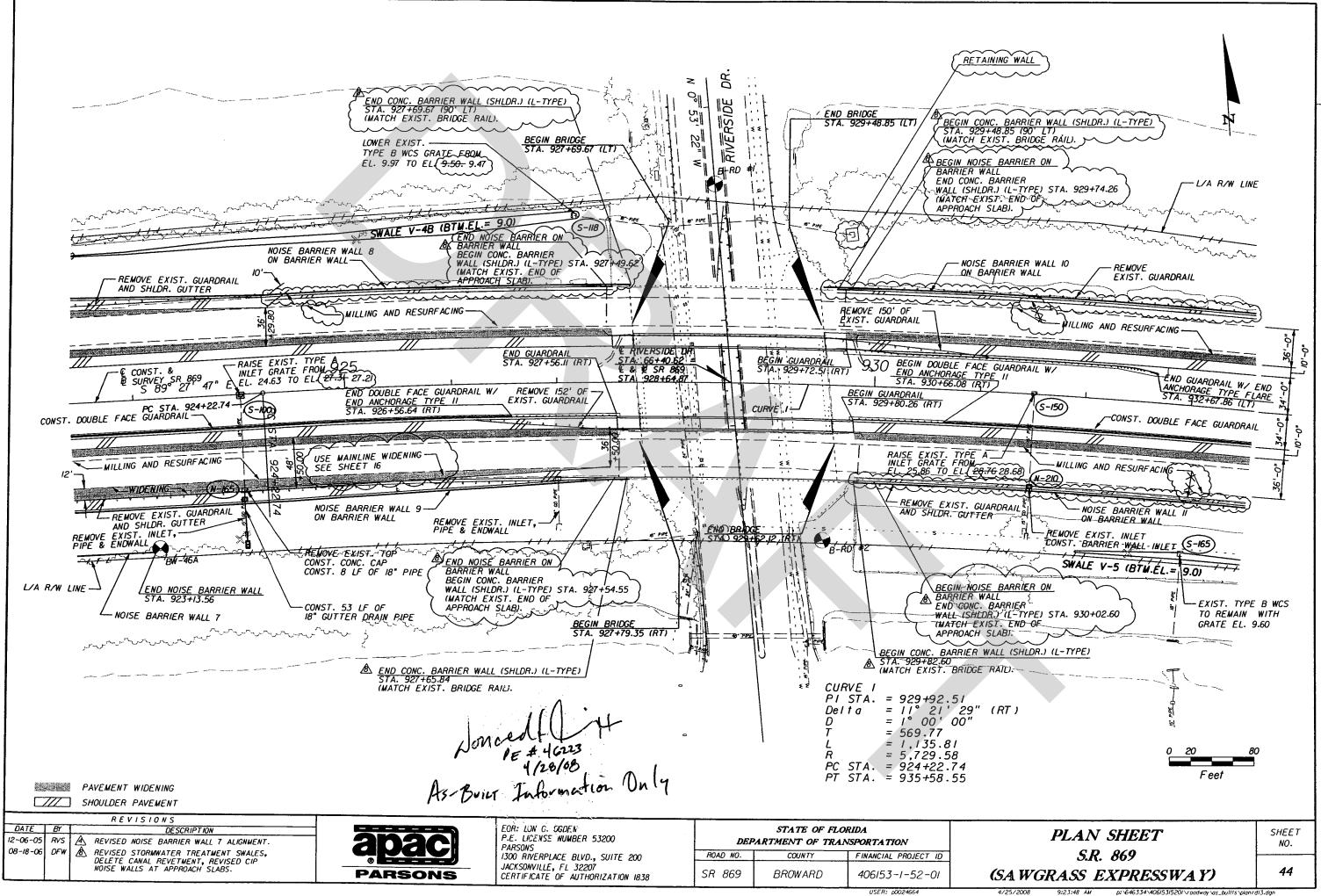


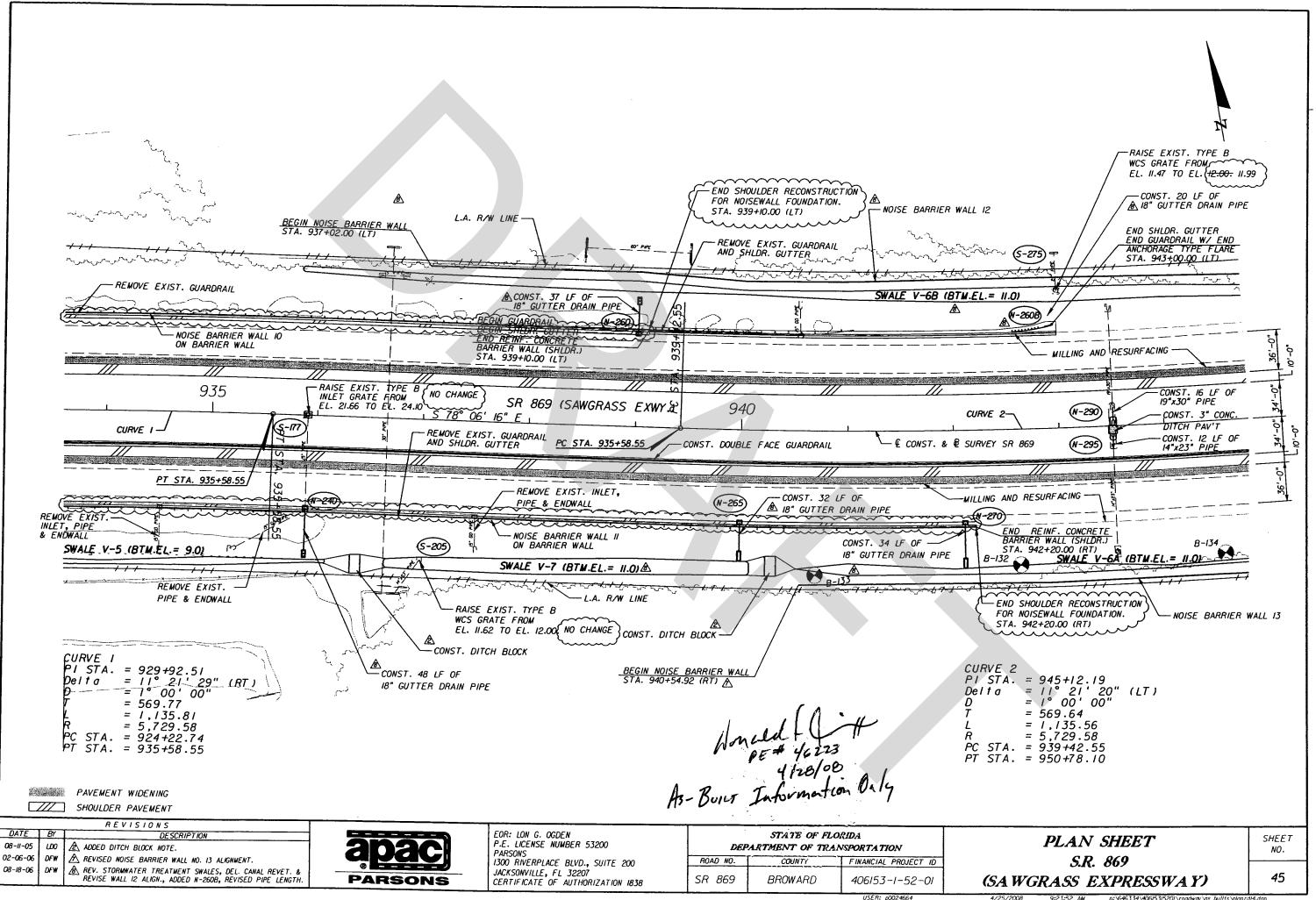


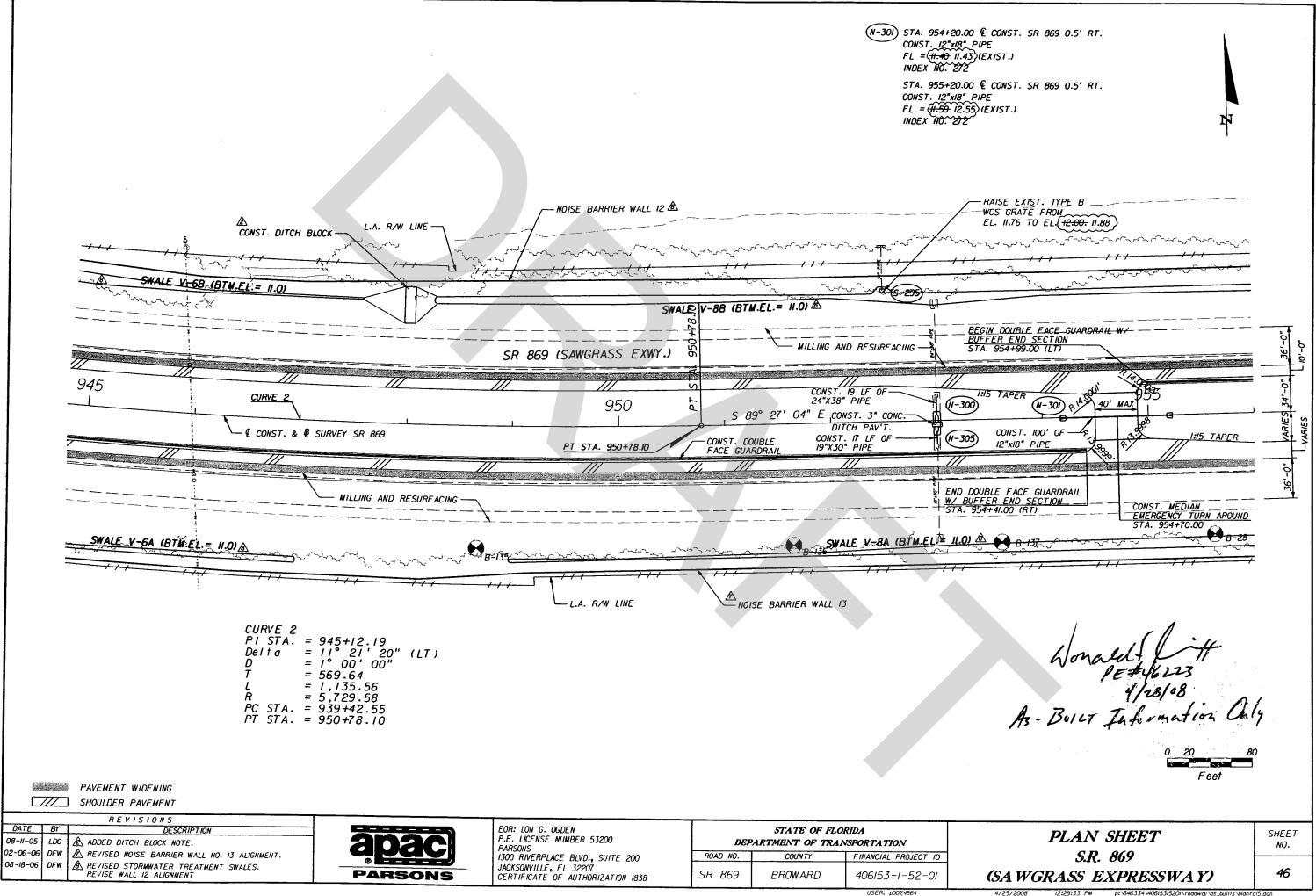




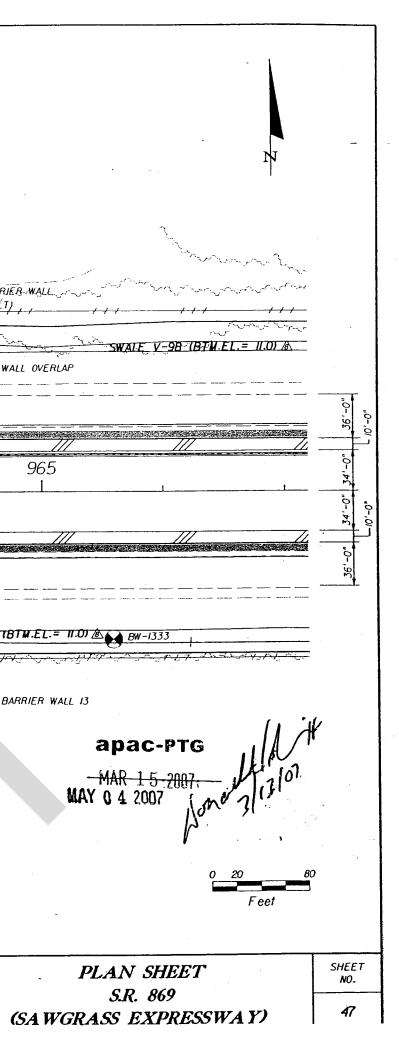




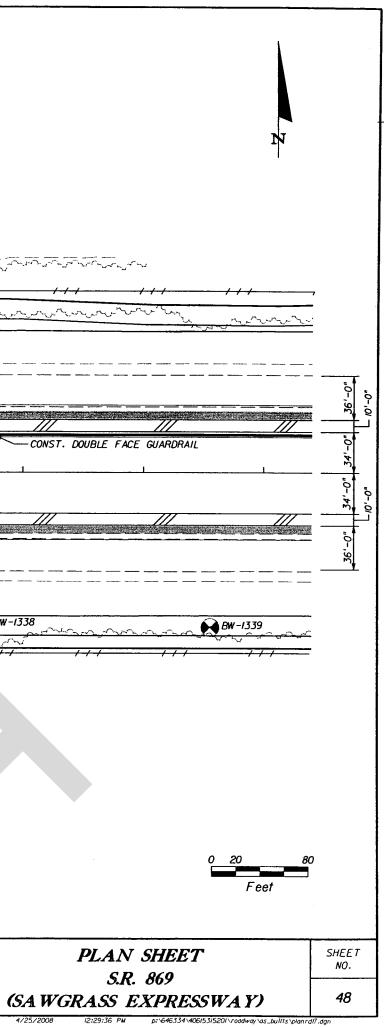




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l			STA. 962+4	BARRIER WALL OVERLAP			·	END NOISE BARRIER V A STA. 963+66-66 (LT)
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	B-2		MILLING AND RE		A. R/W LINE			B-32 SWALE V-9A
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7		4 4	MILLING AND RE		A. R/W LINE			B-32 SWALE V-9A
7	PAVEMENT WIDENING	4 4	MILLING AND RE		A. R/W LINE			B-32 SWALE V-9A
7	PAVEMENT WIDENING SHOULDER PAVEMENT	4 4	MILLING AND RE		A. R/W LINE			B-32 SWALE V-9A
	PAVEMENT WIDENING SHOULDER PAVEMENT REVISIONS	4 4	MILLING AND RE	. OGDEN		STATE	OF FLORIDA	B-32 SWALE V-9A
DATE BY 08-11-05 LDO	PAVEMENT WIDENING SHOULDER PAVEMENT R E V I S I O N S DESCRIPTION ADDED DITCH BLOCK NOTE.	SWALE V-8A TOTWEL- 11	MILLING AND RE	COGDEN SE MUNBER 53200		STATE DEPARTMENT	OF FLORIDA DF TRANSPOR	B-32 SWALE V-9A ANOISE O
DATE BY 08-11-05 LDO	PAVEMENT WIDENING SHOULDER PAVEMENT R E V I S I O N S DESCRIPTION ADDED DITCH BLOCK NOTE.	SWALE V-8A TOTWEL- 11	MILLING AND RE	COGDEN SE NUMBER 53200 PLACE BLVD., SUITE 200	ROAD NO	STATE DEPARTMENT (COUNT	OF FLORIDA DF TRANSPOR Y FIM	B-32 SWALE V-9A A NOISE D TATION WCIAL PROJECT ID
DATE BY 08-11-05 LDO	PAVEMENT WIDENING SHOULDER PAVEMENT R E V I S I O N S DESCRIPTION	SWALE V-8A TOTWEL- 11	MILLING AND RE	COGDEN SE MUNBER 53200		STATE DEPARTMENT (COUNT	OF FLORIDA DF TRANSPOR Y FIM	B-32 SWALE V-9A ANOISE O

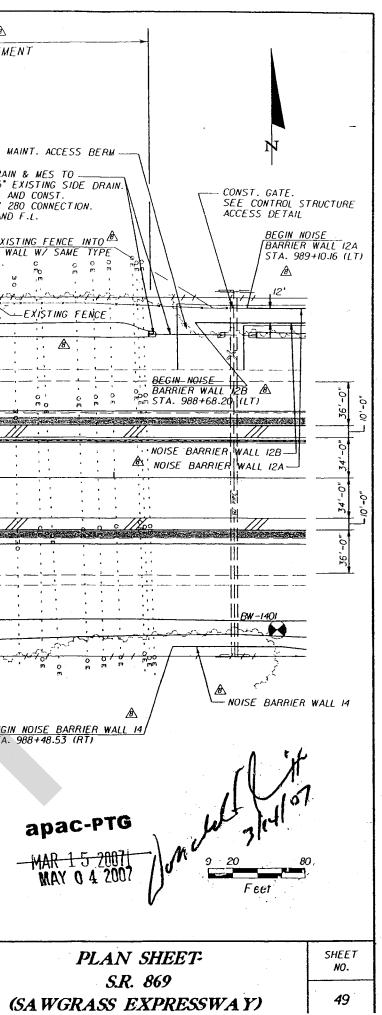


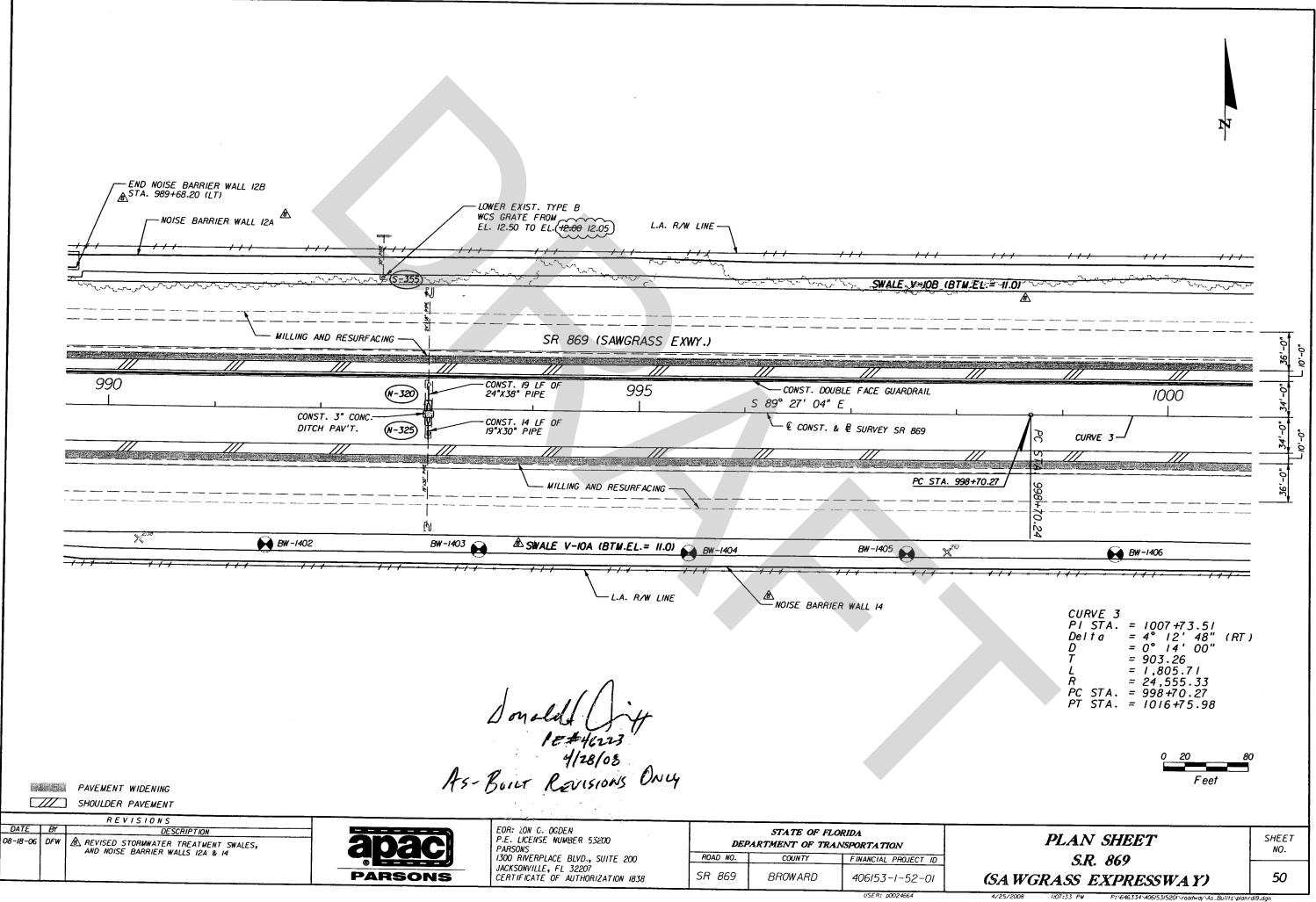
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		TO REMAIN AT EL. 12.03 (WAS REPAIRED) 11.57			۵
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	×	SWALE V-98 (BTW.EL.= 11.0)	A 5-325	<u>[].]</u>	
		AND RESURFACING -			
		WD RESORFALING		SR 869 (SAWGRAS	SS EXWY.)
	070				
1	970	S 89° 27' 04" E	(N-310)	CONST. 20 LF OF 24"X38" PIPE	975
			. 3" CONC.	CONST. 16 LF OF	
	- ¥ (0)	WST. & & SURVEY SR 869 CONST DITCH	PAV'T. (N-315)	19"X 30" PIPE	
	//////				
	~ MIL	LING AND RESURFACING		2 2	
		anne			
	BW-1335	Bw-1336 SWAL	E V-9A (BTM.EL.=	H.0) & DW-1337	
DW 1334	Bw-1335	BW-1336 SWALL	E V-9A (BTM.EL.=	BW-1337	
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		BW-1336 SWALL	E V-9A (BTM.EL.=		
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		/			
	NOISE BARRIER WALL 13	▲ <u>END NOISE BARRIEF</u> STA. 983+48.43 (RT	WALL	L.A. R/W LIN	E
		STA. 983+48.43 (RI	·)		
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PAVEMENT WIDENING					
REVISIONS			······································		·····
DATE BY DESCRIPTING		P.E. LICI	I G. OGDEN ENSE NUMBER 53200		OF FLORIDA F TRANSPORTATION
02-06-06 DFW 🛆 REVISED NOISE BARRIER WALL		PARSONS 1300 RM	ERPLACE BLVD., SUITE 200	ROAD NO. COUNT	Y FINANCIAL PROJECT ID
08-18-06 DFW A REVISED TREATMENT SWALES.	AND NOISE WALLS 12, 12A,		VILLE, FL 32207 TATE OF MITHORIZATION 1838	SR 869 BROWA	RD 406153-1-52-01

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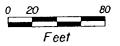


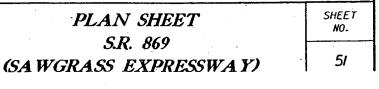
· · ·				
/ / / /	CONST. DITCH BLOCK	L.A. R/W LINE	<i> </i>	···
SWALE V-IOB (BTWEL11:0)	And the second and th		SWALE V-IIB (BTM.EL	.= 11.0)
	- +	SR 869 (SAWGRASS E	xwy.)	
		1005 CONST. DOUBLE FAC		
<u>_</u>	l	CURVE 3	€ CONST. & € SURVEY SR 869	t
			MILLING AND RESURFACING	X
SWALE V-10A (BTM.EL.= 11.0 BW-1407)) 	BW-1409 SWALE V-11A	(BTM.EL.= 11.0) A BW-1410	
	CONST. DITCH BLOCK	L.A. R/W LINE		Noise
	ECONST. DITCH BLOCK			NOISE
	ECONST. DITCH BLOCK	CURVE 3 PI STA. = 1007 +73. Delta = 4° 12' 4 D = 0° 14' 0 T = 903.26 L = 1,805.71	8" (RT) 0"	Moise Noise
	CONST. DITCH BLOCK	CURVE 3 PI STA. = 1007+73. Delta = 4° 12' 4 D = 0° 14' 0 T = 903.26	8" (RT) O" 3 4	Noise
PAVEMENT WIDENING	CONST. DITCH BLOCK	CURVE 3 PI STA. = 1007+73. Delta = 4° 12' 4 D = 0° 14' 0 T = 903.26 L = 1,805.71 R = 24,555.3 PC STA. = 998+70.2	8" (RT) O" 3 4	NOISE

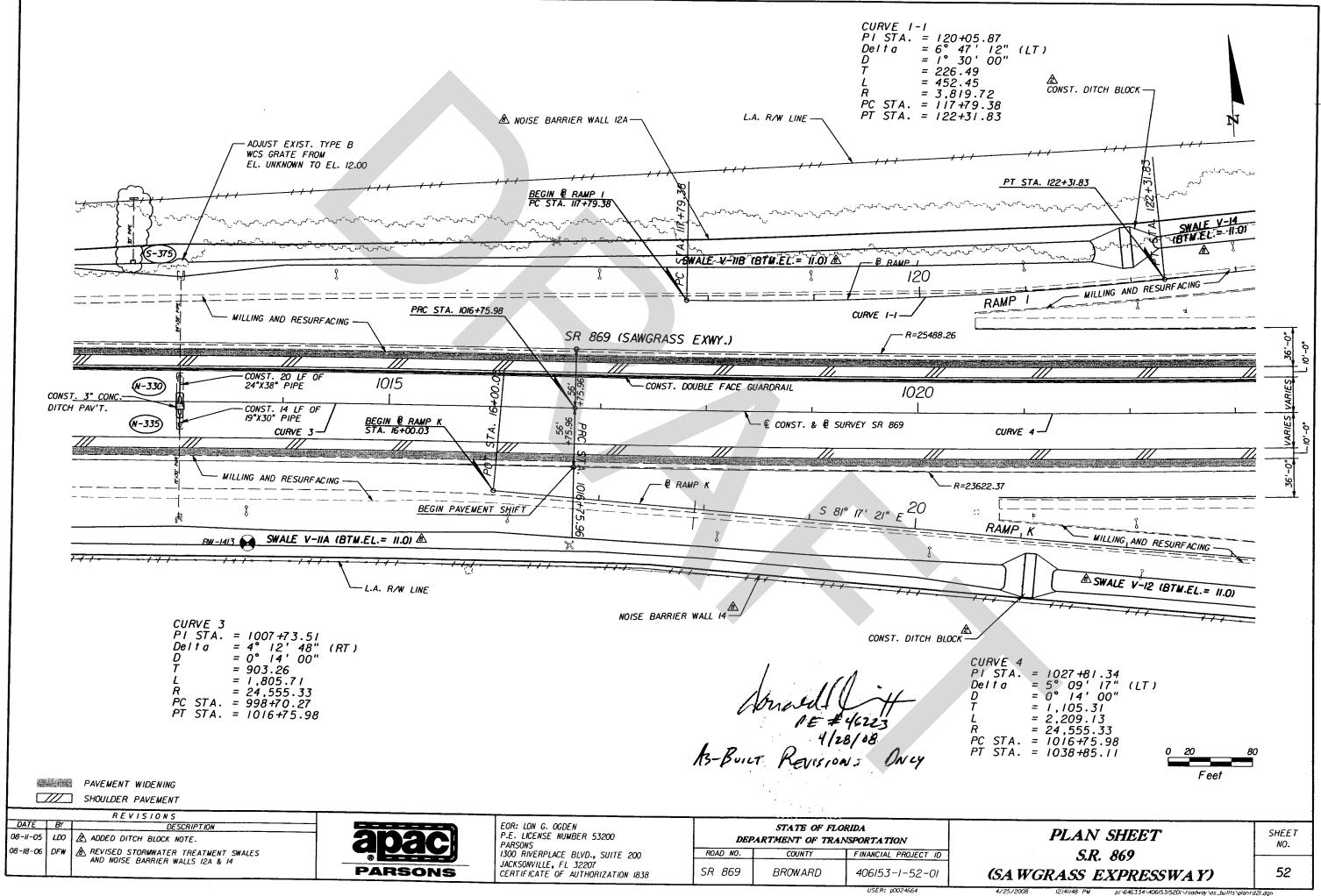
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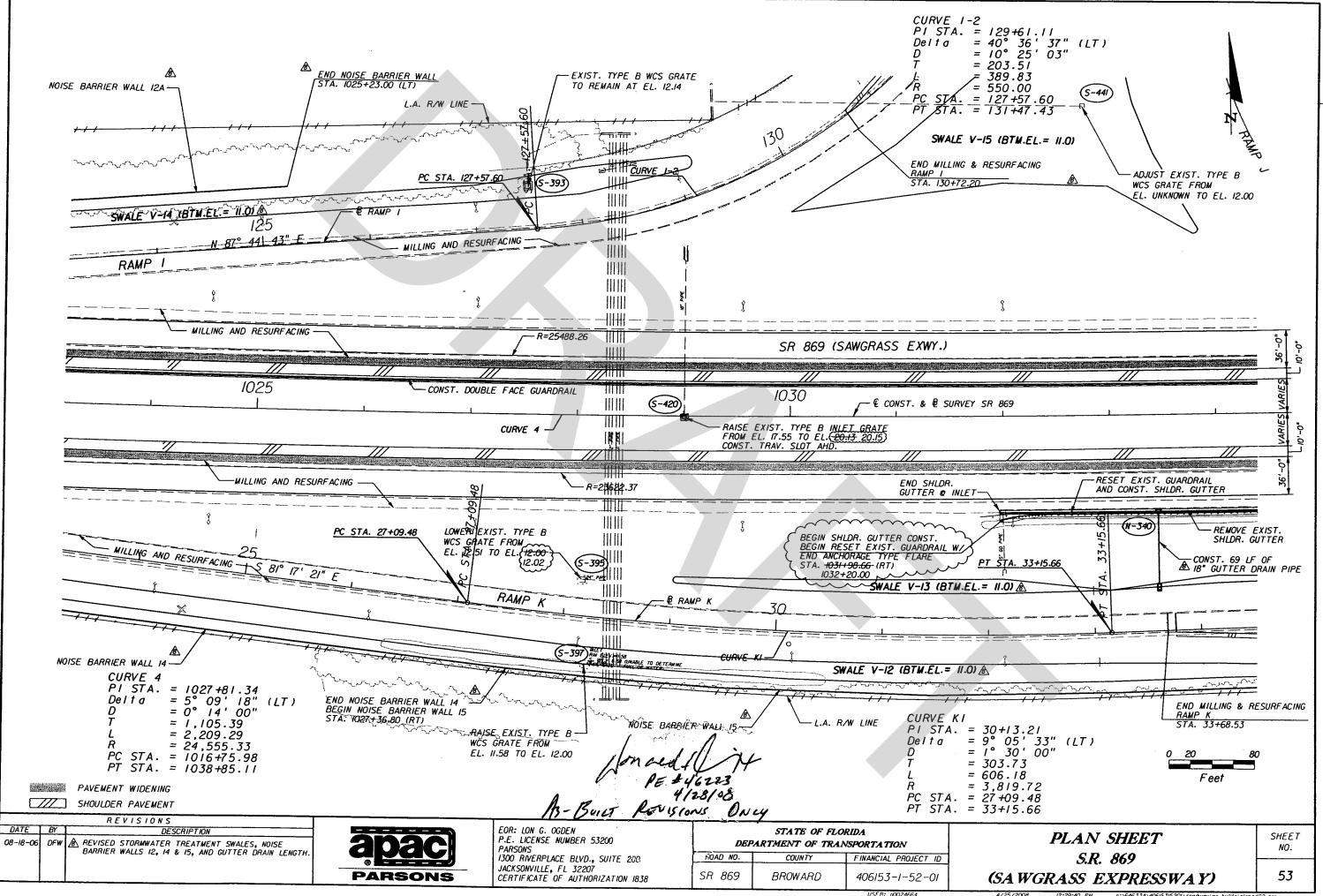
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ARRIER WALL 14

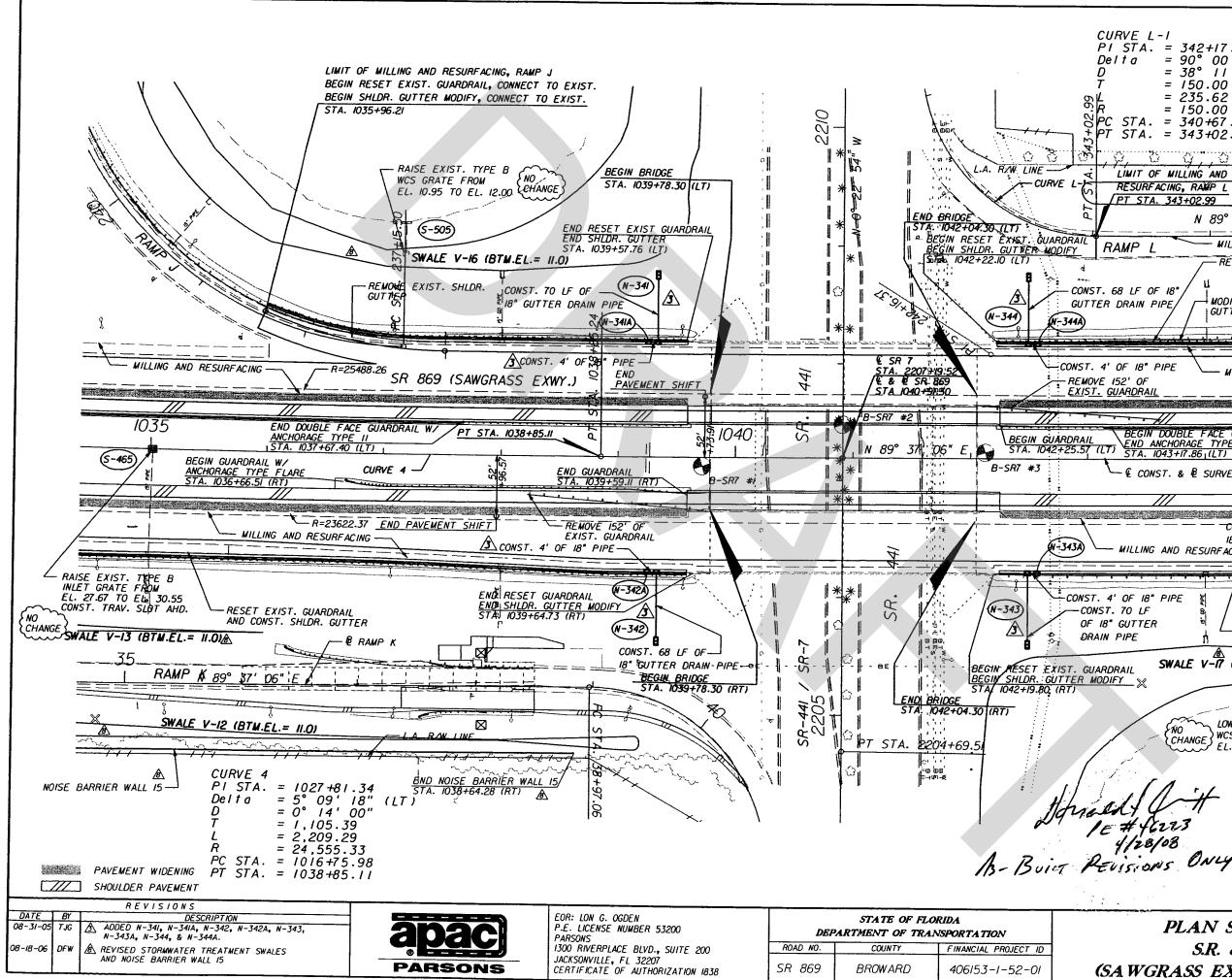




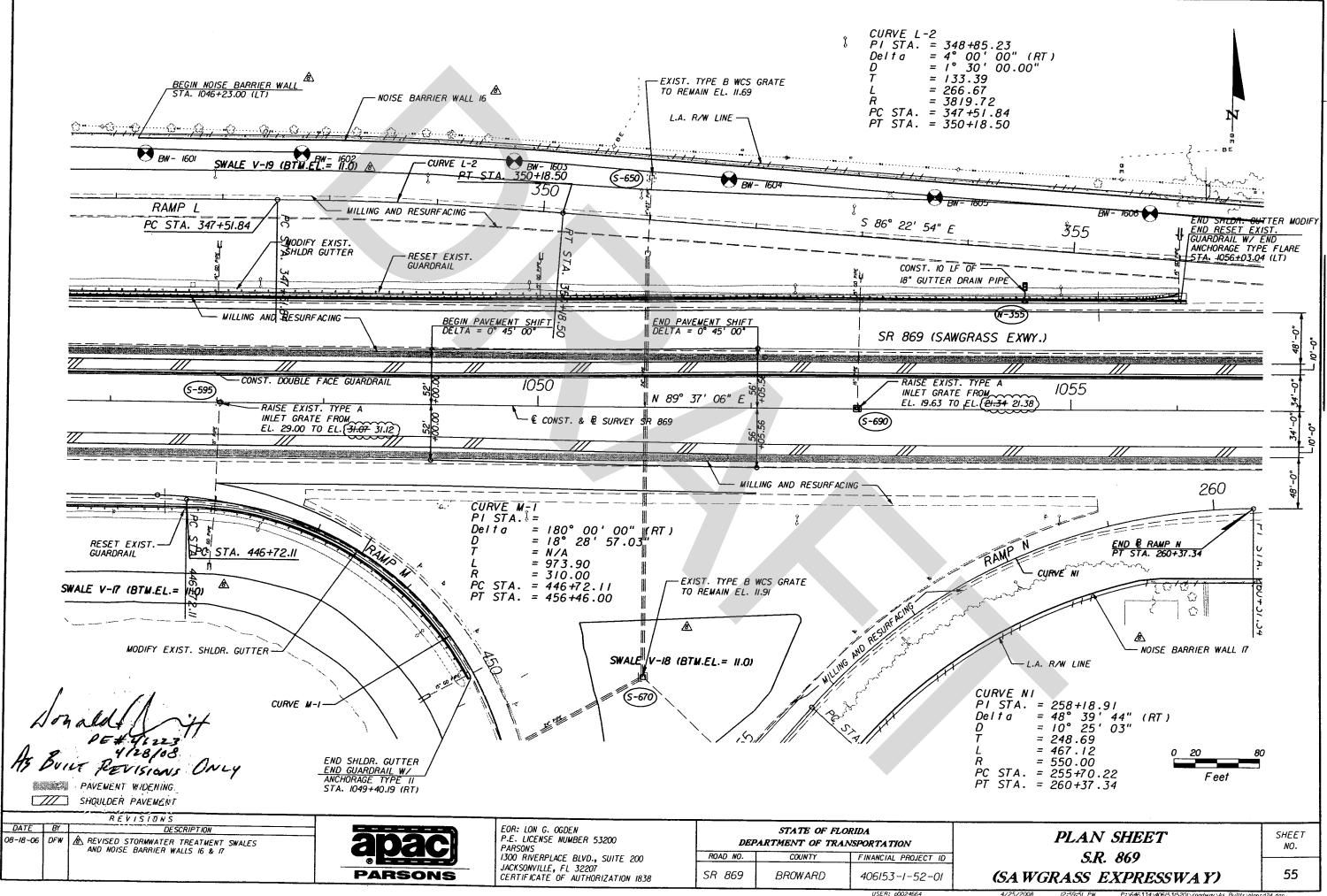


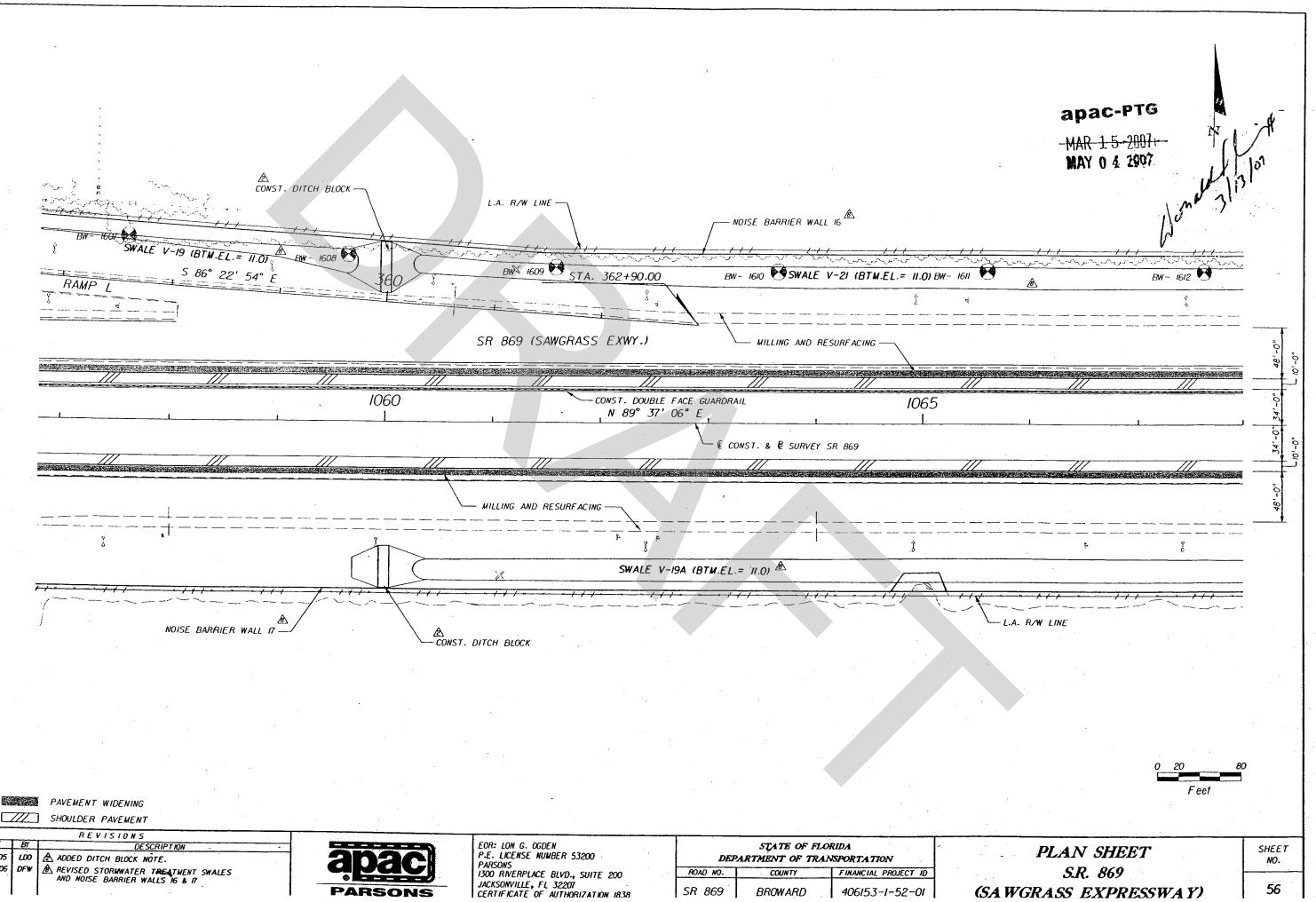


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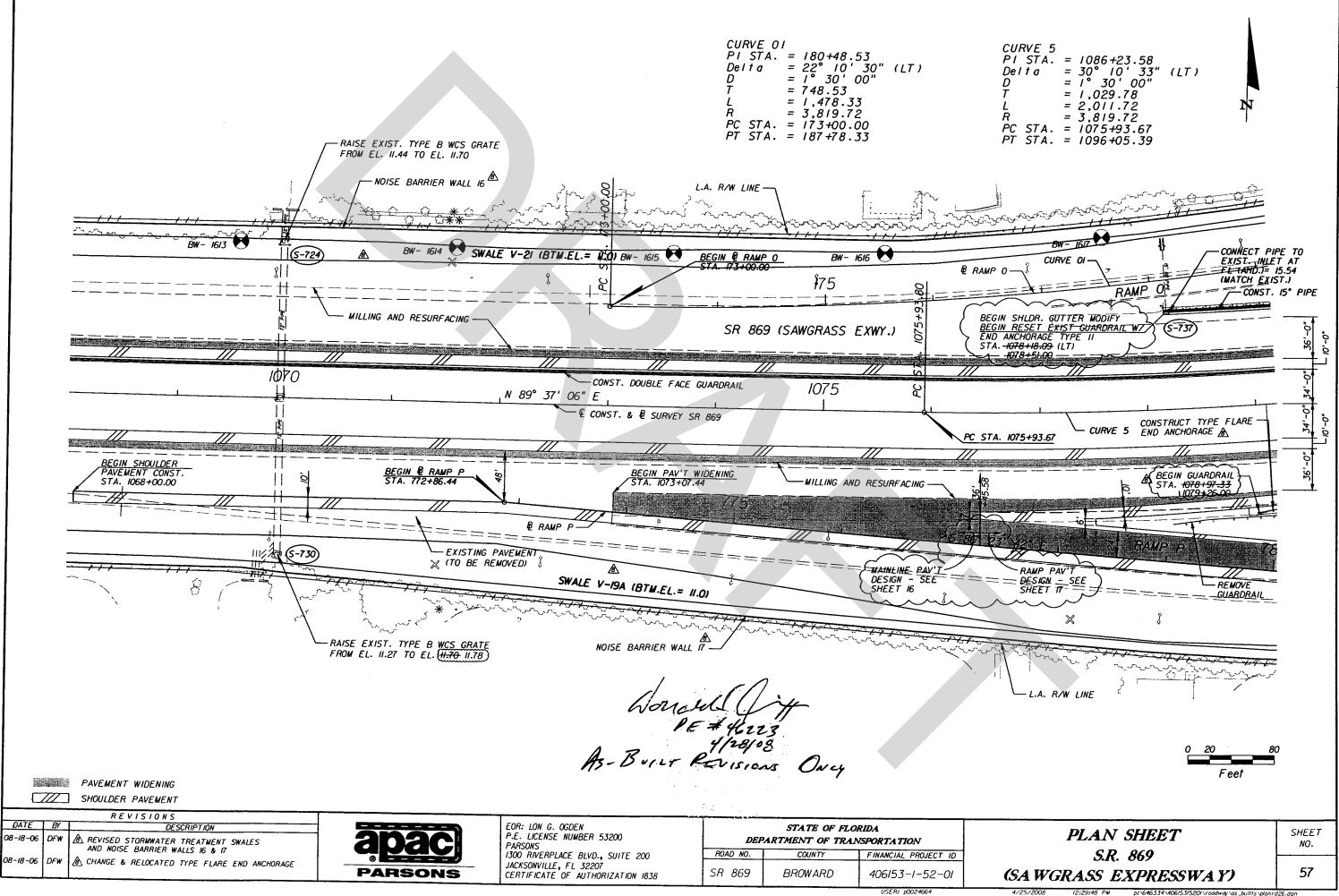


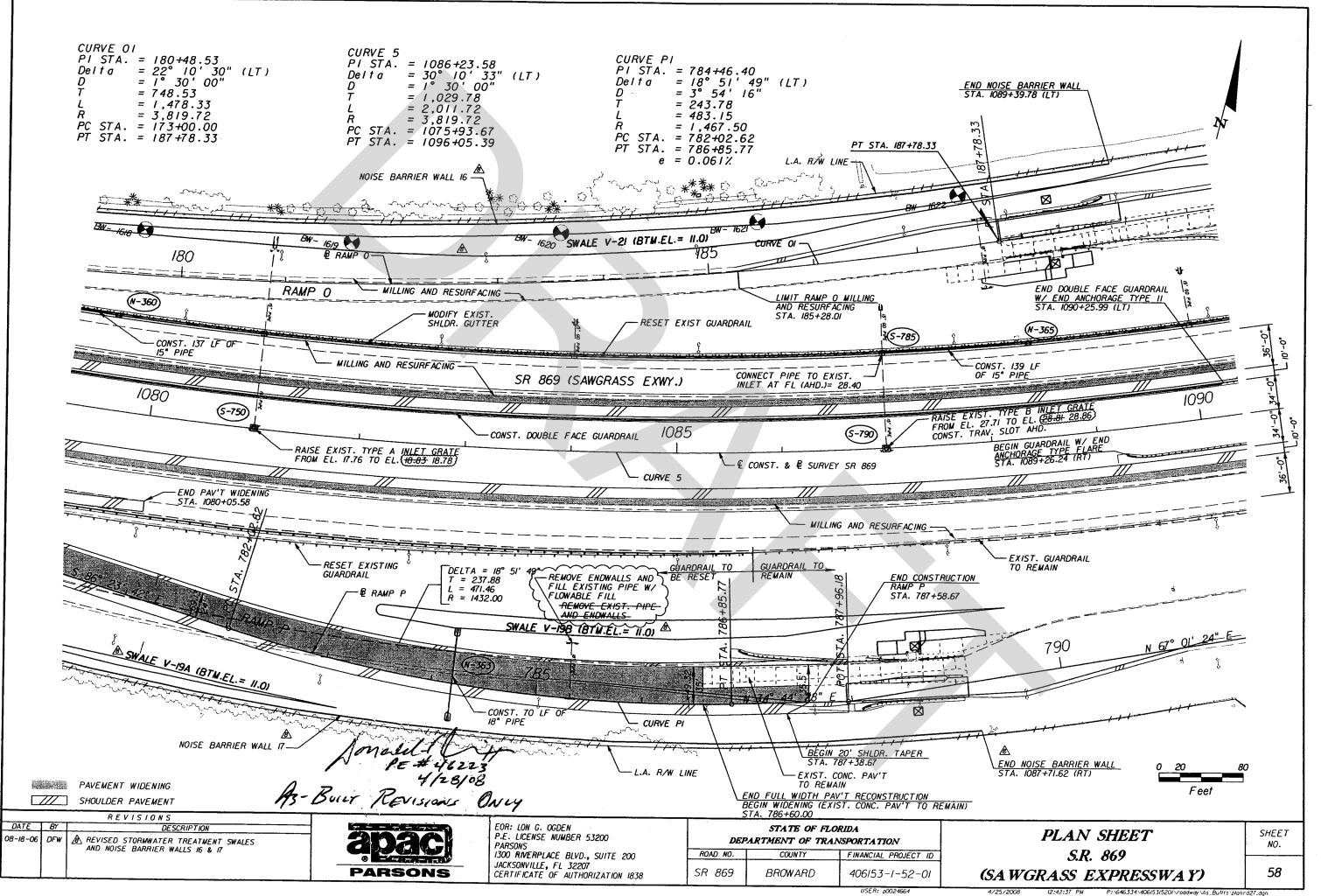
CURVE L-I PI STA. = 342 + 17.37Delta = 90° 00' 00" (LT) D = 38° 11' 49.87" = 150.00 = 235.62 = 150.00 8PC STA. = 340+67.37 PT STA. = 343+02.99 õ <u>õ, ö</u> ö LIMIT OF MILLING AND SWALE V-19 RESURFACING, RAMP L PT STA. 343+02.99 (BTM.EL.= 11.0) ▲ N 89° 37' 06" E 345 MILLING AND RESURFACING RESET EXIST. GUARDRAIL CONST. 64 LF OF 18" GUTTER DRAIN PIPE MODIFY EXIST. SHLDR. GUTTER N-345 MILLING AND RESURFACING BEGIN GUARDRAIL BEGIN DOUBLE FACE GUARDRAIL W/ 1045 STA. 1042+25.57 (LT) STA. 1043+17.861(LT) € CONST. & € SURVEY SR 869 CONST. 71 LF OF 18" GUTTER DRAIN PIPE-MILLING AND RESURFACING (N-350) REMOVE EXIST. SHLDR. GUTTER -RESET EXIST. GUARDRAIL SWALE V-17 (BTM.EL.= 11.0) (5-565) LOWER EXIST. TYPE B NO CHANGE WCS GRATE FROM CHANGE EL. 12.15 TO EL. 12.00 CONC. FLUME AND . 32" CONC. HEADWALL 20 Feet PLAN SHEET SHEET NO. S.R. 869 54 (SA WGRASS EXPRESSWAY) p:\646334\40615315201\roadway\as_bullts\planrd23.dgr

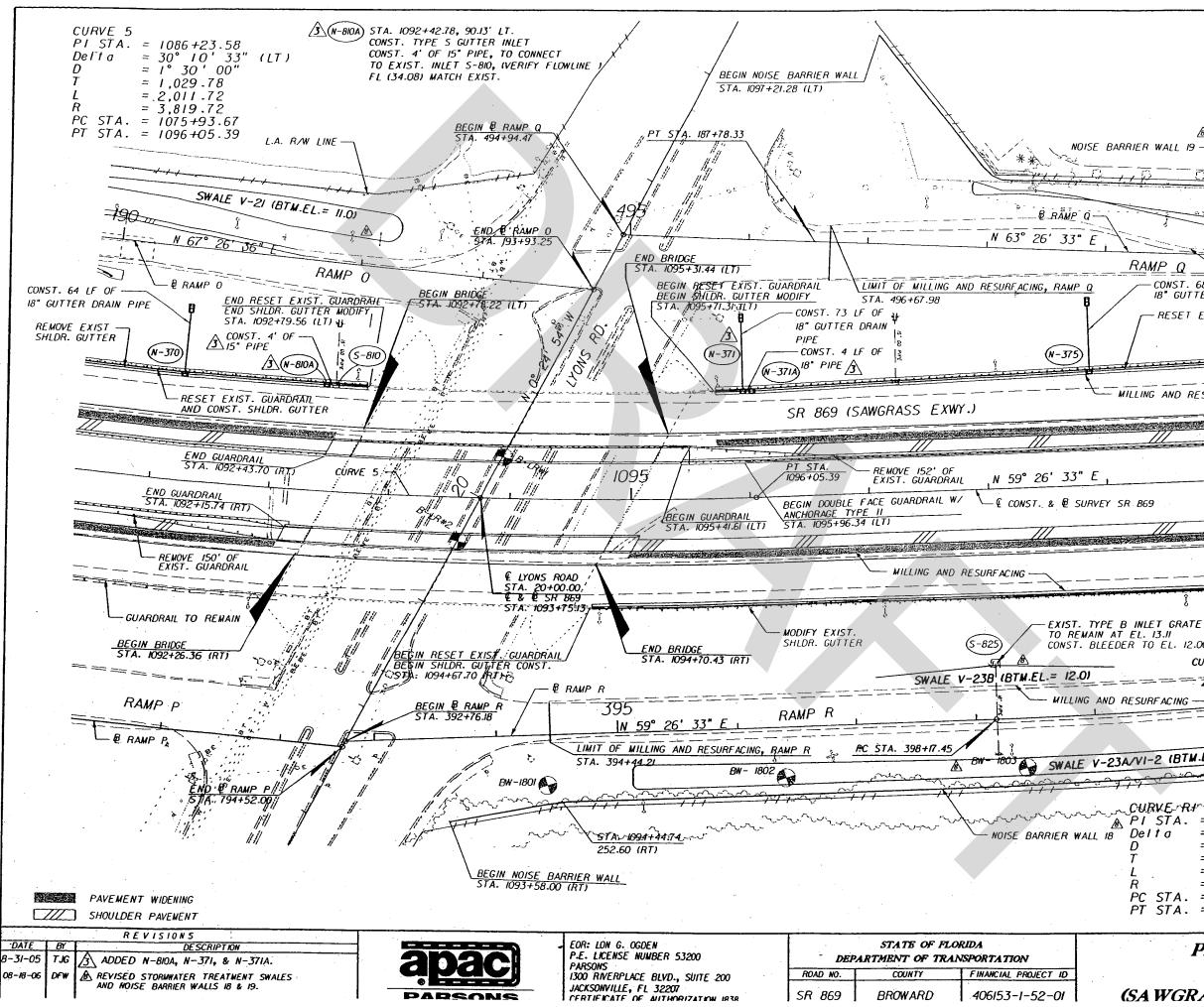




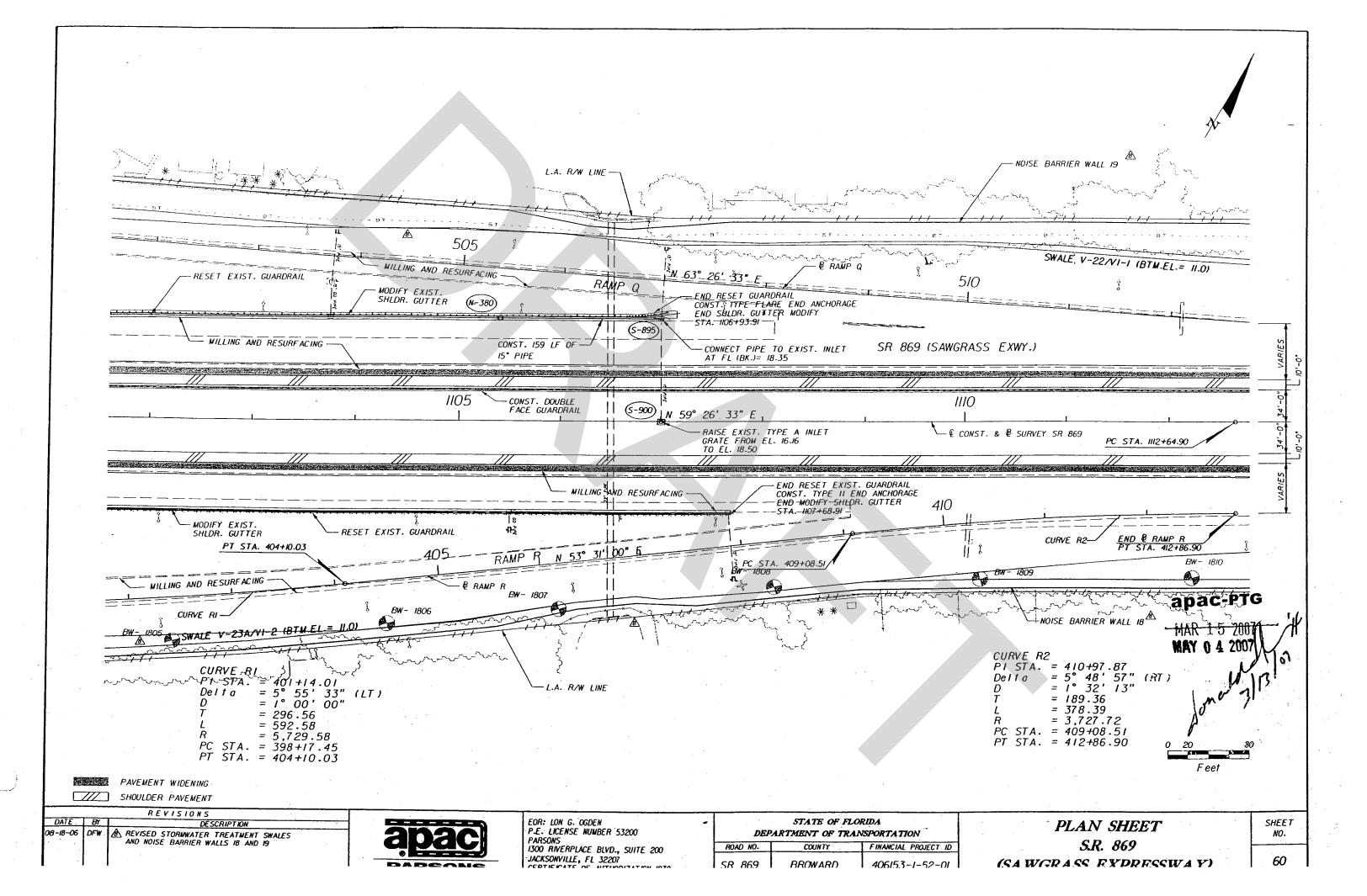
SHOULDER PAVEMENT					
REVISIONS DATE BY DB-II-05 LD0 ADDED DITCH BLOCK NOTE.	apac	EOR: LON G. OGDEN P.E. LICENSE NUMBER 53200 PARSONS	DEP,	STATE OF FLA ARTMENT OF TRA	
08-18-06 DFW A REVISED STORMWATER TREATMENT SWALES AND NOISE BARRIER WALLS 16 & 17	PARSONS	1300 RIVERPLACE BLVD., SUITE 200 JACKSONVILLE, FL 32207 CERTIFICATE OF AUTHORIZATION 1838	road no. SR 869	COUNTY BROWARD	FINANCIAL PROJECT ID 406153-1-52-01

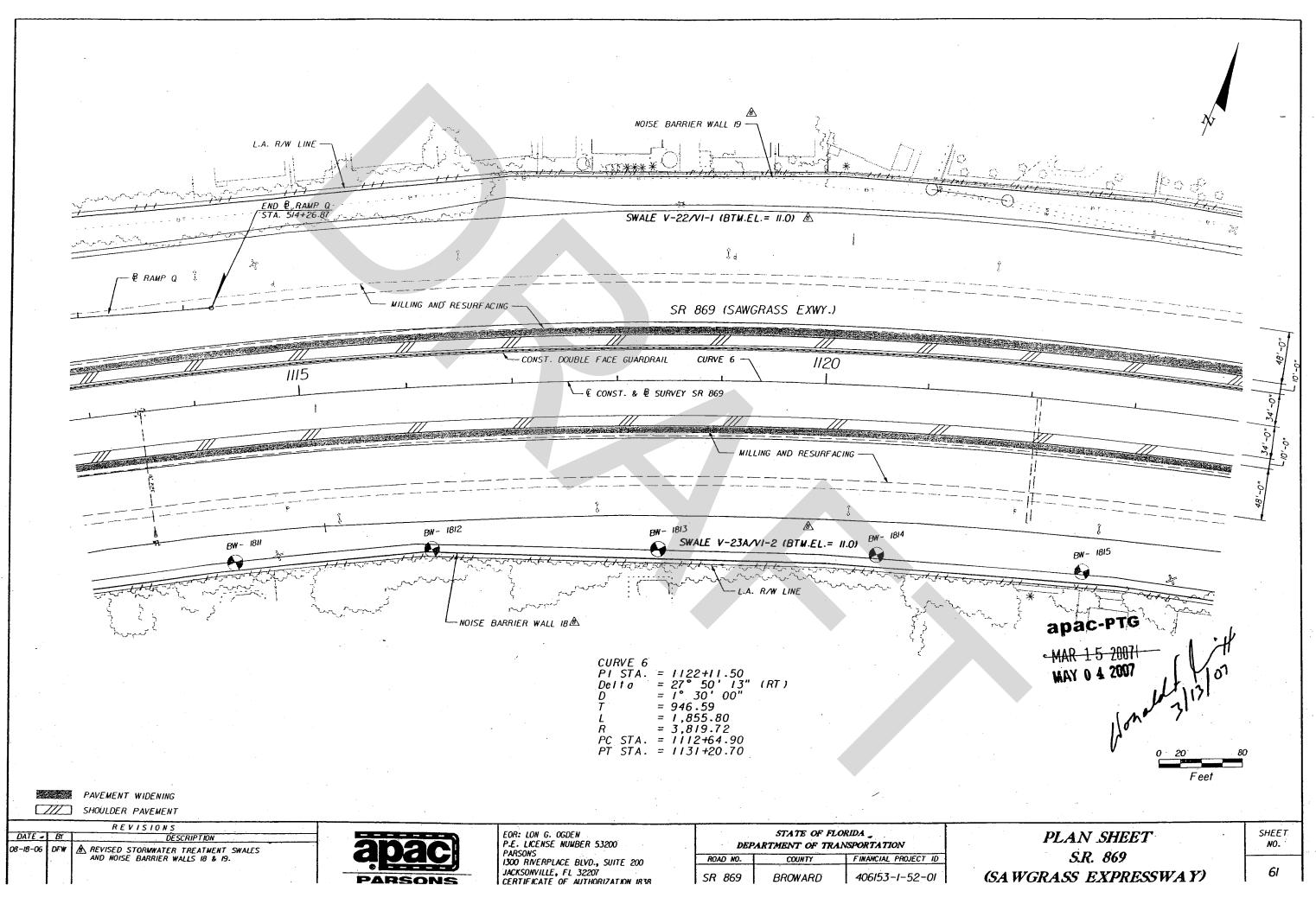




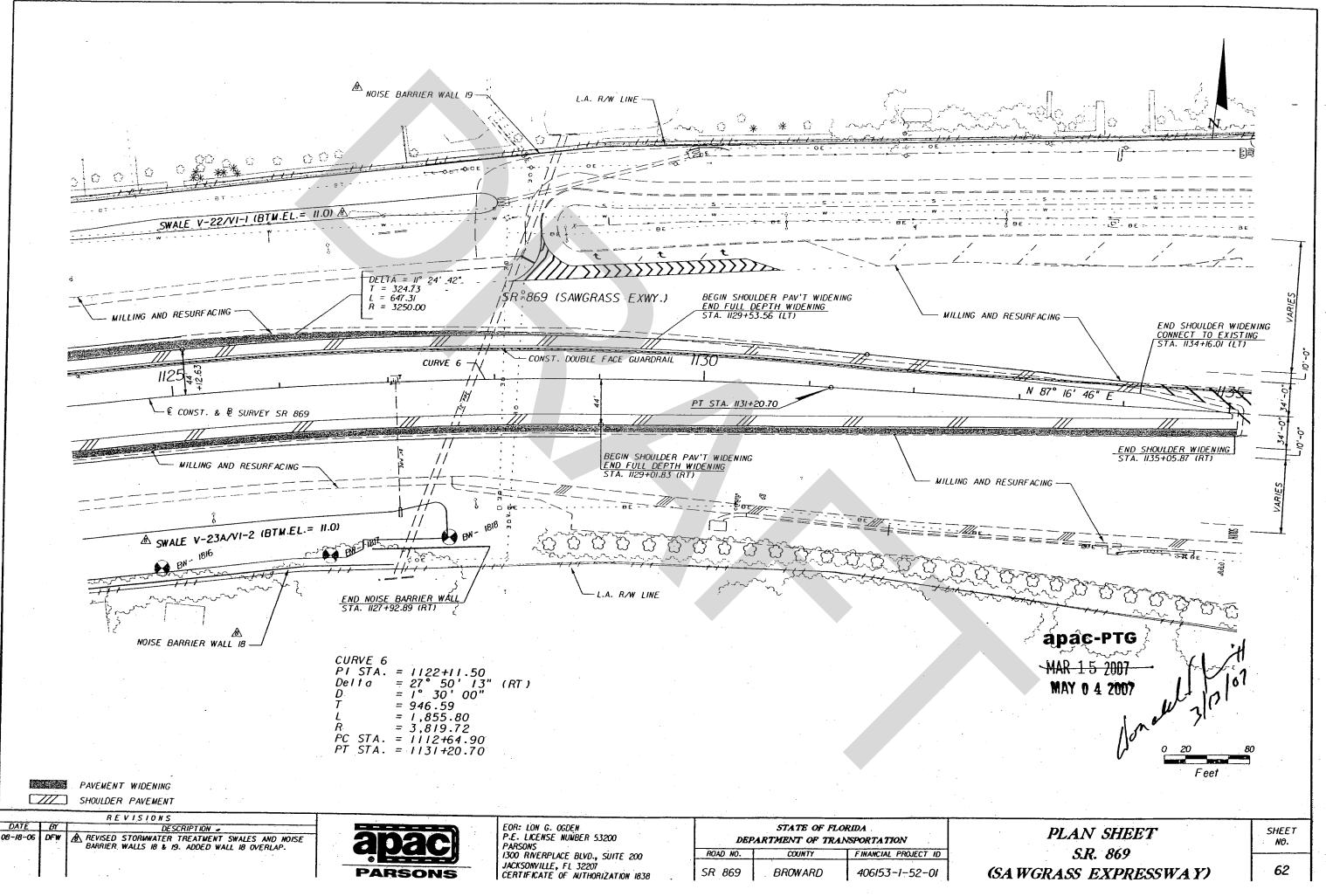


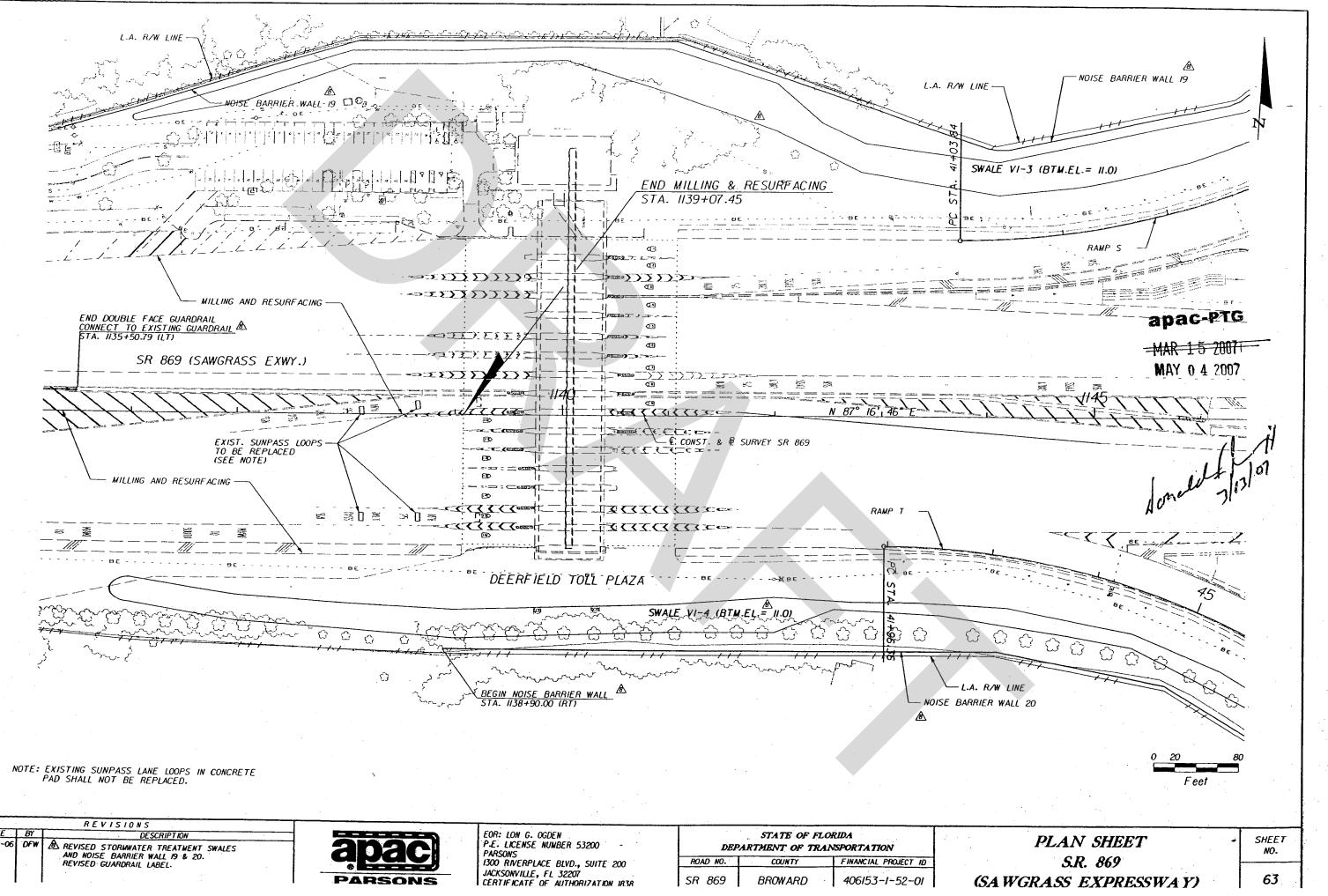
ß L.A. R/W INS NOISE BARRIER WALL 19 * ASWALE V-22/VI-1 (BTM.EL.= 11.0) RAMP_Q MILLING AND RESURFACING - CONST. 68 LF OF IB" GUTTERDRAIN RESET EXIST. GUARDRAIL MILLING AND RESURFACING 1100 (s-850) RAISE EXIST. TYPE A INLET GRATE FROM EL. 27.44 IO-EL- 29.86 RESET EXIST. GUARDRAIL apac-PTG CONST. BLEEDER TO EL. 12.00 MAR 15-2007-CURVE F ====207MAY 0 4 2007 SWALE V-23A/VI-2 (BTM.EL.= 11.0) ÷ĹĂ. R∕W LINE CURKE-RI-~ A PI STA. = 401+14.01 = 5° 55' 33" Delta (IT)= 1° 00' 00" 20 = 296.56 = 592.58 Feet = 5,729.58 PC STA. = 398+17.45 PT STA. = 404 + 10.03SHEET PLAN SHEET NO. S.R. 869 59 (SA WGRASS EXPRESSWAY)



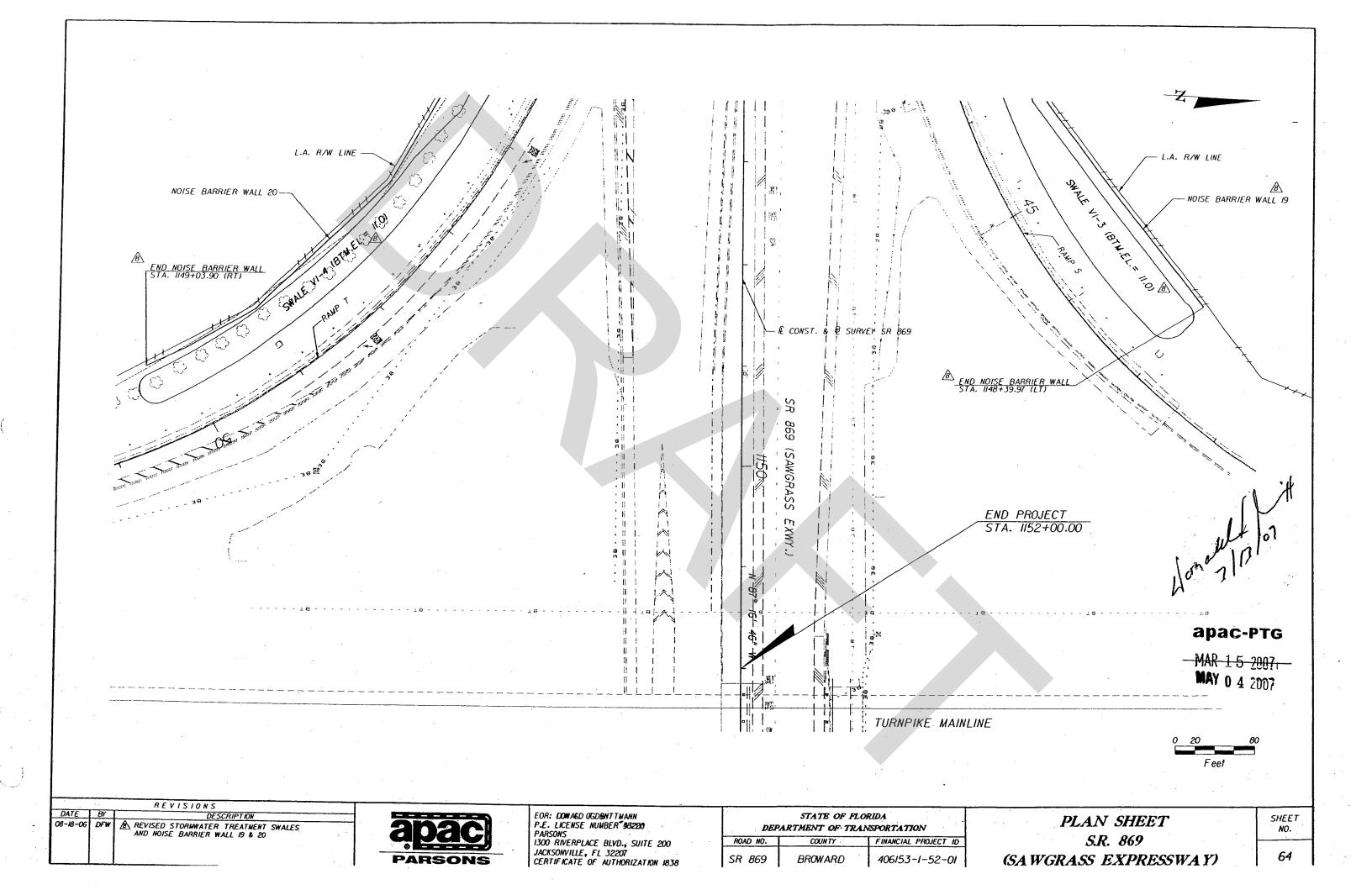


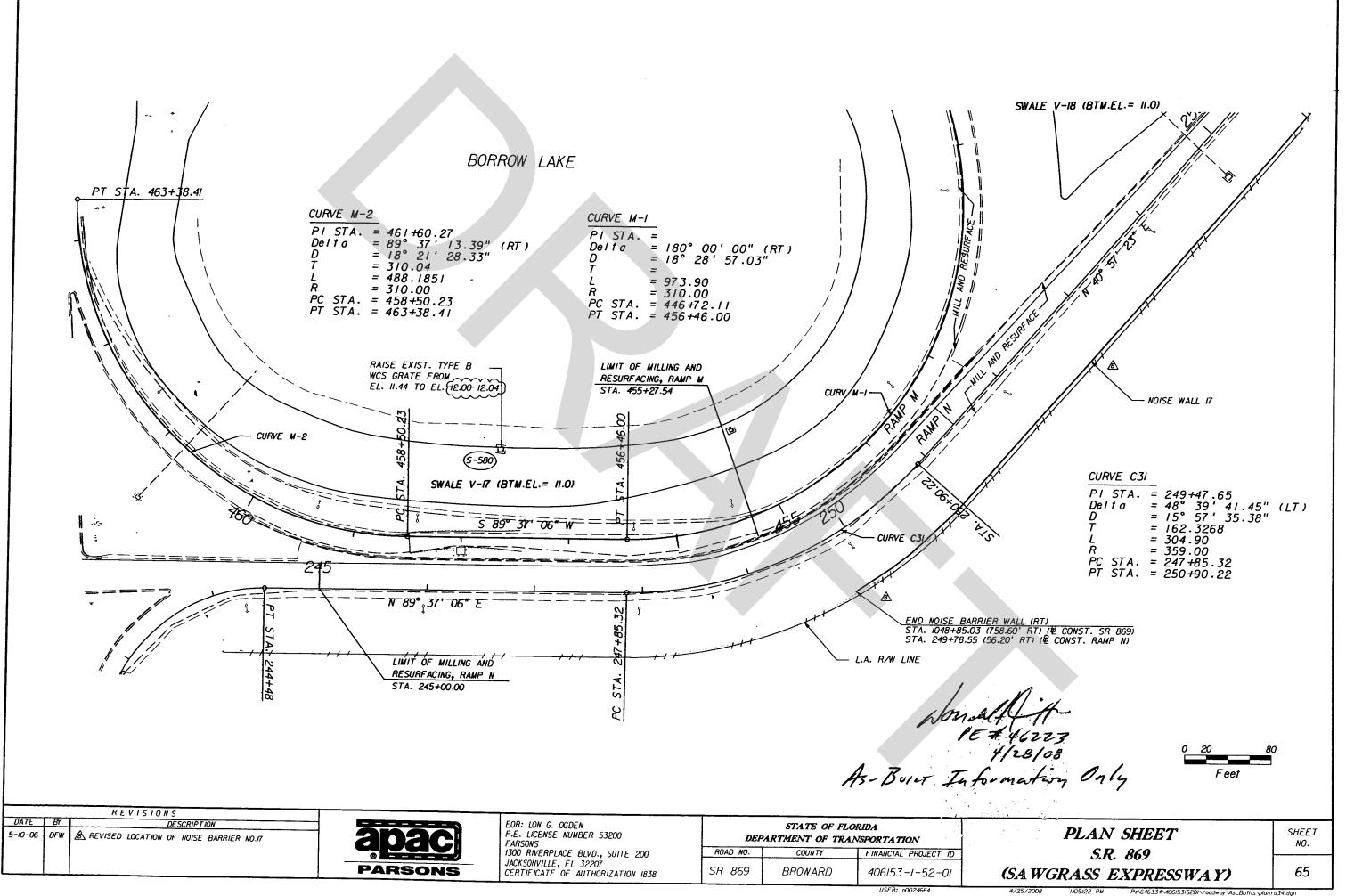
STATE OF FLORIDA _ DEPARTMENT OF TRANSPORTATION						
ROAD NO.	COUNTY	FINANCIAL PROJECT ID				
R 869	BROWARD	406153-1-52-01				

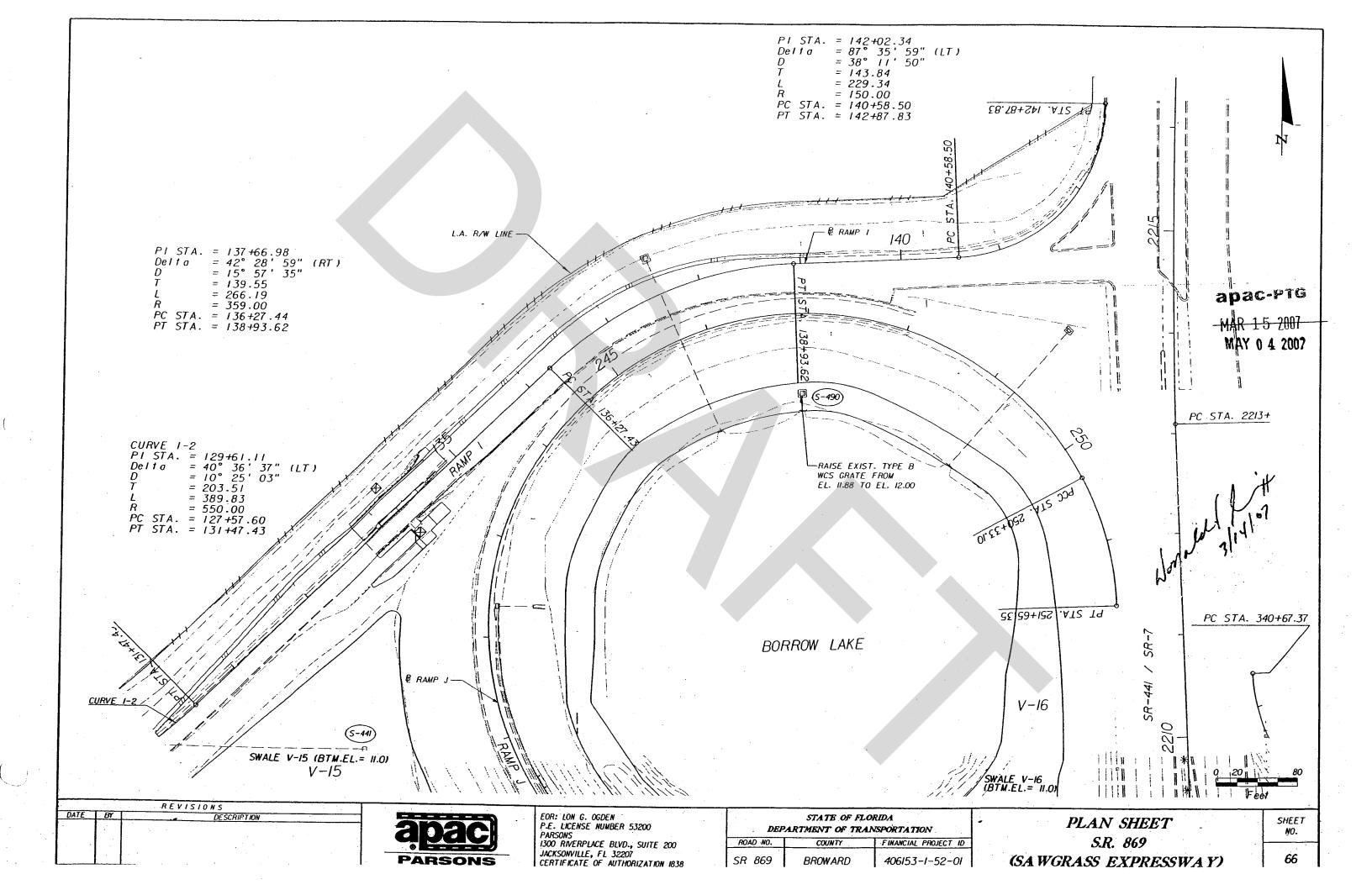


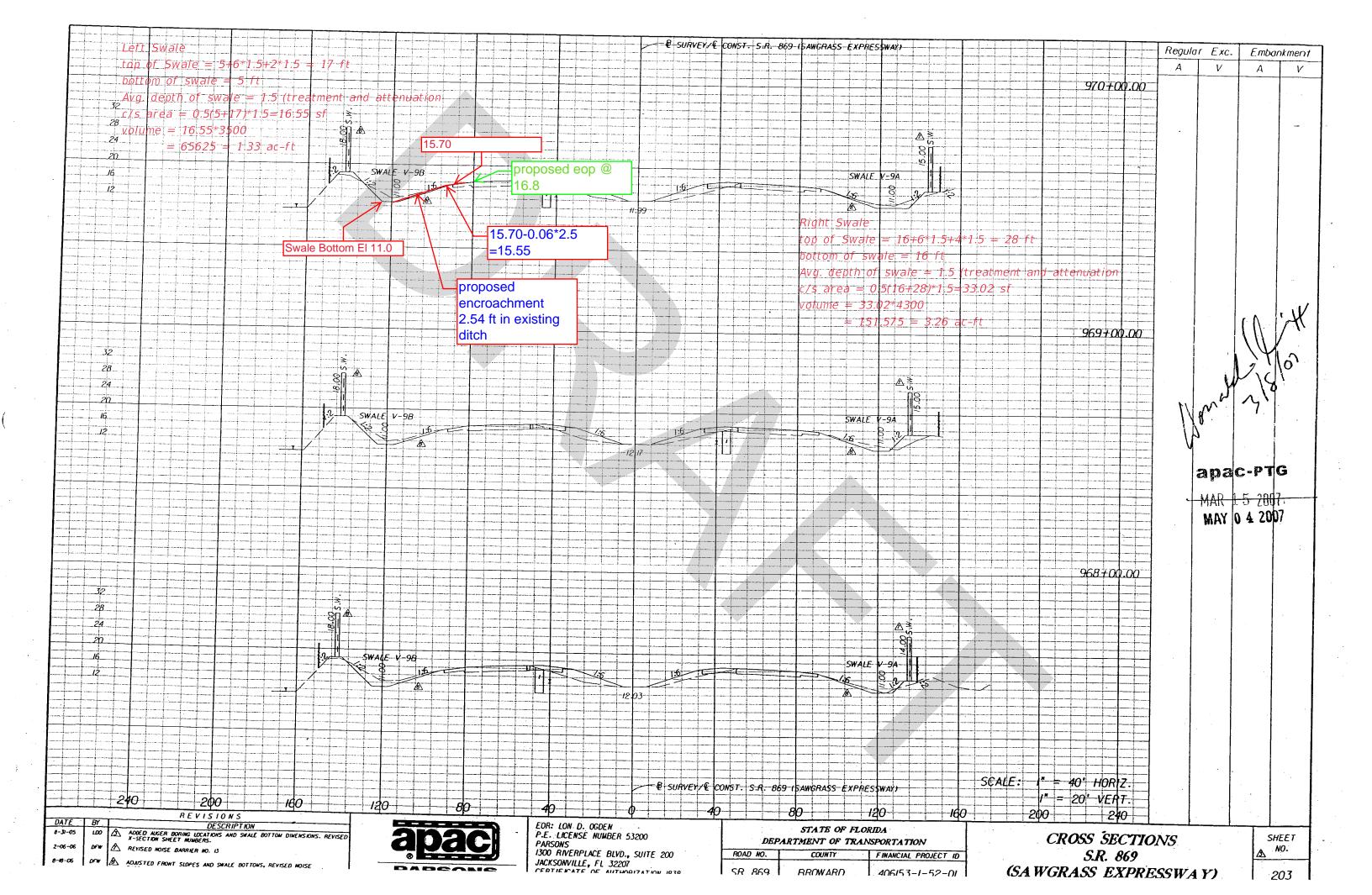


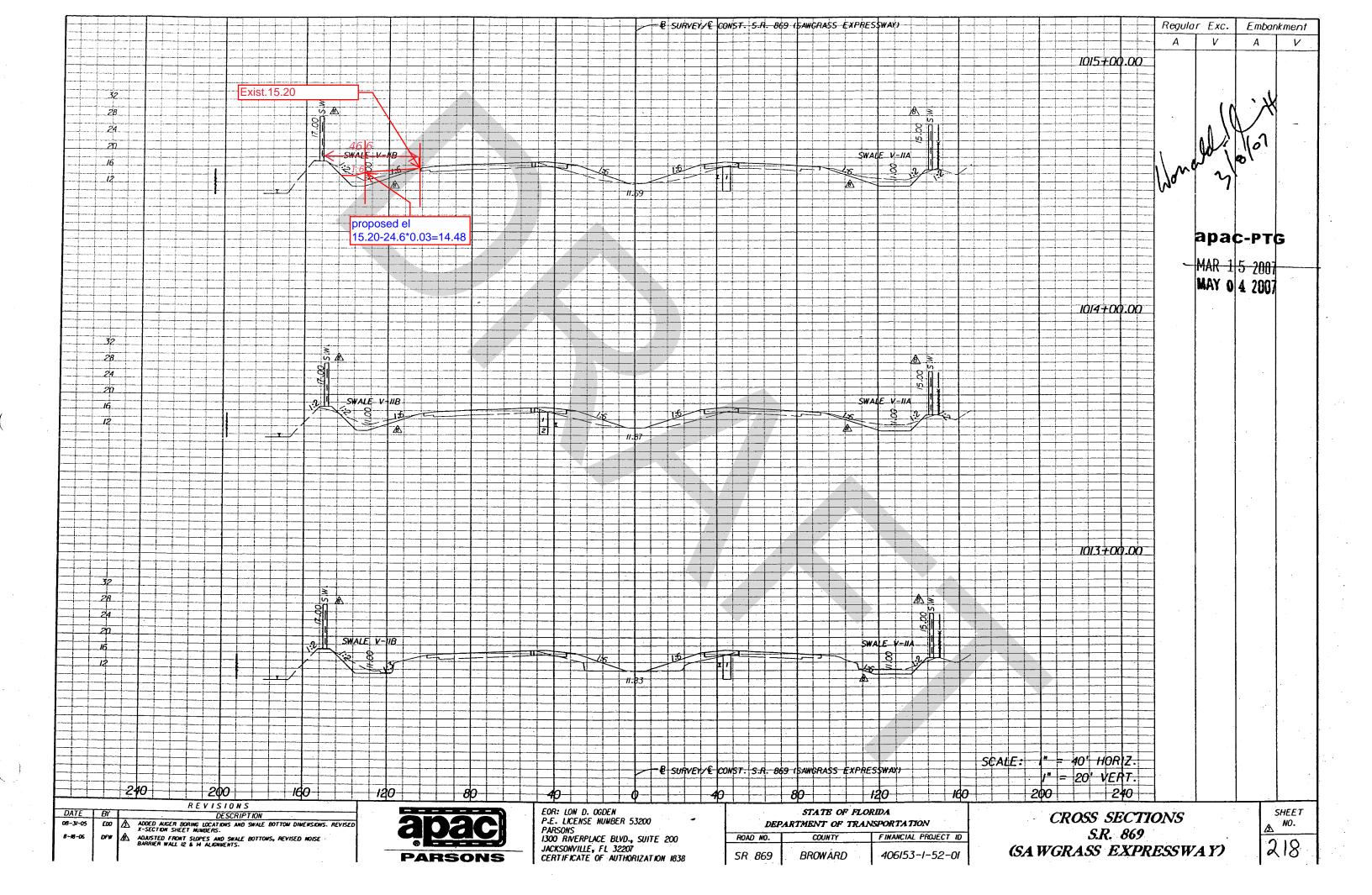
	REVISIONS	T	T				
DATE	BY DESCRIPTION		EOR: LON G. OGDEN		STATE OF FL	ORIDA	B. B. B. S.
08-18-06	DFW A REVISED STORMWATER TREATMENT SWALES		P.E. LICENSE NUMBER 53200 - PARSONS	DEF	PARTMENT OF TRA	INSPORTATION	l ·
1 [AND NOISE BARRIER WALL 19 & 20. REVISED GUARDRAIL LABEL.	avac	1300 RIVERPLACE BLVD., SUITE 200	ROAD NO.	COUNTY	FINANCIAL PROJECT ID	1 . ·
		PARSONS	JACKSONVILLE, FL 32207	SR 869	BROWARD	406153-1-52-01	6

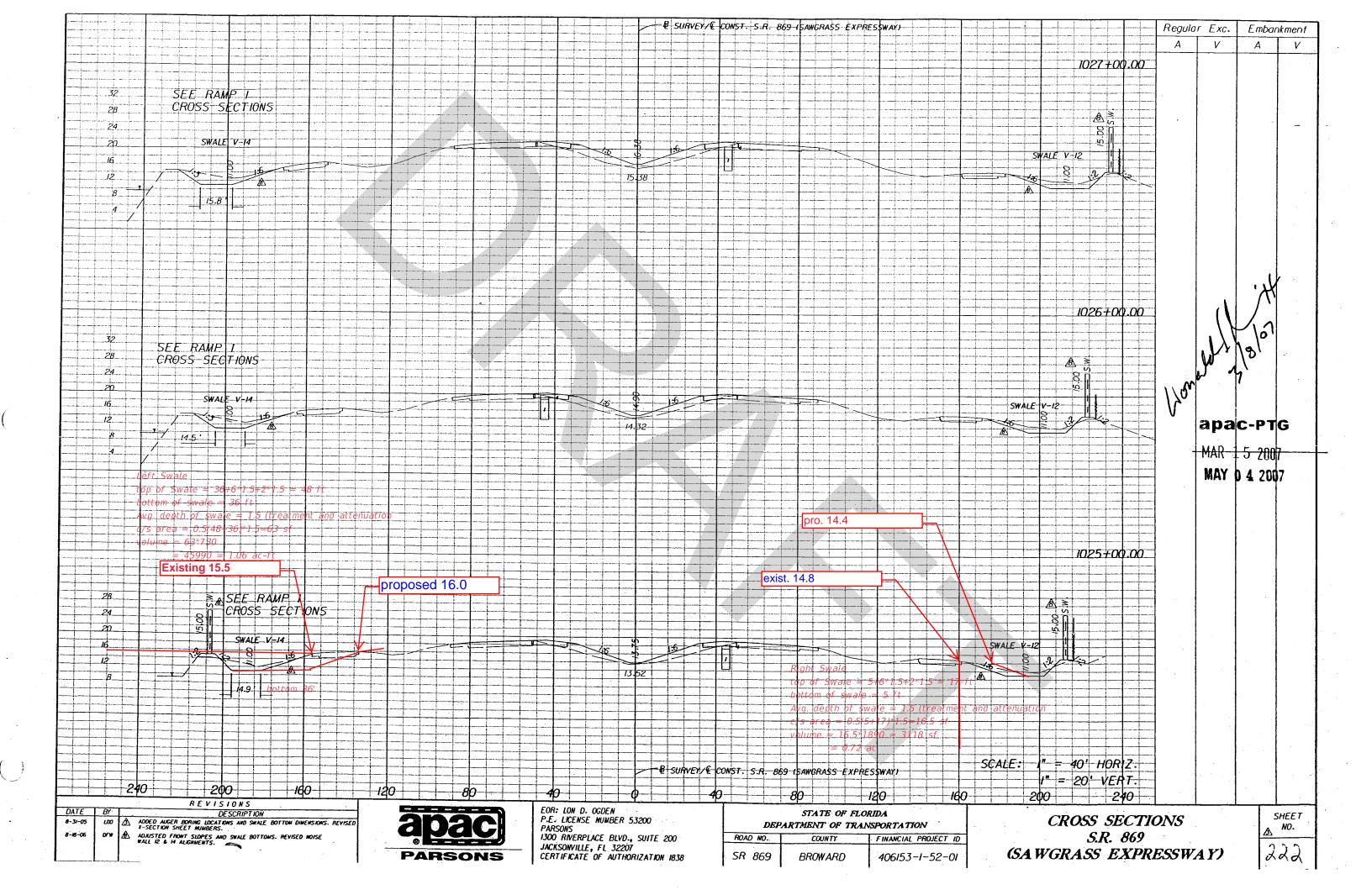


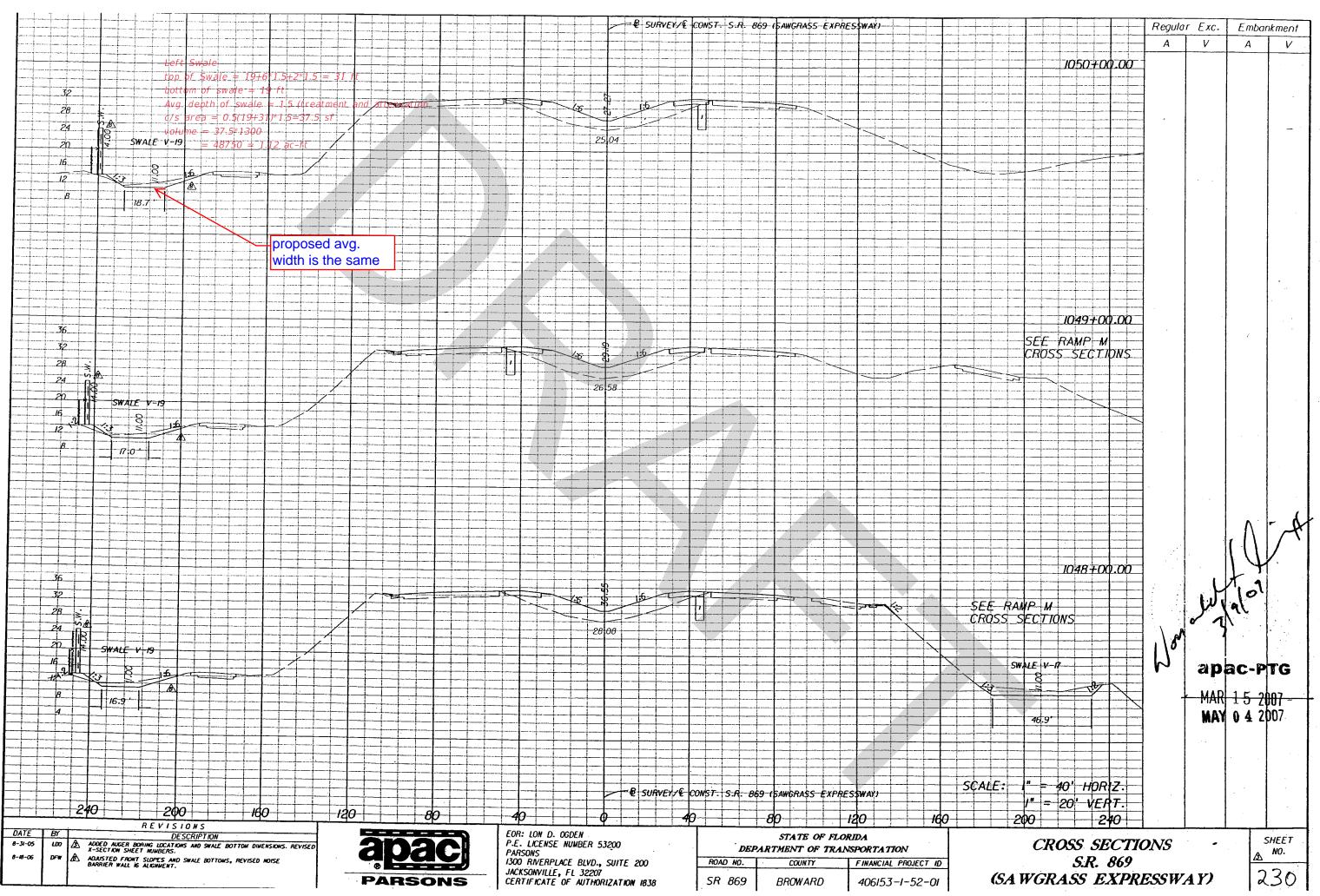


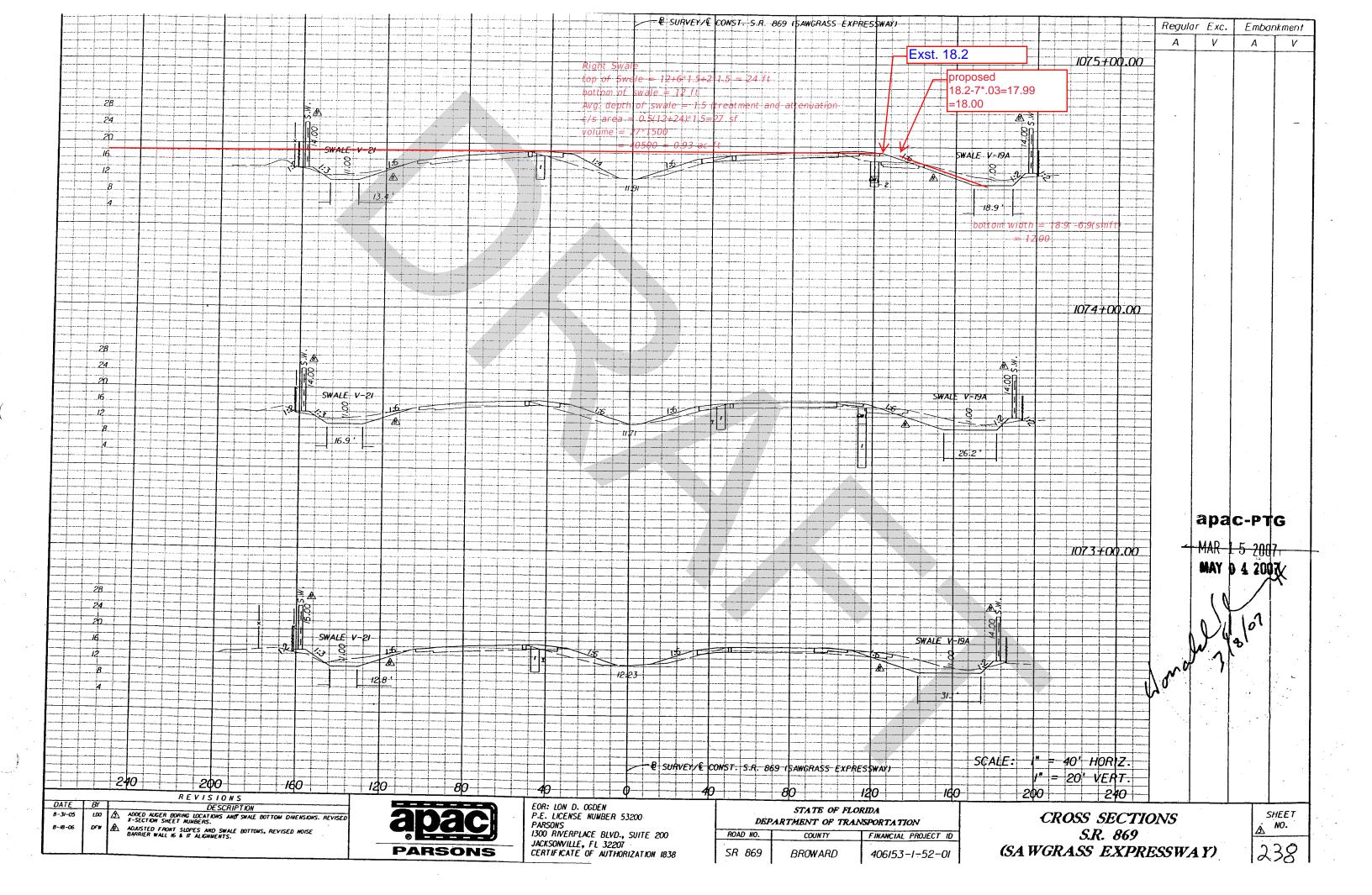




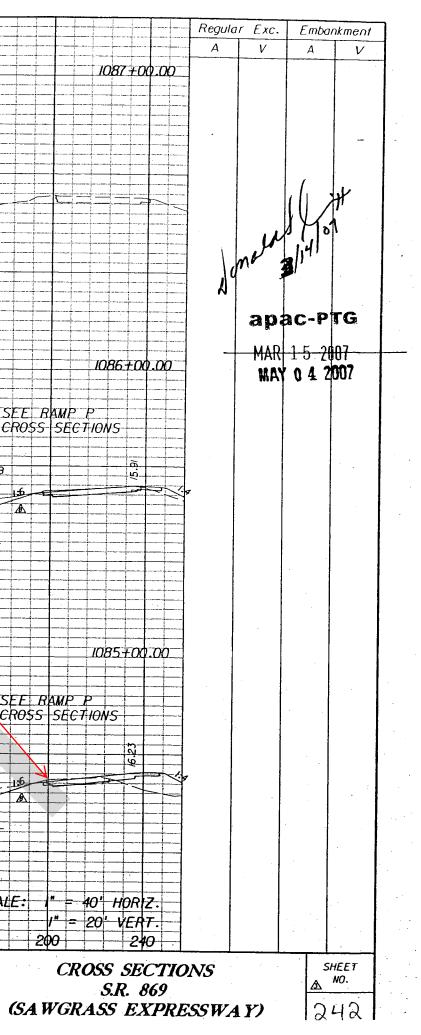








		₽ SURVEY/€ G	ONST. S.R. 869 ISAWGRASS EXPRESSWAY
		27.67	
16 21 SWALE V-21			Swale v is
			512'
32			
		15	
24 		27 40	
16 SWALE V-21			SWALE V-
		Right Swale	
		$\frac{1}{100} \text{ of } \frac{1}{100} \text{ of } \frac{1}{100} = \frac{1}{100} \frac{1}{1$	= <u>30</u> ft [6.1]
Left Swale			threnk and attenuation
$\frac{top of Swale}{bottom of swale} = 20+6*1.5+2*1.5 = 32 ft$			Exist. El 15.0'
$\frac{1}{Avg_{-}} \frac{depth}{depth} \frac{depth}{depth} = 1.5 (treatment and$		= 26540 = 0.61 ac-ft	
<u>c/s area = 0.5(20+32)*1.5=39 sf</u> <u>36</u> volume = <u>39</u> 1000			
		2579	
16 SWALE V-21			Swate v-
		SURVEY/& CON	VST. S.R. 869 (SAWGRASS EXPRESSWAY) SI
240 20 160	120 80	40 40	80 120 160
R E V I S I O N S DATE BY DESCRIPTION 8-30-05 LOO ADDED AUGER BORING LOCATIONS AND SHALE BOTTOM DIMENSIONS. REVISED x-SECTION SHEET MUMBERS. x-SECTION SHEET MUMBERS.	50000	EOR: LON D. OGDEN P.E. LICENSE NUMBER 53200	STATE OF FLORIDA
I		PARSONS	DEPARTMENT OF TRANSPORTATION
8-18-06 OFW A ADJUSTED FRONT SUDPES AND SWALE BOTTONS, REVISED NOISE BARRIER WALL IS ALIGNMENT.		ISON RIVERPLACE BLVD., SUITE 200 JACKSONVILLE, FL 32207	ROAD NO. COUNTY FINANCIAL PROJECT ID



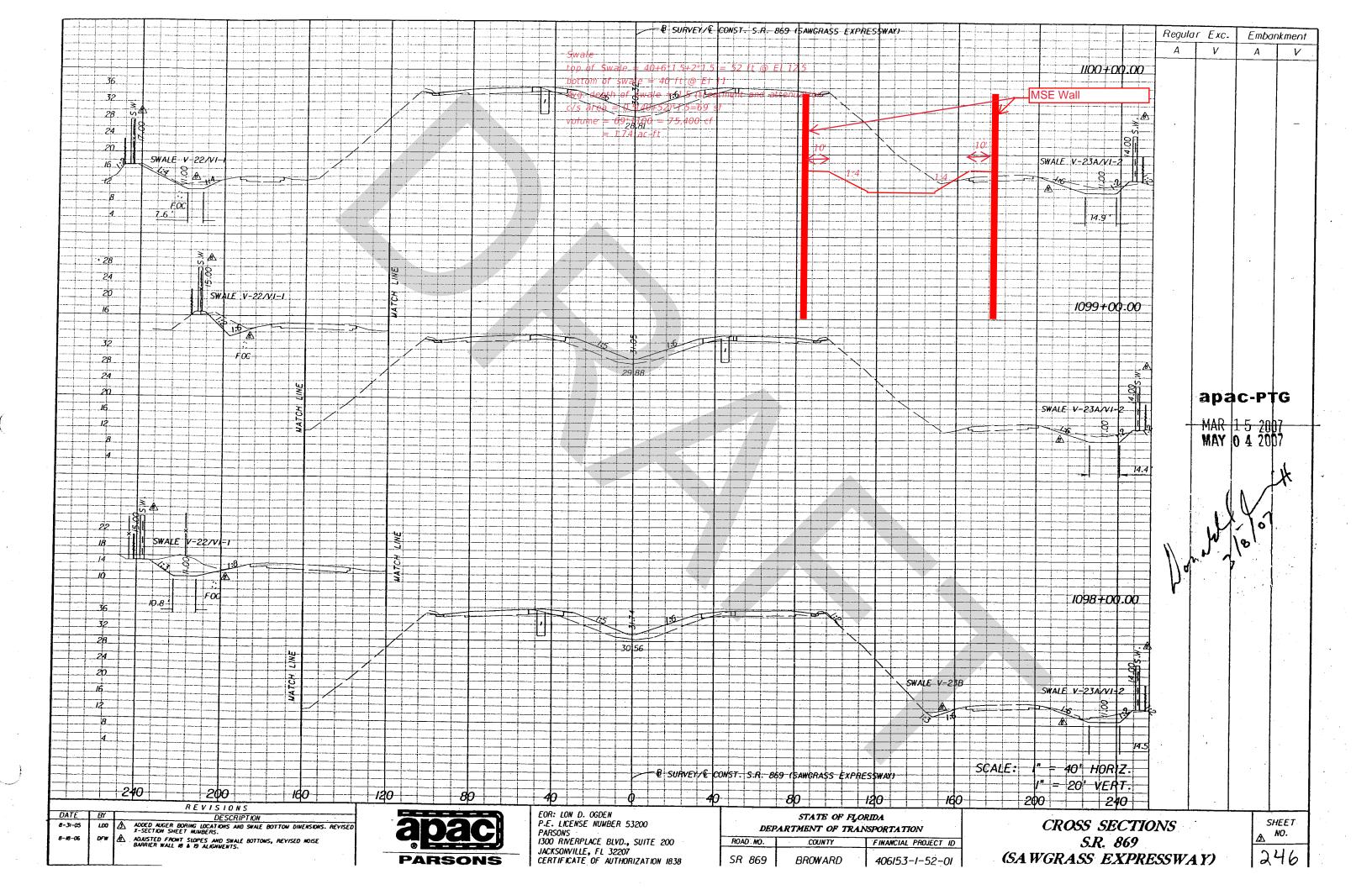


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ETRIC ENGINEERING, INC CONSULTING ENGINEERS VEST PALM BEACH - PANAM

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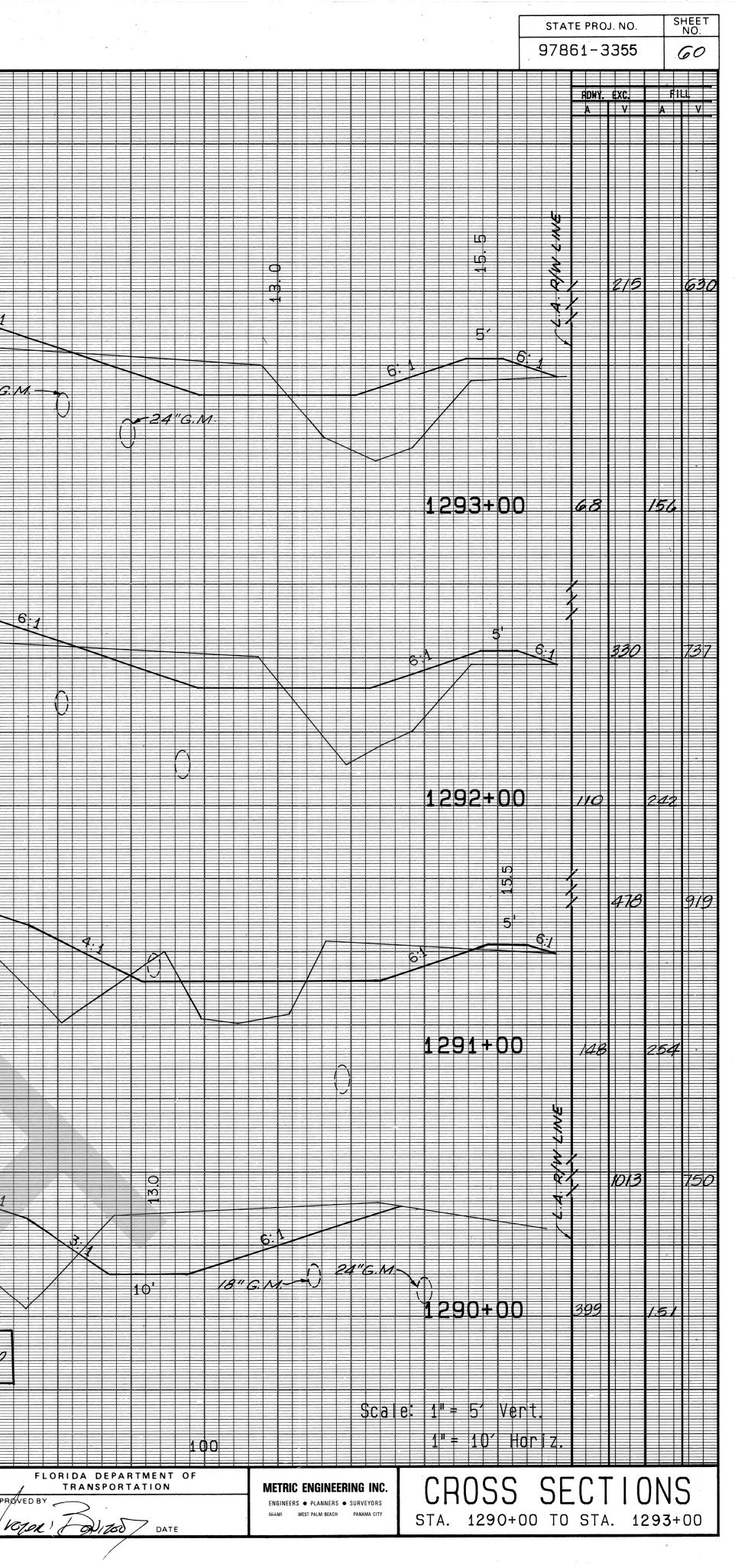
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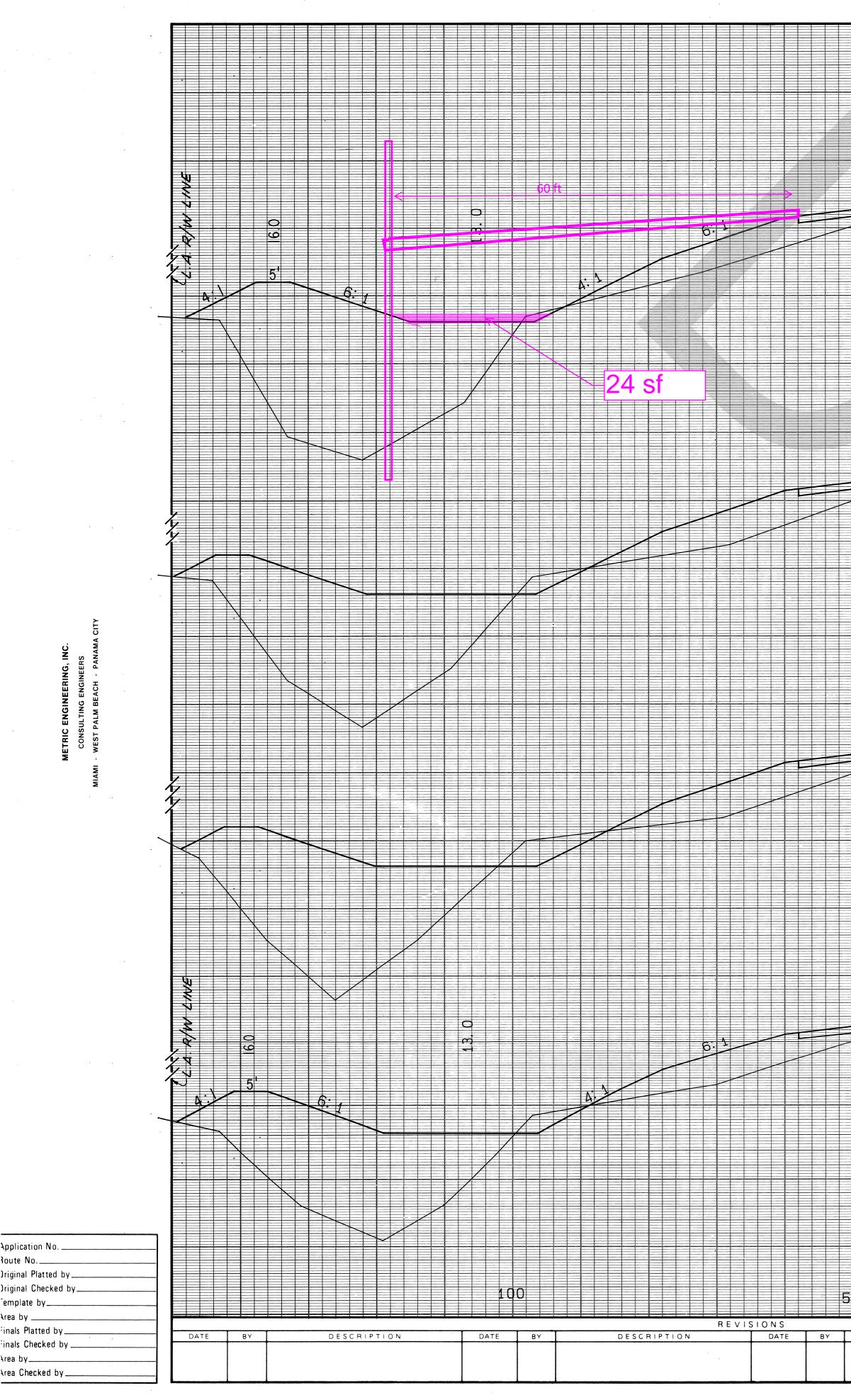
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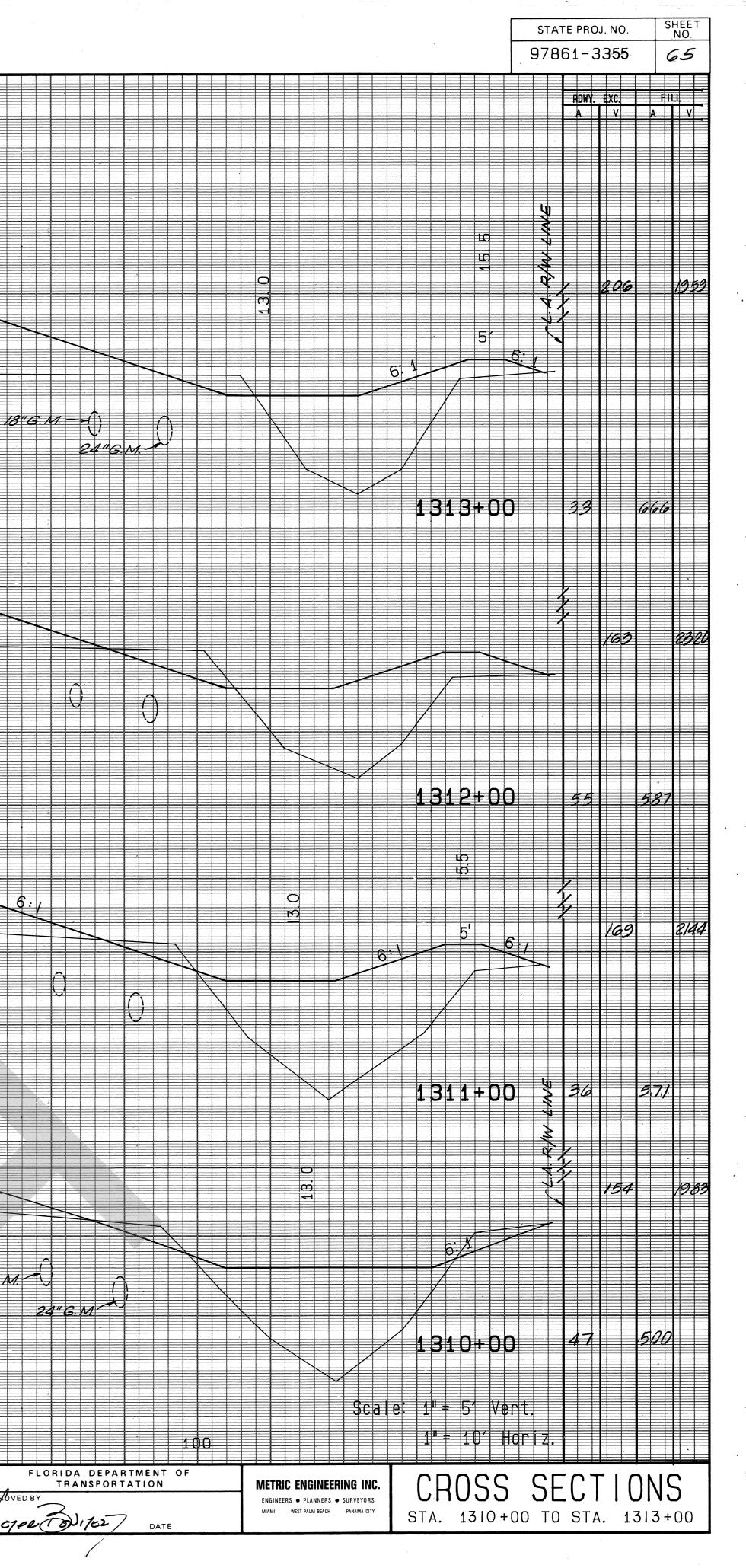
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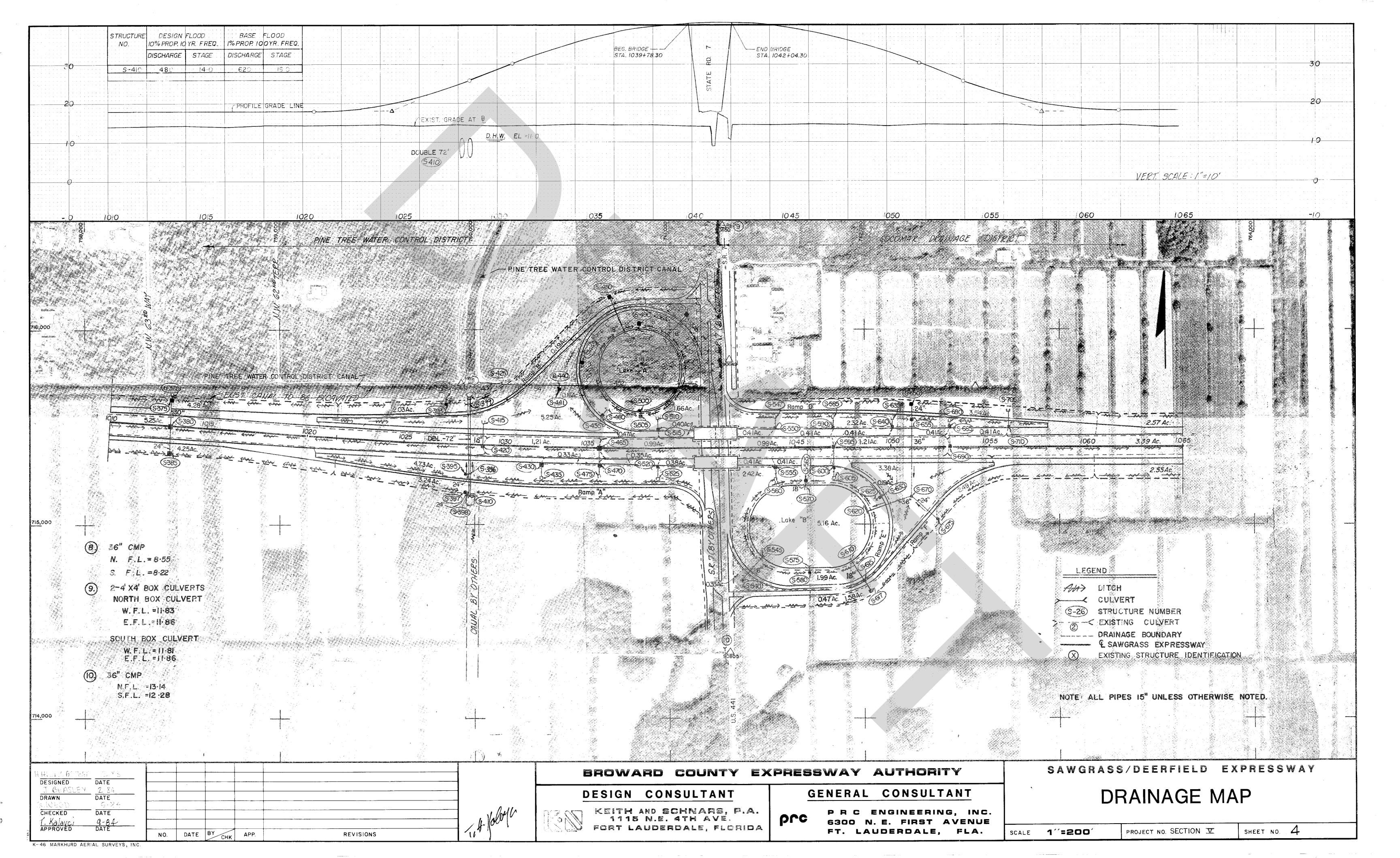


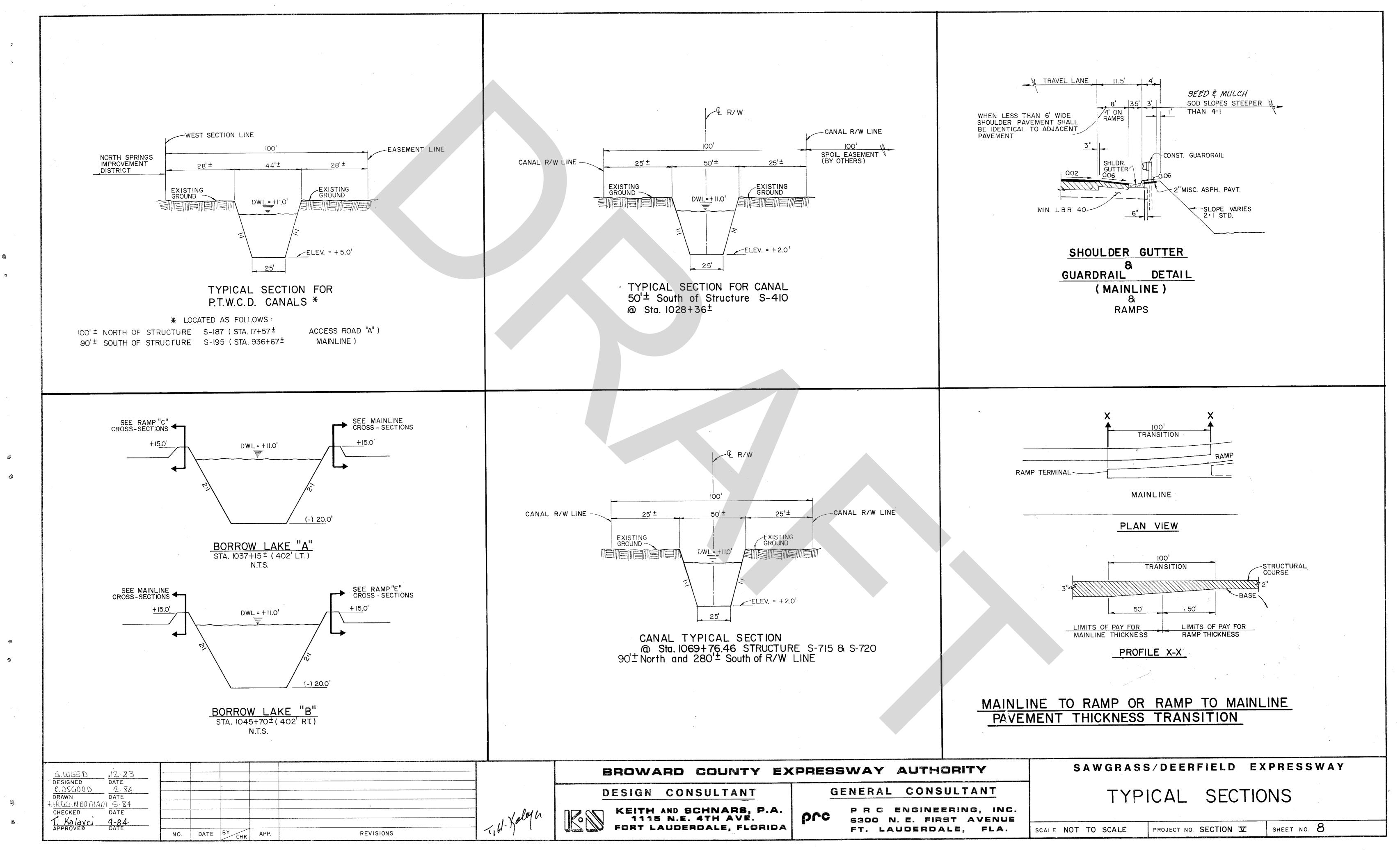
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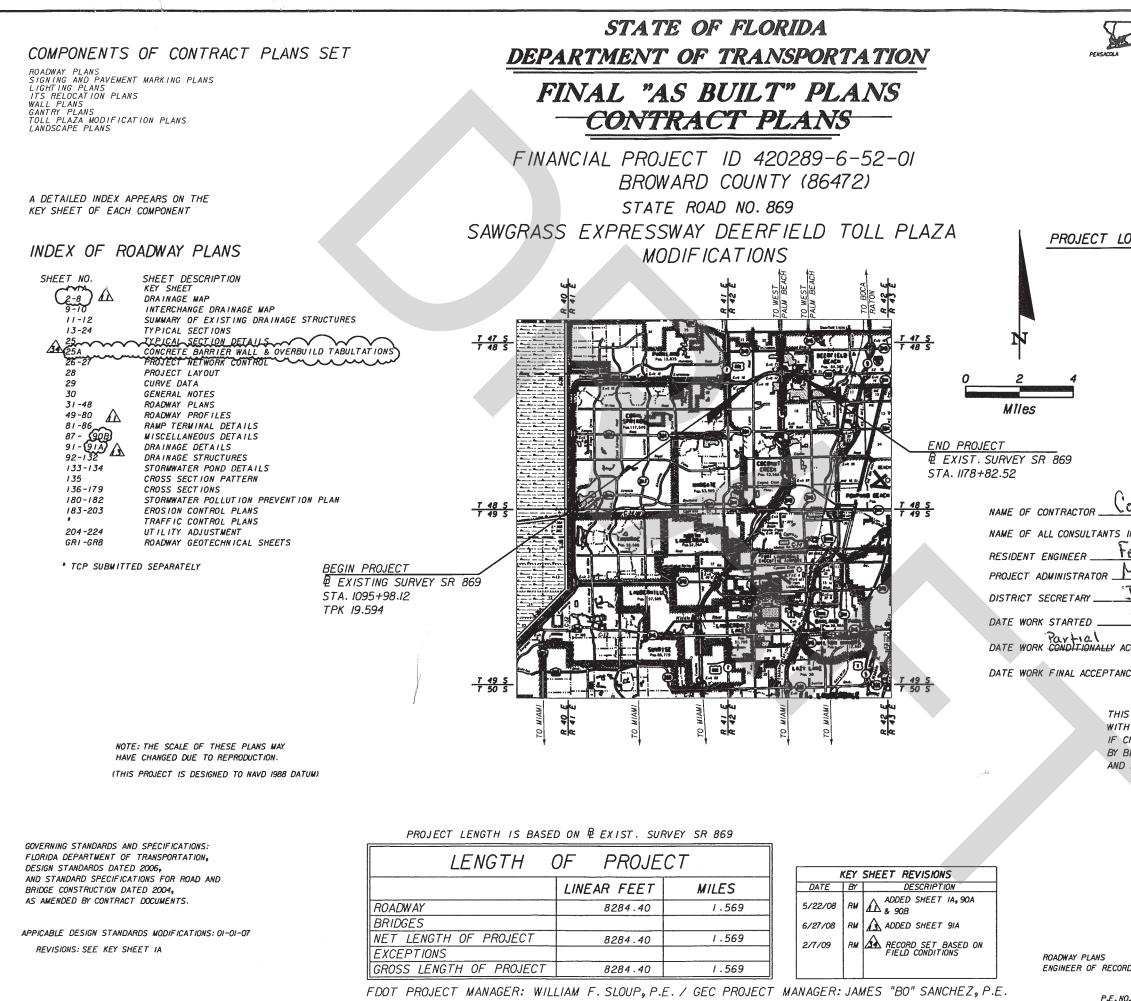
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ROADWAY SHOP DRAWINGS			
TO BE SUBMITTED TO:			
RENATO MARRERO. P.E.			
JACOBS			
3750 N.W. 87th Avenue, Sulte 750			
Miami, FL. 33178 Tel. (305) 718–0599			
PLANS PREPARED BY:			
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JACOBS			
3750 N.W. 87th Avenue, Suite 750			
3750 N.W. 87th Avenue, Suite 750 Miami, Fiorida 33178 U.S.A. Tel. (305) 718_0599_	NG NG 7	9 - C - C - C - C - C - C - C - C - C -	1977
Tel. (305) 718-0599 Contract No. E-8H59			
Vendor No. 43-1621641			
CERTIFICATE OF AUTHORIZATION NO	.6572		
Community Asphalt Co	irp.		
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COMPLIANCE WITH THESE PLANS AS			
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STATE OF FLORIDA DEPARTMENT OF TRANSPORTATION

CONTRACT PLANS

FINANCIAL PROJECT ID 420289-6-52-01 BROWARD COUNTY (86472) STATE ROAD NO. 869

REVISIONS

- M ROADWAY PLANS: 2, 5, 11, 19, 32 THRU 37, 39, 81 THRU 86, 93, 98, 118, 119 128, 132 184, 189, 191, 204, & 205 (REVISED 10/23/07) TC-7, TC-25A, TC-25B AND TC-25C (REVISED 11/14/07)
- ROADWAY PLANS: TC-22A AND TC-103 (REVISED 12/04/07)
- A ROADWAY PLANS: 14, 15, 37, 38, 89, 189, 190, 210, 211, TC-120, TC-156, TC-172 (REVISED 12/14/07) LIGHTING PLANS: L-13 & L-14 (REVISED 12/14/07) GANTRY PLANS: GC-I (REVISED 12/14/07) TOLL PLAZA MODIFICATION PLANS: E-3 (REVISED 12/14/07)
- \land ROADWAY PLANS: TC-62A, TC-63A, TC-64A, TC-145A & TC-146A (REVISED 1/10/08)
- A ROADWAY PLANS: 13, 14, 16 THRU 21 (REVISED 12/19/07) TOLL PLAZA MODIFICATION PLANS: AL-4.5 & AL-6.2 (REVISED 1/15/08)
- 123/08) ROADWAY PLANS: 5, 33, 37, 189 & 210 (REVISED 1/23/08) TOLL PLAZA MODIFICATION PLANS: SL-3, SL-3.2 & SL-3.3 (REVISED 1/23/08)
- A ROADWAY PLANS: TC-I, TC-25A, TC-25B, AND TC-26 THRU TC-29 (REVISED 02/12/08) 30, AND 140 THRU 162 (REVISED 02/15/08)
- ROADWAY PLANS: TC-33A, TC-33B AND TC-100A (REVISED 02/19/08)
- A ROADWAY PLANS: 210, TC-6, TC-17 THRU TC-19, TC-33C, TC-74A THRU TC-74C, TC-130, TC-132 AND TC-139 (REVISED 03/13/08) LIGHTING PLANS: L-5, L-7, L-10 THRU L-14, L-16 THRU L-21 AND L-23 (REVISED 02/21/08)
- TOLL PLAZA MODIFICATION PLANS: E-10 (REVISED 03/10/08) ROADWAY PLANS: TC-IIT THRU TC-I20 (REVISED 05/20/08)
- LIGHTING PLANS: L-5, L-13 & L-15 (REVISED 05/15/08)
- A ROADWAY PLANS: IA, 36, 39, 87, 90A & 90B (REVISED 05/22/08) TOLL PLAZA MODIFICATION PLANS: AL-3.1 (REVISED 07/10/08) TOLL PLAZA MODIFICATION PLANS: PL-1.1& PL-2.1 (REVISED 04/07/08)
- TOLL PLAZA MODIFICATION PLANS: SL-3.2 (REVISED 05/29/08) GANTRY PLANS: GA-0.2 AND GF-2 THRU GF-4 (REVISED 05/29/08)
- A ROADWAY PLANS: 1, 37 & 91A (REVISED 06/27/08) LIGHTING PLANS: L-I3 AND L-I4 (REVISED 06/11/08)
- ROADWAY PLANS: 4, 5, 15, 36, 37, 56, 57, 89, 108-118, 155-159, 188, 189, 209, 210, TC-120, TC-121, TC-127, TC-129, TC-132, TC-139, TC-143, TC-156, TC-157, TC-164, TC-165, TC-172, TC-173 AND TC-179 (REVISED 07/01/08) SIGNING & PAVEMENT MARKINGS: S-II THRU S-I3 (REVISED 07/01/08) LIGHTING PLANS: L-12 THRU L-16 & L-18 (REVISED 07/01/08) TOLL PLAZA MODIFICATION PLANS: E-I7 (REVISED 07/01/08)
- A ROADWAY PLANS: 89, TC-142A, TC-142B & TC-142C (REVISED 08/12/08) TC-4 THRU TC-6, TC-11, TC-12, TC-14 THRU TC-16, TC-18, TC-19, TC-121, TC-129, TC-139 THRU TC-145, TC-157 THRU TC-159, TC-173 THRU TC-175, TC-179 AND TC-180 (REVISED 08/19/08) GANTRY PLANS: GF-I AND GF-5 (REVISED 08/26/08) TOLL PLAZA MODIFICATION PLANS: AL-2.3 (REVISED 07/21/08) TOLL PLAZA MODIFICATION PLANS: SL-3.4 (REVISED 09/29/08) TOLL PLAZA MODIFICATION PLANS: L-31 (REVISED 08/21/08) TOLL PLAZA MODIFICATION PLANS: SL-4.5 (REVISED 08/28/08) GANTRY PLANS: GE-2 THRU GE-4 (REVISED 09/29/08) TOLL PLAZA MODIFICATION PLANS: SL-2.7 (REVISED 09/29/08)
- ROADWAY PLANS: TC-129 & TC-130 (REVISED 09/09/08)
- ▲ ROADWAY PLANS: TC-129 & TC-130 (REVISED 09/16/08) ROADWAY PLANS: 38, 89, 112, & 157 (REVISED 09/18/08)

SIGNING & PAVEMENT MARKING PLANS: S-55 (REVISED 09/17/08) SIGNING & PAVEMENT MARKING PLANS: 5-22, 5-23, 5-27, 5-30 THRU 5-32, S-35, S-70, S-72 THRU S-77 (REVISED 10/06/08) SIGNING & PAVEMENT MARKING PLANS: S-84 (SS-3) (REVISED 11/06/08) TOLL PLAZA MODIFICATION PLANS: SL-3.5 (REVISED 09/24/08) TOLL PLAZA MODIFICATION PLANS: AL-6.2 (REVISED 09/30/08) TOLL PLAZA MODIFICATION PLANS: SL-3.6 THRU SL-3.8 (REVISED 11/06/08)

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- ROADWAY PLANS: TC-15, TC-142A, TC-142B, TC-142C, TC-143 AND TC-144 (REVISED 10/14/08) TOLL PLAZA MODIFICATION PLANS: SL-3.5 & SL-3.9 (REVISED 10/15/08)
- TOLL PLAZA MODIFICATION PLANS: SL-2.1 THRU SL-2.3 (REVISED 10/27/08) TOLL PLAZA MODIFICATION PLANS: SL-3.1 (REVISED 10/28/08)

R VOID

- ROADWAY PLANS: 40, 87 & TC-25D (REVISED 11/07/08) LIGHTING PLANS: L-12 THRU L-16 & L-18 (REVISED 11/03/08) TOLL PLAZA MODIFICATION PLANS: E-16 THRU E-21 (REVISED 11/03/08)
- ROADWAY PLANS: TC-16 & TC-153 THRU TC-159 (REVISED 11/11/08)
- ROADWAY PLANS: 43 AND 168 THRU 170 (REVISED 11/15/08) SIGNING & PAVEMENT MARKINGS PLANS: S-87 (SS-6) (REVISED 11/18/08)
- SIGNING & PAVEMENT MARKINGS PLANS: 5-87 (SS-6) & S-87A (SS-6A) (REVISED 12/8/08) TOLL PLAZA MODIFICATION PLANS: SL-3.1, SL-4 THRU SL-4.4 & SW-1 (REVISED 12/8/08)

A VOID

- GANTRY PLANS (URS): GB-13, GB-16 & GB-31 (REVISED 12/24/08)
- TOLL PLAZA MODIFICATION PLANS: AL-6.1 & AL-6.2 (REVISED 01/21/09)
- 👧 signing & pavement markings plans: 5-86 (SS-5)& S-87 (SS-6)(REVISED 01/06/09) TOLL PLAZA MODIFICATION PLANS: TCW-5 (97) (REVISED 01/08/09)
- ROADWAY PLANS: TC-16 & TC-157 THRU TC-159 (REVISED 01/08/09)
- A ROADWAY PLANS: 39, 40, 160 & 161 (REVISED 01/20/09) GANTRY PLANS: SW-7 THRU SW-9 (REVISED 01/20/09)
- ROADWAY PLANS: 30 (REVISED 2/2/09) GANTRY PLANS: GA-0.2 (REVISED 2/2/09)

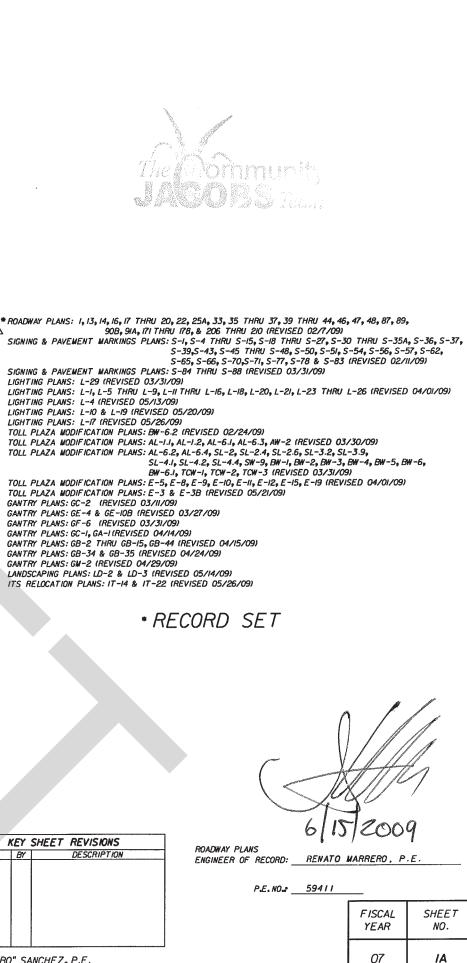
KEY SHEET REVISIONS DATE BY DESCRIPTION

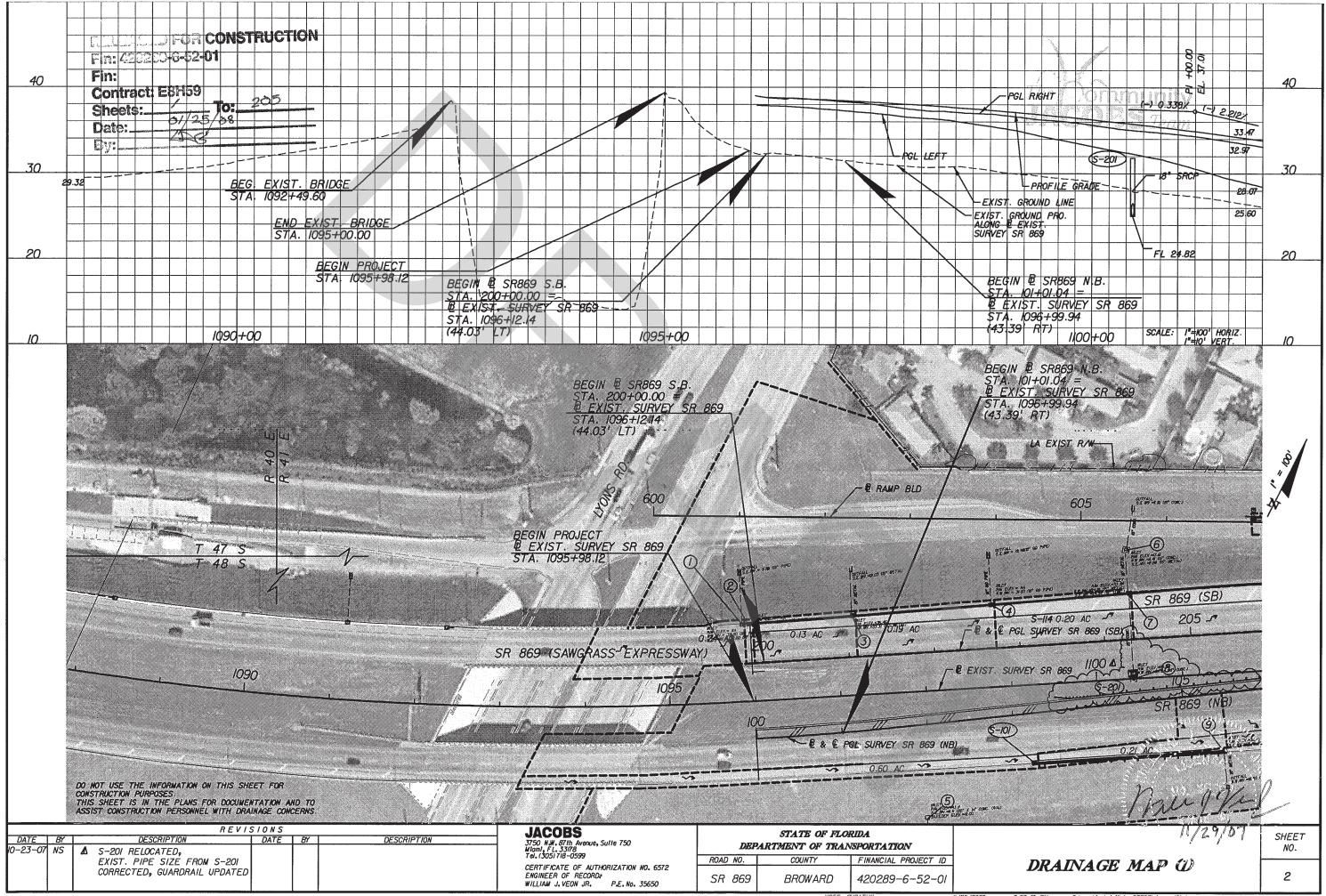
FDOT PROJECT MANAGER: WILLIAM F. SLOUP, P.E. / GEC PROJECT MANAGER: JAMES "BO" SANCHEZ, P.E.

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LIGHTING PLANS: L-29 (REVISED 03/31/09) LIGHTING PLANS: L-4 (REVISED 05/13/09) LIGHTING PLANS: L-10 & L-19 (REVISED 05/20/09) LIGHTING PLANS: L-IT (REVISED 05/26/09)

GANTRY PLANS: GC-2 (REVISED 03/11/09) GANTRY PLANS: GE-4 & GE-IOB (REVISED 03/27/09) GANTRY PLANS: GF-6 (REVISED 03/31/09) GANTRY PLANS: GC-I, GA-I (REVISED 04/14/09) GANTRY PLANS: GB-2 THRU GB-15, GB-44 (REVISED 04/15/09) GANTRY PLANS: GB-34 & GB-35 (REVISED 04/24/09) GANTRY PLANS: GM-2 (REVISED 04/29/09) LANDSCAPING PLANS: LD-2 & LD-3 (REVISED 05/14/09) ITS RELOCATION PLANS: IT-14 & IT-22 (REVISED 05/26/09)





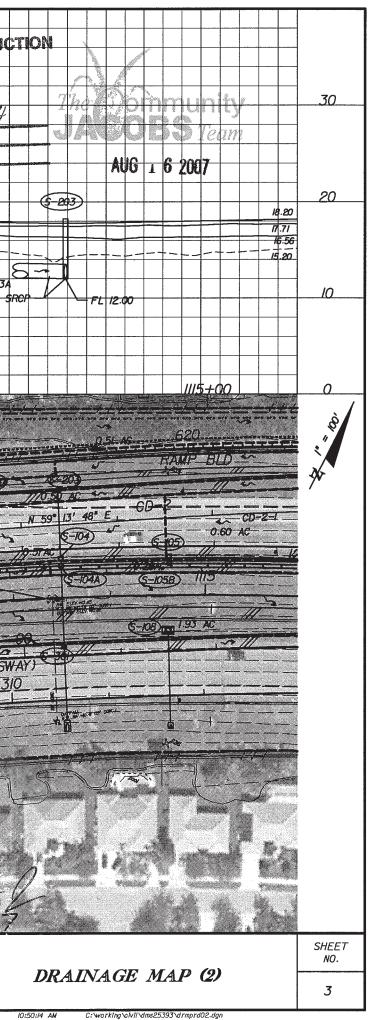
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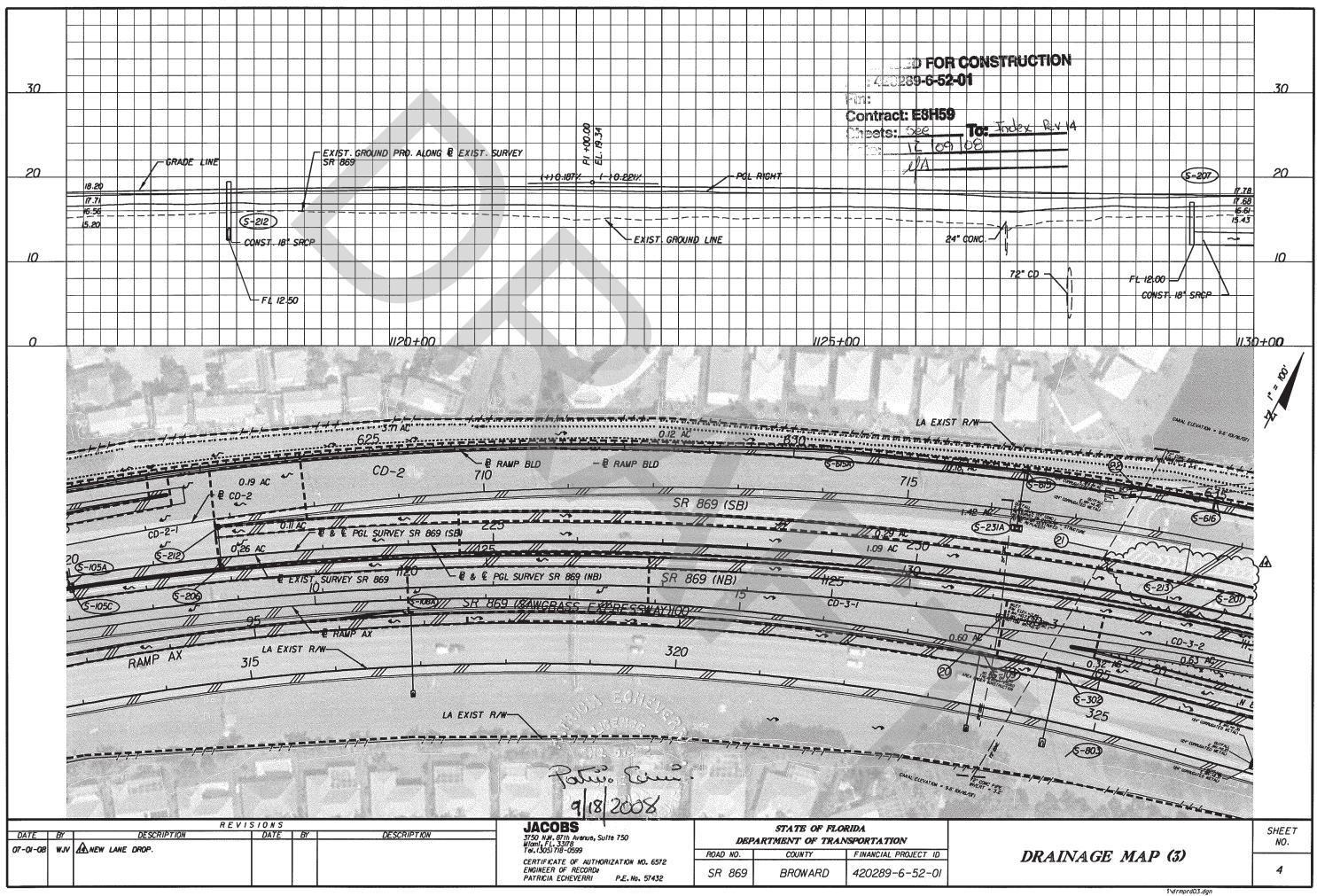
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				CERTIFICATE OF AUTHORIZATION NO. 6572	ROAD NO. COUNTY	FINANCIAL PROJECT ID	

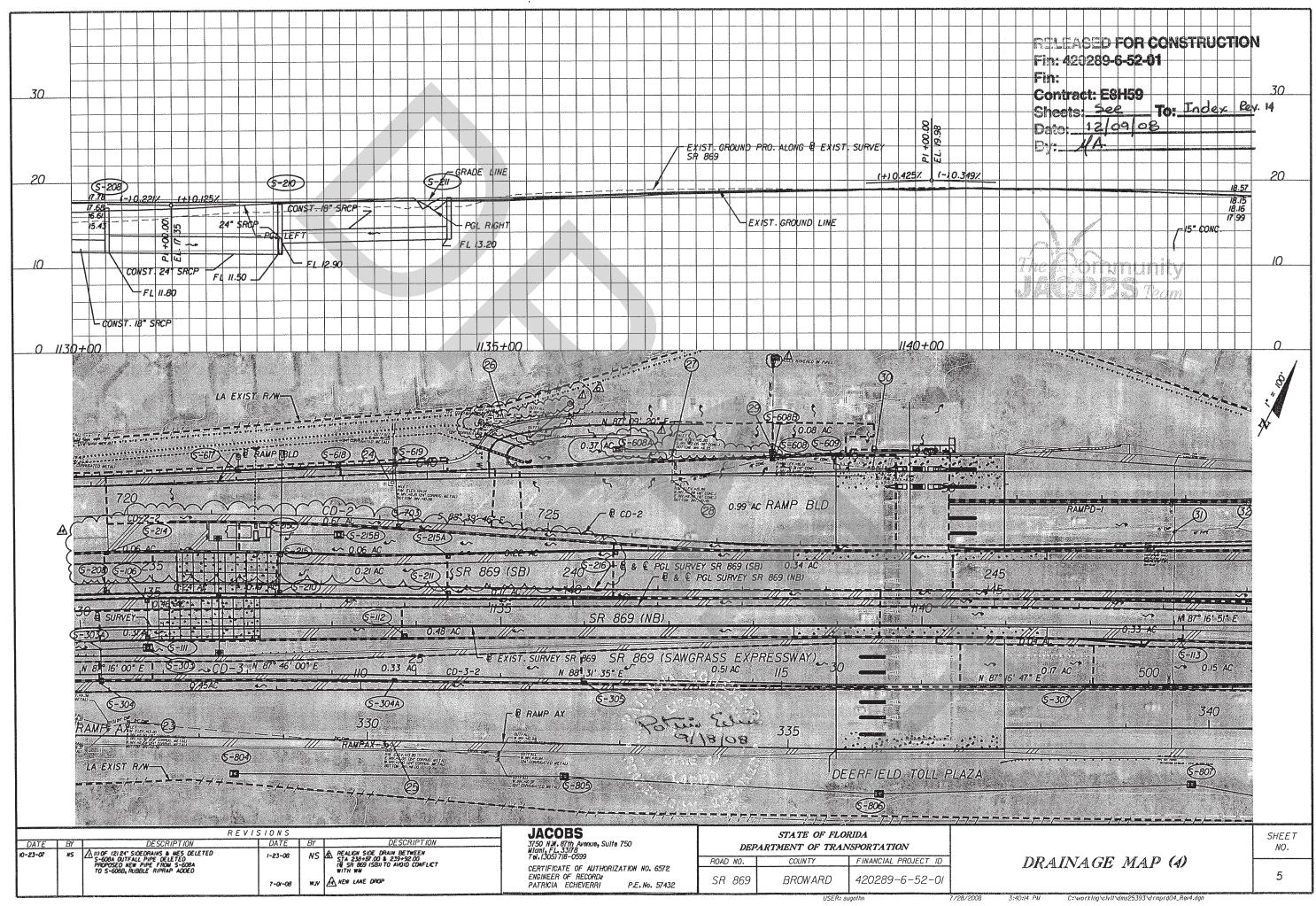
CERTIFICATE OF AUTHORIZATION NO. 6572 ENGINEER OF RECORD: WILLIAM J.VEON JR. P.E.NO. 35650 SR 869 BROWARD

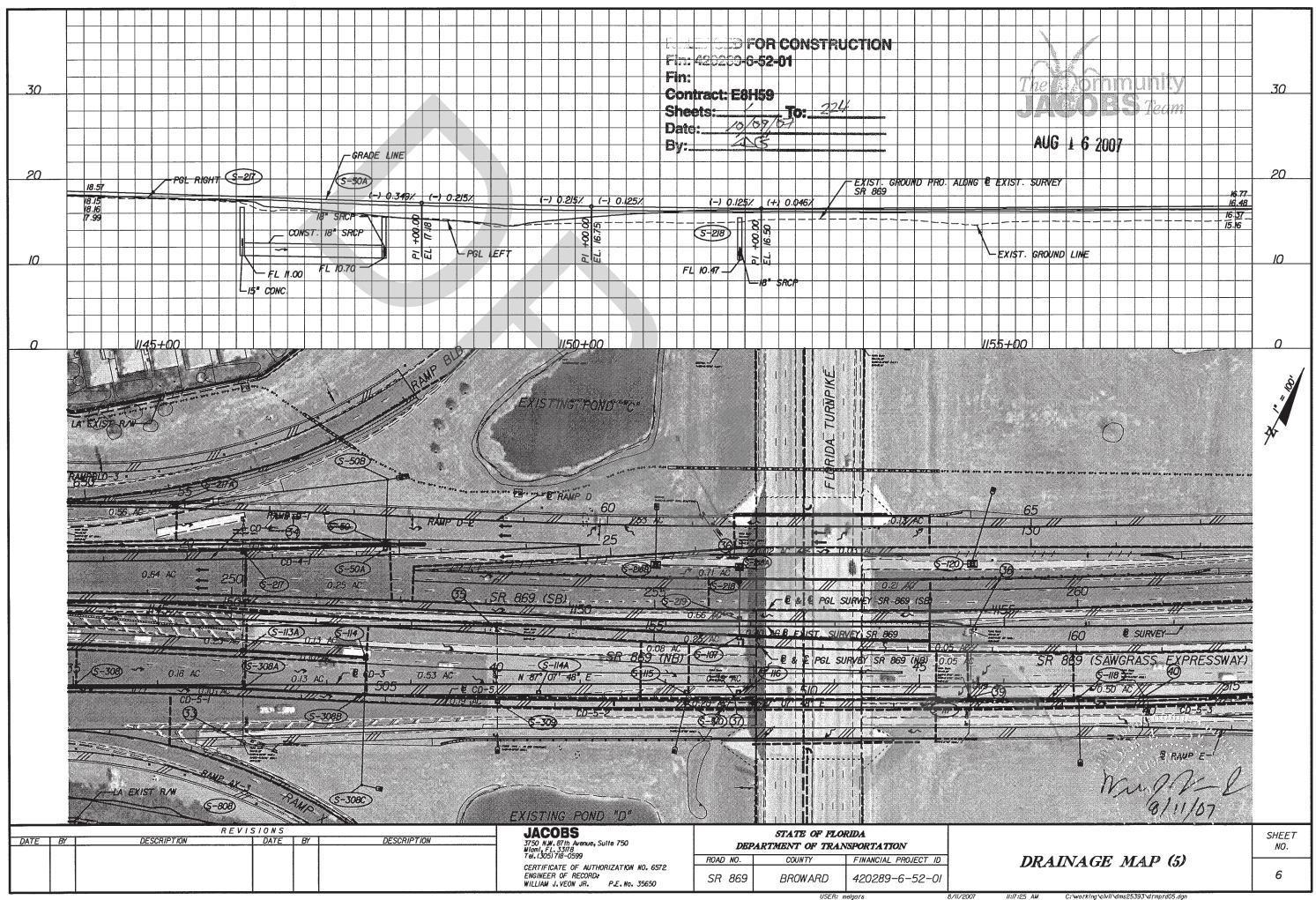
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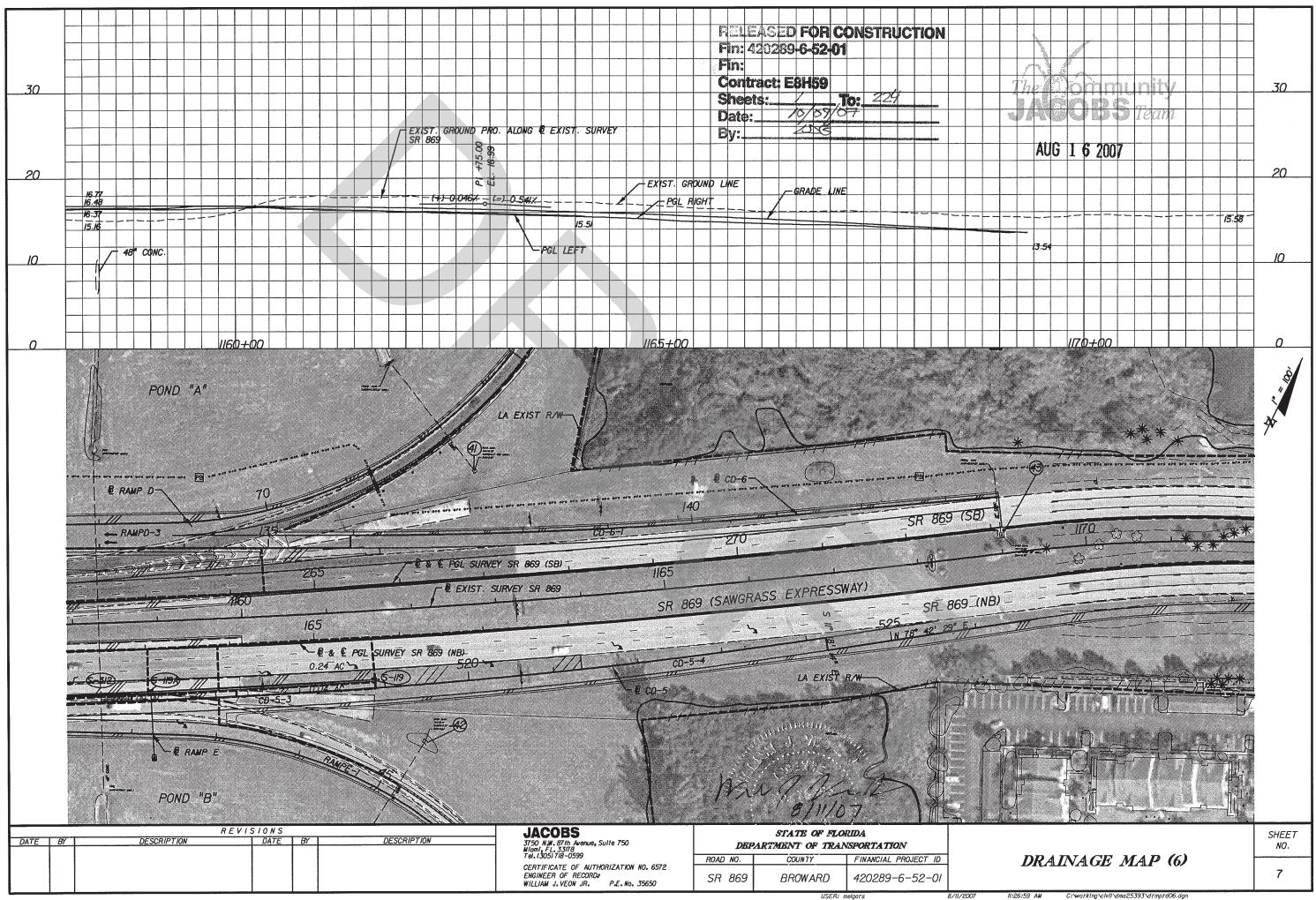
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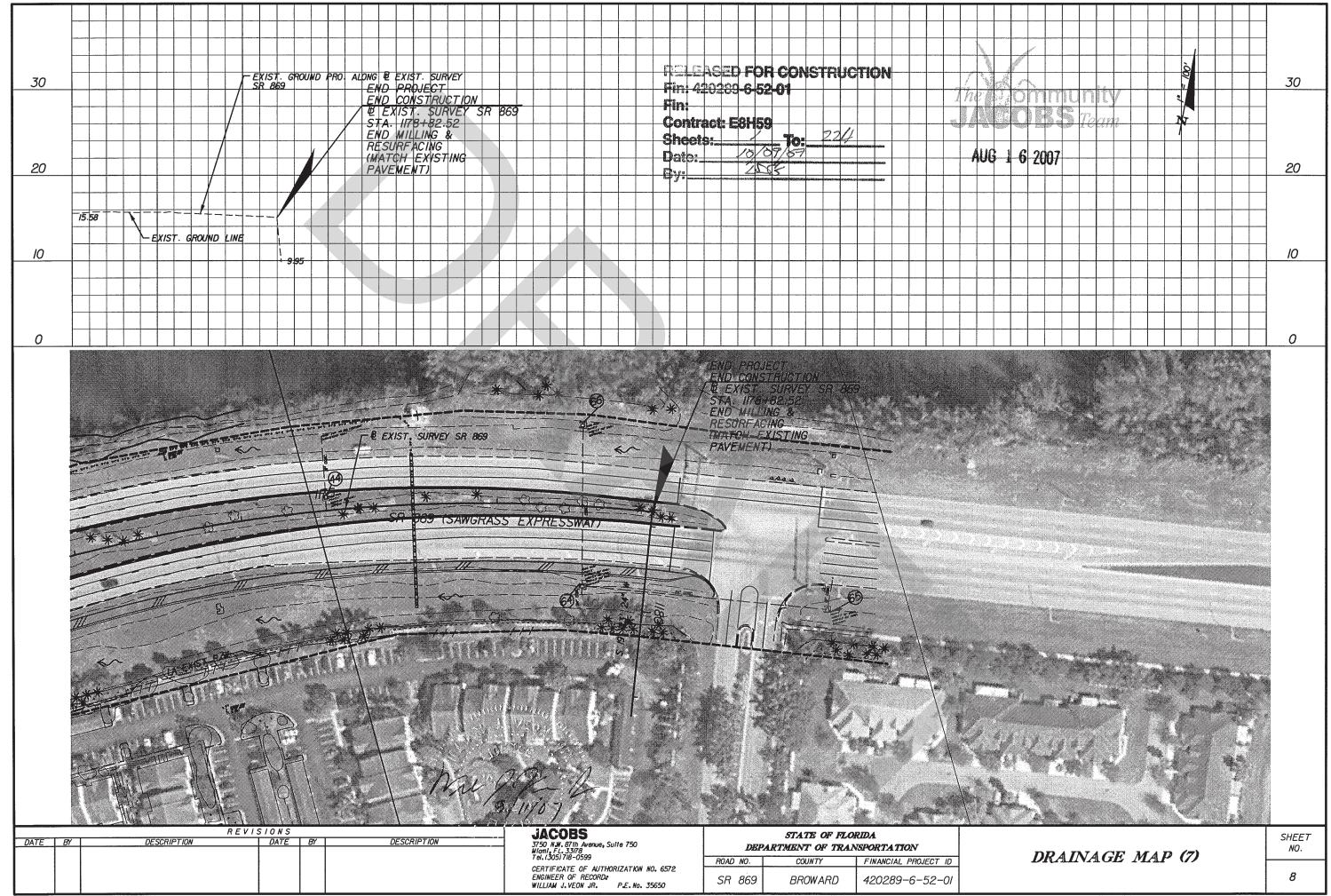






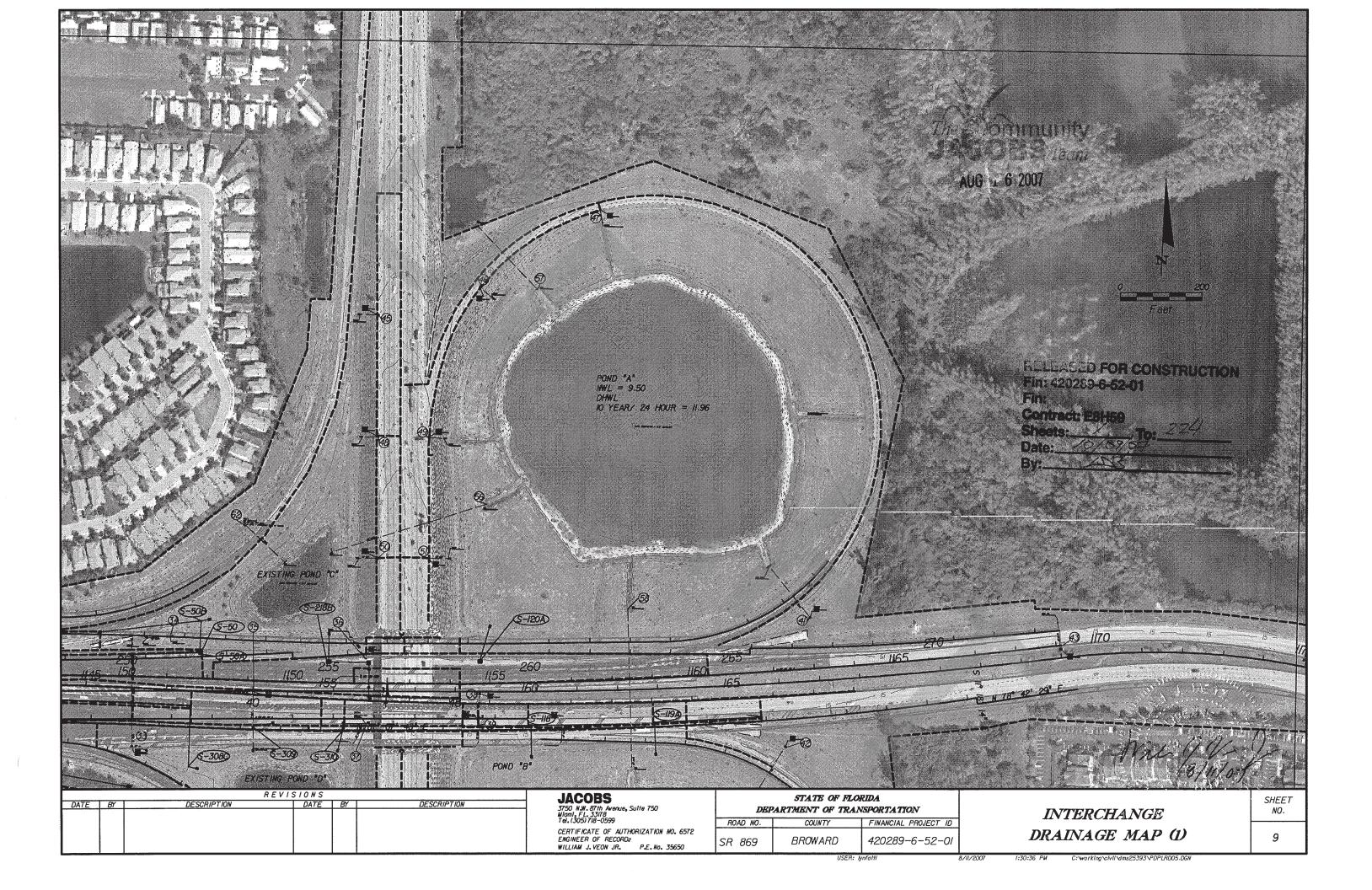


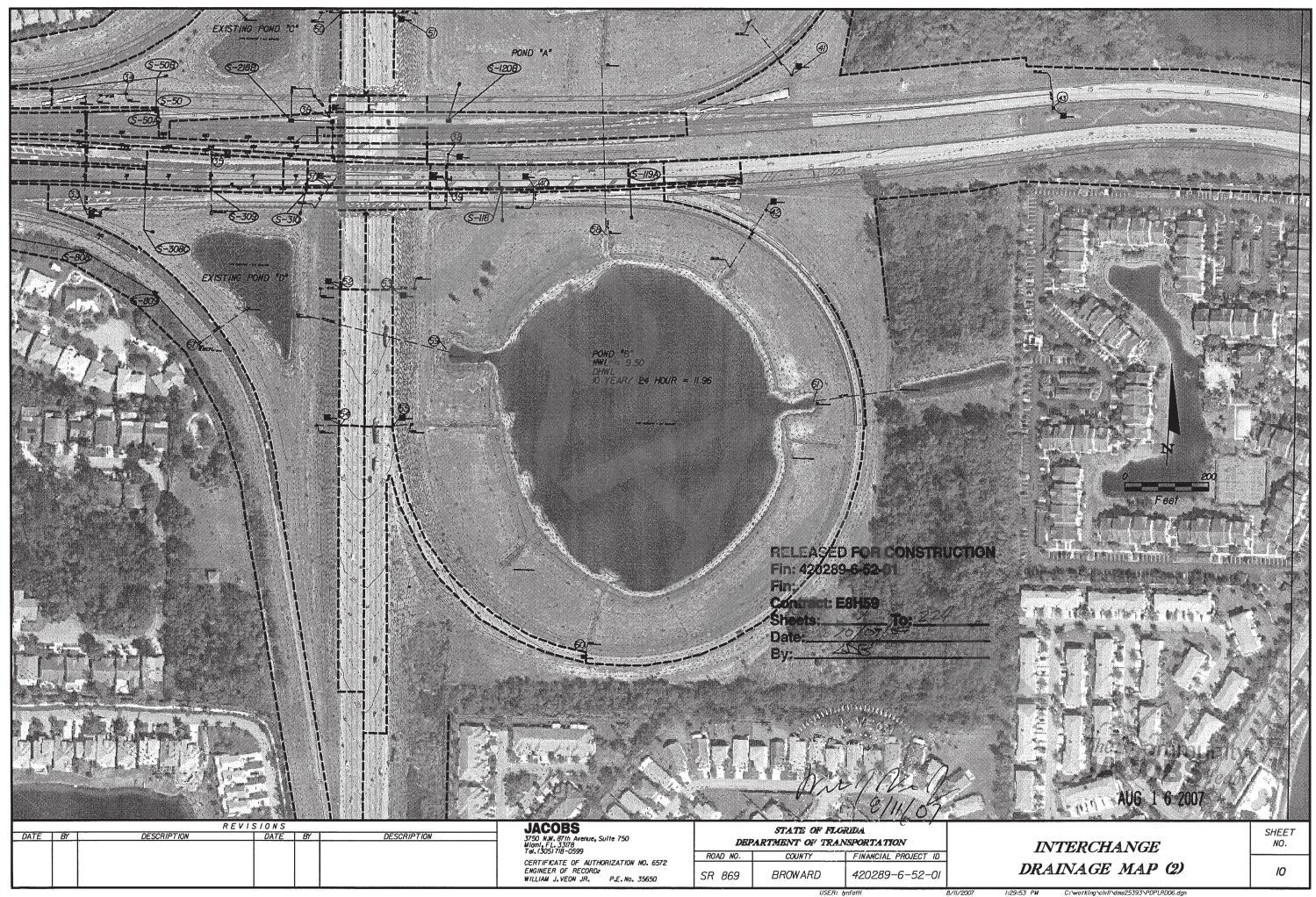




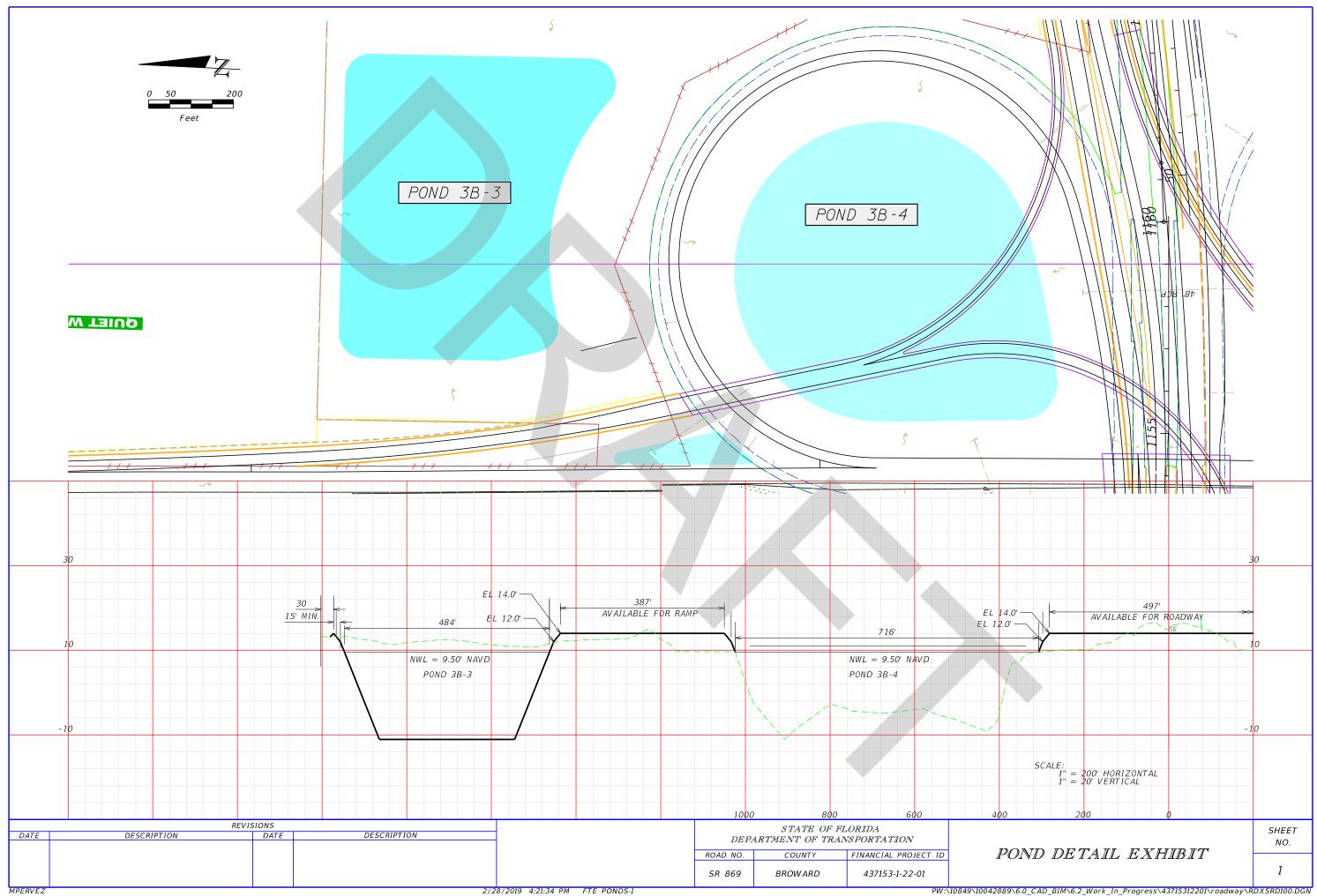
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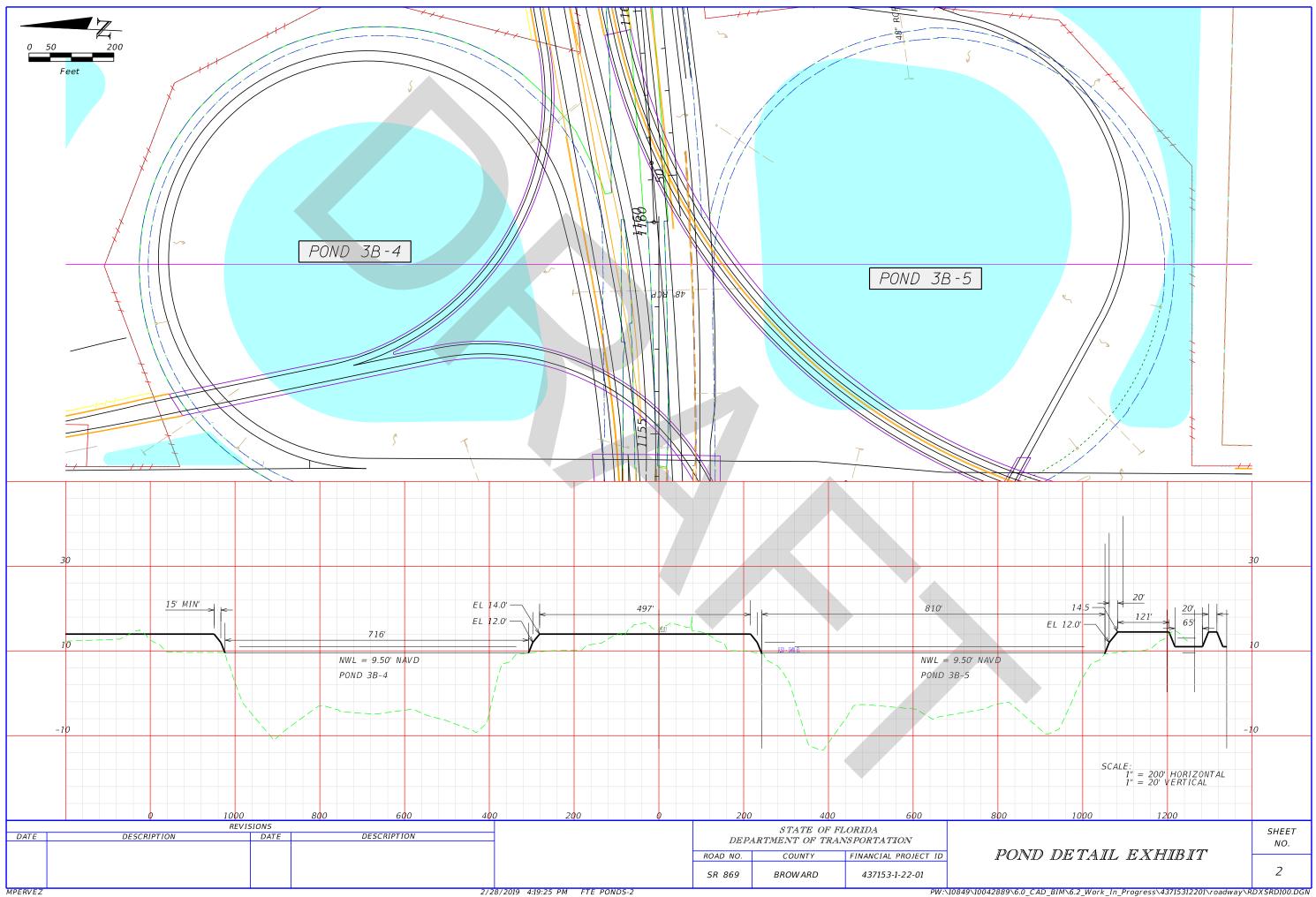
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CONCEPTUAL POND DETAIL SHEET POND 3B-3, 3B-4 and 3B-5



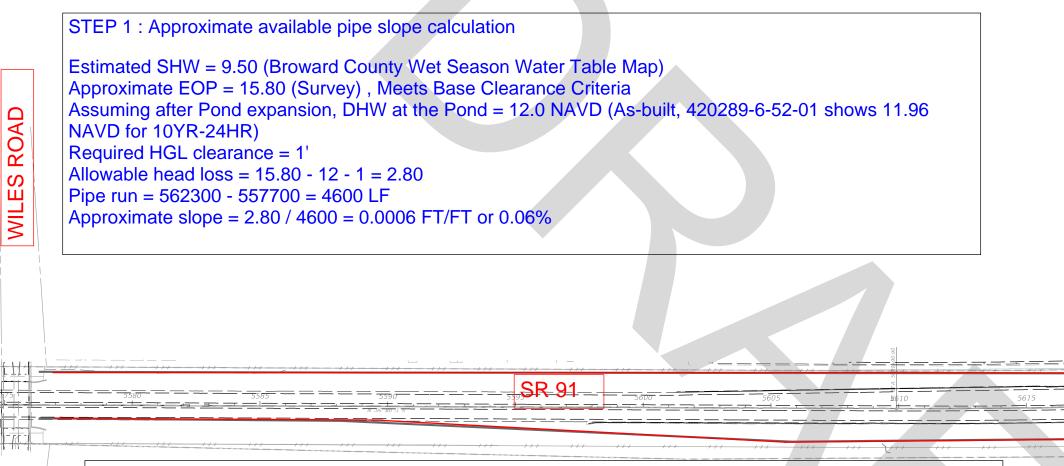


HYDRAULIC EVALUATION OF CONCEPTUAL STORMSEWER SYSTEM TRUNKLINE DISCHARGING TO WET DETENTION PONDS

	HYDRAULIC EVALUATION OF STORM SEWER SYSTEM														
Basin and Location	Storm Sewer Trunkline Run Len				Hydraulic Fall		Min. Hydraulic Clearance Required for farthest inlet	Minimum Required EOP elevation	Approximate existing EOP elevation	Wet Season Water Table		Base	Remarks		
	From Station	To Station	ft-NAVD	%	ft-NAVD	ft-NAVD	ft-NAVD	ft-NAVD	ft-NAVD	ft-NAVD	ft	ft			
3B, FTE-South	557700	562300	4600	0.07%	3.22	12	1	. 16.22	15.80	9.5	1.5		EOP to be raised by 0.42 ft (5 inch)		
3B, FTE-North	565900	568000	2100	0.10%	2.10	12	1	. 15.10	15.80	9.5	1.5	4.10			
4, SR 869 East of Interchange	116500	120800	4300	0.07%	3.01	12		. 16.01	13.20	9.5	1.5		EOP to be raised by 2.81 ft		
3B, SR 869 West of Interchange	111300	115000	3700	0.07%	2.59	12		. 15.59	16.00	9.5	1.5	4.59			

CONCEPTUAL PIPE SIZING CALCULATION ALONG TURNPIKE

STORMSEWER PIPE SIZE CALCULATION (CONCEPTUAL)

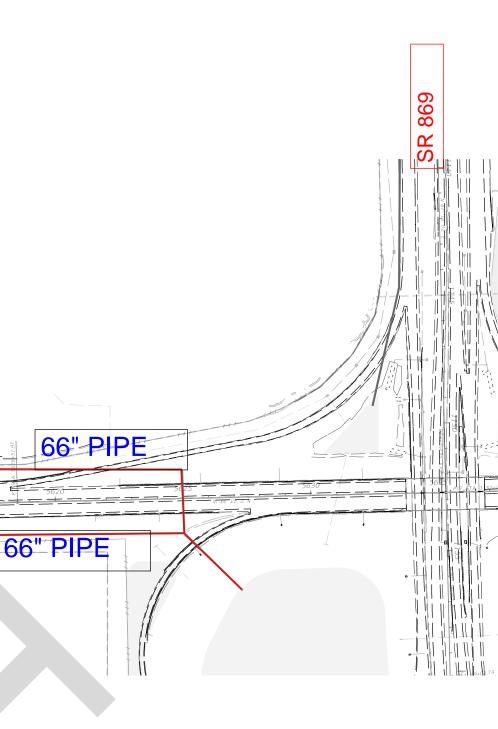


STEP 2: Approximate pipe size based on estimated Q

Basin Length = 563500 - 557500 = 6000 LFImpervious Width = 277 FTArea = $6000 \times 277 / 43560 = 38.15 \text{ Ac}$ Half of the roadway = 38.15 / 2 = 19.08 AcEstimated Tc = L / $60V = 5000 / (60 \times 2.5 \text{ fps}) = 33.33 \text{ min}$ Total Tc = 10 + 33.33 = 43.33 minFrom IDF curve of zone 10, 10 Year, i = 4.4 in/hrDischarge Q = $0.95 \times 4.4 \times 19.08 = 80 \text{ cfs}$ From Hydraulic Calculator, for pipe flowing full with Q = 80 cfs, required friction slope for a 66" pipe is approximately 0.05 %

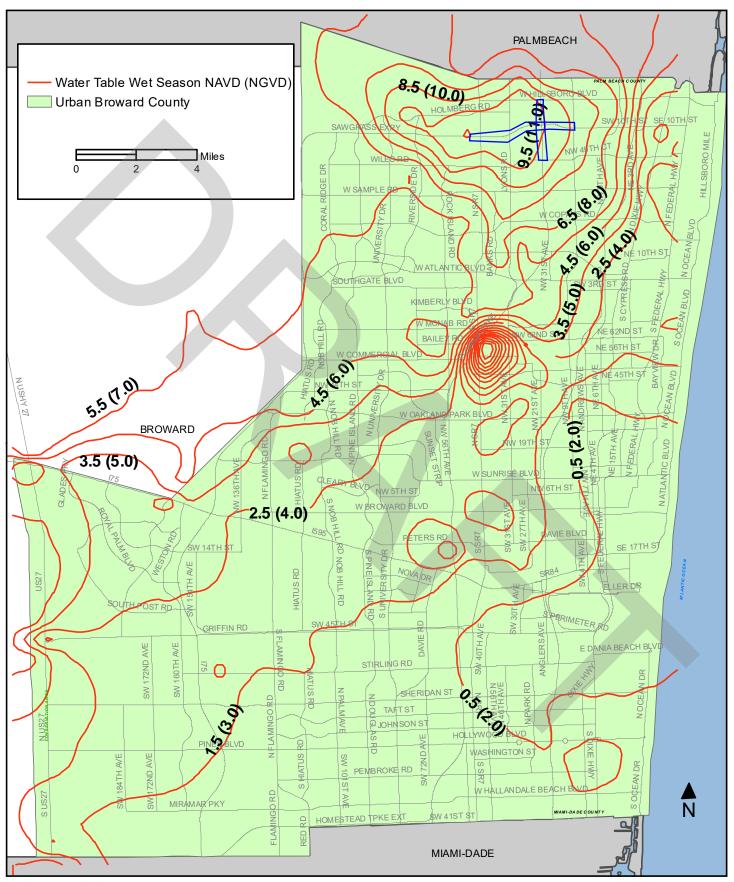
CONCLUSION:

Comparing the results from STEP 1 and STEP 2, approximately maximum 66" pipe will be required to carry runoff from each side of Turnpike.



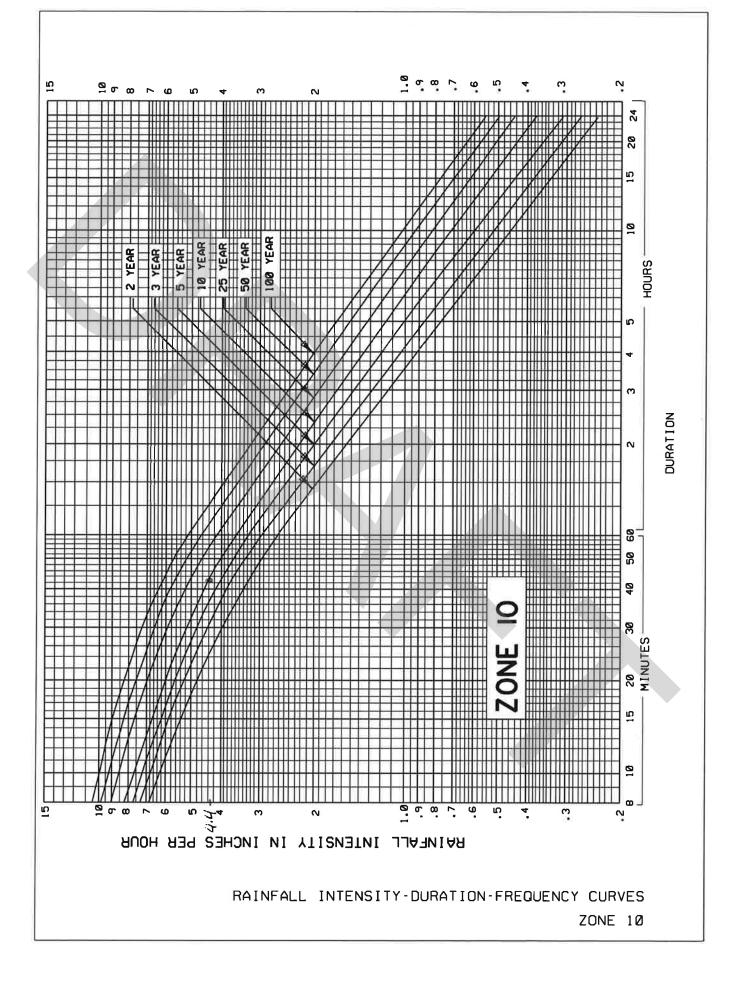


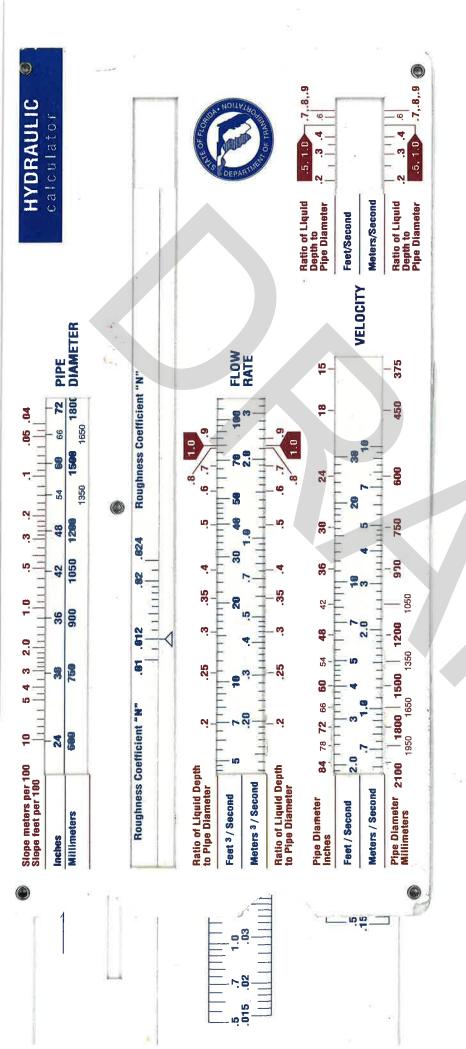
WATER TABLE MAP - AVERAGE WET SEASON



Division Name: Planning and Environmental Regulation Department Name: Environmental Protection and Growth Management This map is for conceptual purposes only and should not be used for legal boundary determinations. Elevation converted from NGDV to NAVD using the FEMA approved conversion factor for Broward County of (-) 1.5

Drainage Manual IDF Curves







Draft Pond Siting Report

APPENDIX H

Design Aids



NOAA Atlas 14, Volume 9, Version 2 Location name: Pompano Beach, Florida, USA* Latitude: 26.3002°, Longitude: -80.1947° Elevation: 16 ft** * source: ESRI Maps ** source: USGS



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-ba	PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) ¹ Average recurrence interval (years) Duration 1 2 5 10 25 50 100 200 500 1000														
Duration				Average	recurrence	interval (y	ears)								
Duration	1	2	5	10	25	50	100	200	500	1000					
5-min	0.563 (0.444-0.717)	0.645 (0.508-0.822)	0.779 (0.612-0.996)	0.891 (0.696-1.15)	1.05 (0.793-1.39)	1.17 (0.866-1.57)	1.29 (0.926-1.79)	1.41 (0.977-2.02)	1.57 (1.05-2.32)	1.69 (1.11-2.55)					
10-min	0.824 (0.650-1.05)	0.944 (0.744-1.20)	1.14 (0.896-1.46)	1.31 (1.02-1.68)	1.53 (1.16-2.03)	1.71 (1.27-2.31)	1.88 (1.36-2.62)	2.06 (1.43-2.95)	2.30 (1.54-3.40)	2.48 (1.63-3.74)					
15-min	1.00 (0.793-1.28)	1.15 (0.907-1.47)	1.39 (1.09-1.78)	1.59 (1.24-2.04)	1.87 (1.42-2.48)	2.08 (1.55-2.81)	2.30 (1.65-3.19)	2.51 (1.75-3.60)	2.80 (1.88-4.15)	3.02 (1.98-4.56)					
30-min	1.58 (1.25-2.01)	1.82 (1.43-2.32)	2.21 (1.73-2.82)	2.53 (1.98-3.25)	2.98 (2.26-3.96)	3.32 (2.47-4.49)	3.67 (2.64-5.10)	4.02 (2.79-5.76)	4.49 (3.01-6.64)	4.84 (3.17-7.30)					
60-min	2.16 (1.70-2.75)	2.47 (1.94-3.14)	2.99 (2.35-3.82)	3.45 (2.69-4.43)	4.11 (3.13-5.50)	4.64 (3.46-6.31)	5.20 (3.76-7.27)	5.79 (4.03-8.34)	6.61 (4.44-9.82)	7.25 (4.75-10.9)					
2-hr	2.74 (2.18-3.47)	3.12 (2.47-3.94)	3.77 (2.98-4.79)	4.36 (3.43-5.57)	5.24 (4.03-7.00)	5.96 (4.48-8.08)	6.73 (4.91-9.38)	7.56 (5.31-10.9)	8.73 (5.92-12.9)	9.66 (6.38-14.5)					
3-hr	3.07 (2.45-3.87)	3.48 (2.77-4.39)	4.24 (3.36-5.36)	4.93 (3.89-6.27)	6.00 (4.65-8.03)	6.91 (5.23-9.37)	7.89 (5.79-11.0)	8.97 (6.34-12.9)	10.5 (7.17-15.5)	11.8 (7.79-17.5)					
6-hr	3.58 (2.87-4.47)	4.12 (3.30-5.15)	5.12 (4.09-6.43)	6.07 (4.83-7.67)	7.56 (5.92-10.1)	8.85 (6.75-12.0)	10.3 (7.59-14.2)	11.8 (8.41-16.9)	14.0 (9.66-20.7)	15.9 (10.6-23.5)					
12-hr	4.02 (3.25-5.00)	4.77 (3.84-5.93)	6.15 (4.94-7.67)	7.44 (5.95-9.33)	9.44 (7.44-12.6)	11.2 (8.57-15.0)	13.0 (9.70-18.0)	15.1 (10.8-21.4)	18.0 (12.5-26.4)	20.4 (13.8-30.1)					
24-hr	4.58 (3.72-5.65)	5.49 (4.45-6.78)	7.16 (5.79-8.87)	8.72 (7.02-10.9)	11 1 (8.84-14.7)	13.2 (10.2-17.6)	15.5 (11.6-21.2)	17.9 (13.0-25.3)	21.5 (15.0-31.2)	24.4 (16.5-35.7)					
2-day	5.40 (4.41-6.61)	6.33 (5.17-7.76)	8.07 (6.57-9.93)	9.73 (7.88-12.0)	12.3 (9.85-16.2)	14.6 (11.3-19.3)	17.0 (12.9-23.2)	19.7 (14.4-27.7)	23.6 (16.6-34.1)	26.8 (18.3-39.0)					
3-day	5.99 (4.92-7.31)	6.92 (5.68-8.46)	8.68 (7.09-10.6)	10.3 (8.41-12.7)	13.0 (10.4-16.9)	15.2 (11.9-20.1)	17.7 (13.4-24.0)	20.5 (15.0-28.6)	24.4 (17.3-35.1)	27.7 (19.0-40.1)					
4-day	6.53 (5.37-7.94)	7.42 (6.10-9.04)	9.13 (7.48-11.1)	10.8 (8.78-13.2)	13.3 (10.8-17.4)	15.6 (12.3-20.6)	18.1 (13.8-24.5)	20.8 (15.3-29.1)	24.9 (17.6-35.7)	28.2 (19.4-40.6)					
7-day	7.95 (6.58-9.62)	8.69 (7.18-10.5)	10.2 (8.38-12.3)	11.7 (9.56-14.2)	14.1 (11.5-18.3)	16.3 (12.9-21.4)	18.8 (14.4-25.3)	21.6 (15.9-29.9)	25.7 (18.3-36.7)	29.1 (20.1-41.8)					
10-day	9.09 (7.55-11.0)	9.83 (8.15-11.9)	11.3 (9.36-13.7)	12.8 (10.5-15.6)	15.3 (12.4-19.7)	17.5 (13.9-22.8)	20.0 (15.4-26.8)	22.7 (16.9-31.4)	26.8 (19.2-38.2)	30.3 (21.0-43.3)					
20-day	11.9 (9.97-14.3)	13.2 (11.1-15.9)	15.5 (12.9-18.6)	17.5 (14.5-21.1)	20.4 (16.6-25.8)	22.9 (18.1-29.2)	25.4 (19.6-33.4)	28.1 (20.9-38.2)	31.9 (22.9-44.7)	34.9 (24.5-49.7)					
30-day	14.4 (12.1-17.1)	16.1 (13.5-19.3)	19.1 (15.9-22.8)	21.5 (17.9-25.9)	24.8 (20.1-30.9)	27.4 (21.7-34.7)	30.0 (23.1-39.1)	32.6 (24.3-43.8)	36.1 (26.0-50.1)	38.7 (27.2-54.8)					
45-day	17.7 (14.9-21.0)	19.9 (16.7-23.6)	23.4 (19.6-27.9)	26.2 (21.8-31.3)	29.8 (24.1-36.7)	32.5 (25.8-40.8)	35.1 (27.1-45.3)	37.6 (28.0-50.1)	40.8 (29.3-56.1)	43.0 (30.4-60.6)					
60-day	20.7 (17.5-24.5)	23.1 (19.5-27.4)	26.8 (22.6-31.9)	29.8 (24.9-35.5)	33.5 (27.2-41.1)	36.3 (28.8-45.2)	38.8 (30.0-49.8)	41.2 (30.7-54.6)	44.1 (31.8-60.4)	46.1 (32.7-64.8)					

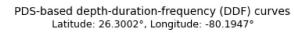
¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

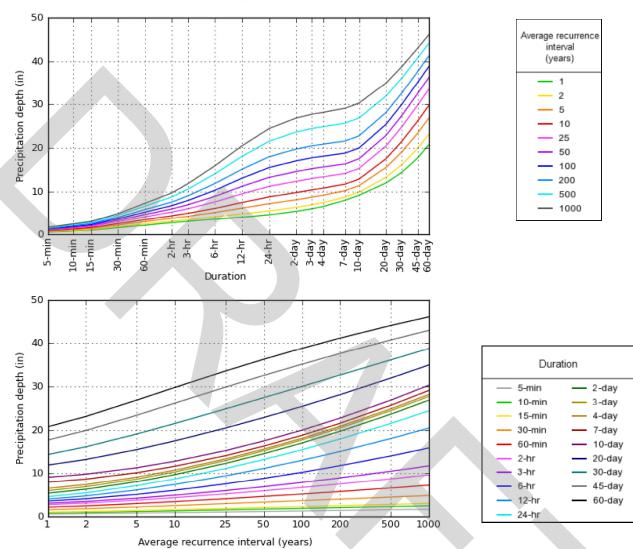
Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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PF graphical



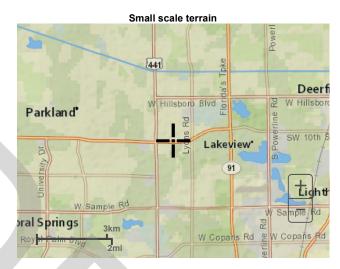


NOAA Atlas 14, Volume 9, Version 2

Created (GMT): Tue Feb 26 20:29:06 2019



Maps & aerials



Large scale terrain



Large scale map

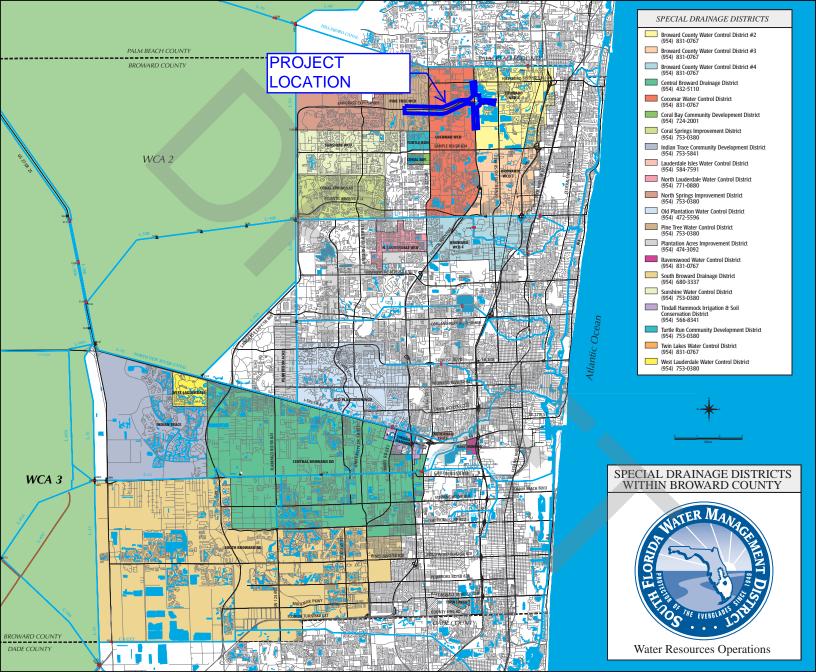




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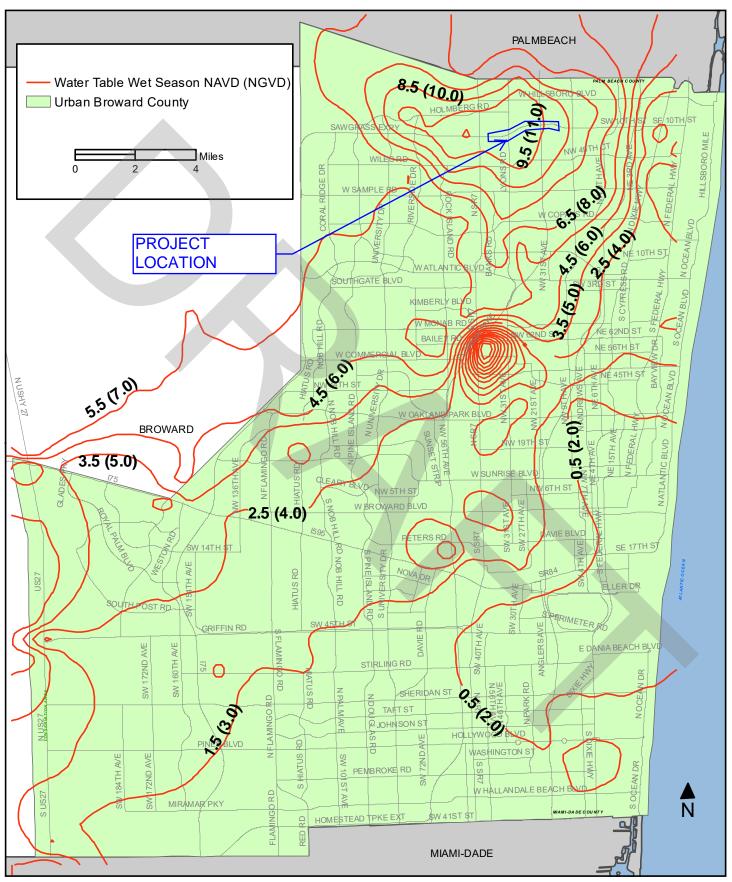
US Department of Commerce National Oceanic and Atmospheric Administration National Weather Service National Water Center 1325 East West Highway Silver Spring, MD 20910 Questions?: HDSC.Questions@noaa.gov

Disclaimer





WATER TABLE MAP - AVERAGE WET SEASON



Division Name: Planning and Environmental Regulation Department Name: Environmental Protection and Growth Management This map is for conceptual purposes only and should not be used for legal boundary determinations. Elevation converted from NGDV to NAVD using the FEMA approved conversion factor for Broward County of (-) 1.5



www.floridasturnpike.com/sawgrass